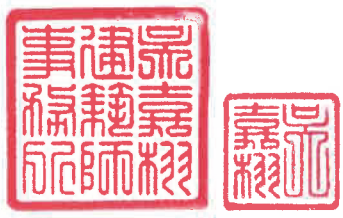
臺中市立長億高級中學
圖書館資訊暨教學大樓新建工程

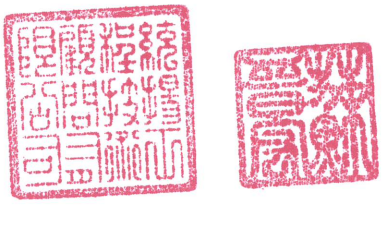
結 構 計 算 書



業 主：臺中市立長億高級中學

規劃設計監造單位：吳嘉栩建築師事務所

中華民國110年09月 (定稿版)

建築物結構與設備專業技師簽證報告	
建築位置	地號：
	地址：
建築規模	地上　　層 地下　　層
簽證內容	結構部份由本技師事務所簽證負責 計算書　　份 圖　樣　　張 說明書　　份
簽證技師	姓名：蘇 晉 宏
	執業執照號碼：技執字第 003258 號
	許可文號：內授營建管字第 10408099621 號
	公司名稱：統揚工程技術顧問有限公司
	公司地址：台中市西屯區文中路 2 號 10F-1
	事務所電話：04-23110020
	執業圖記   蘇晉宏
日　　期	中 華 民 國　　年　　月　　日
備　　註	登記號碼：230130

結　構　計　算　書

蘇晉宏

工程名稱：臺中市立長億高級中學圖書館

資訊暨教學大樓新建工程

設計單位：吳嘉栩建築師事務所

結構設計：統揚工程技術顧問有限公司

設計日期：2021年09月05日

目 錄

臺中市立長億高級中學

圖書館資訊暨教學大樓新建工程結構計算書

壹、工程概述

一、建築座落：台中市太平區福星段841等3筆地號。

二、建築規模：地上4層、地下1層。

三、建築高度：20.15公尺。

四、用途類別：圖書館、教室。

五、構造類別：鋼筋混凝土。

六、地盤種類：第一類地盤。

七、結構系統：韌性抗彎矩構架系統。

八、鄰近斷層：車籠埔斷層（距離1.77km）。

九、建築物基本資料：

樓層	樓層高度（m）	面積（m^2）	用途
PH	—	99.7	屋頂突出物
RF	**3.80**	345.8	屋頂平台
4F	**4.00**	830.4	階梯教室
3F	**4.00**	830.4	教室
2F	**4.00**	830.4	圖書館
1F	**4.45**	830.4	辦公室
BF	**3.70**	1140.8	停車場

面積以柱心範圍計算

貳、結構分析概要

一、靜載重計算

（1）15cm RC 隔間牆　$2.4\times0.15+2\times0.02=0.4t/m^2$

（2）15cm RC 外牆　$2.4\times0.15+2\times0.02+0.02=0.42t/m^2$

（3）25cm RC 外牆　$2.4\times0.25+3\times0.02=0.66t/m^2$

（4）屋突層樓板

15cm R.C.　$2.4\times0.15=0.36t/m^2$

防水材料　$=0.02t/m^2$

隔熱處理　$=0.06t/m^2$

水泥沙漿　$0.02\times2.0=0.04t/m^2$

$\boxed{\Sigma=0.48t/m^2}$

（5）屋頂層樓板

20cm R.C. $2.4\times0.20=0.48t/m^2$

防水材料 $=0.02t/m^2$

隔熱處理 $=0.06t/m^2$

水泥沙漿 $0.02\times2.0=0.04t/m^2$

$\boxed{\Sigma=0.60t/m^2}$

（6）四~一層樓板

20cm R.C. $2.4\times0.20=0.48t/m^2$

水泥沙漿 $0.02\times2.0=0.04t/m^2$

地坪飾材 $=0.03t/m^2$

$\boxed{\Sigma=0.55t/m^2}$

（7）一層樓板室外

25cm R.C. $2.4\times0.25=0.60t/m^2$

水泥沙漿 $0.02\times4.0=0.08t/m^2$

地坪飾材 $=0.05t/m^2$

$\boxed{\Sigma=0.73t/m^2}$

（8）筏式基礎版

50cm R.C. $2.4\times0.5=1.2t/m^2$

水泥沙漿 $0.02\times2.0=0.04t/m^2$

$\boxed{\Sigma=1.24t/m^2}$

樓層	RFL	(A=345.8)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	420	0.15	3.00	3.35	0.00	1	4221.0	Y-1
2	外牆	420	0.15	3.00	3.35	0.00	1	4221.0	Y-2
3	外牆	420	0.15	3.00	12.20	1.80	1	14616.0	Y-4
4	外牆	420	0.15	3.00	11.95	4.50	1	13167.0	Y-5
5	外牆	420	0.15	3.20	6.95	2.21	1	8414.7	X-1,2
6	女兒牆	420	0.15	1.20	17.05	0.00	1	8593.2	X-1,2
7	女兒牆	420	0.15	1.20	17.05	0.00	1	8593.2	X-3
8	女兒牆	420	0.15	1.20	6.95	0.00	1	3502.8	X-4
9	女兒牆	420	0.15	1.20	16.00	0.00	1	8064.0	X-6
10	隔間	400	0.15	3.40	13.45	3.60	1	16852.0	電梯
								90244.9	
				樓版平均	承載重量=			**261.0**	kg/m^2

樓層	4FL	(A=830.4)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	420	0.15	3.20	18.05	0.00	1	24259.2	Y-1
2	外牆	420	0.15	3.20	23.10	1.80	1	30290.4	Y-2
3	外牆	420	0.15	3.20	30.10	1.80	1	39698.4	Y-4
4	外牆	420	0.15	3.20	18.00	4.50	1	22302.0	Y-5
5	外牆	420	0.15	3.20	23.00	6.71	1	28095.9	X-1,2
6	外牆	420	0.15	3.20	3.35	0.00	1	4502.4	X-4
7	外牆	420	0.15	3.20	16.00	0.00	1	21504.0	X-6
8	隔間	400	0.15	3.40	39.10	3.60	1	51736.0	廁所
9	隔間	400	0.15	3.40	7.60	1.80	1	9616.0	電梯
10	隔間	400	0.15	3.40	22.10	0.00	1	30056.0	舞台區
11	隔間	400	0.15	3.30	8.25	1.80	1	10170.0	樓梯外平台
12	隔間	360	0.15	1.15	18.50	0.00	1	7659.0	階梯撐牆
13	隔間	360	0.15	1.00	18.50	0.00	1	6660.0	階梯撐牆
14	隔間	360	0.15	0.85	18.50	0.00	1	5661.0	階梯撐牆
15	隔間	360	0.15	0.70	18.50	0.00	1	4662.0	階梯撐牆
16	隔間	360	0.15	0.65	18.50	0.00	1	4329.0	階梯撐牆
17	隔間	360	0.15	0.50	18.50	0.00	1	3330.0	階梯撐牆
18	隔間	360	0.15	0.35	18.50	0.00	1	2331.0	階梯撐牆
19	隔間	360	0.15	0.20	18.50	0.00	1	1332.0	階梯撐牆
								308194.3	
				樓版平均	承載重量=			**371.1**	kg/m^2

樓層	3FL	(A=830.4)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	420	0.15	3.20	14.35	0.00	1	19286.4	Y-1
2	外牆	420	0.15	3.20	21.10	0.00	1	28358.4	Y-2
3	外牆	420	0.15	3.20	28.05	0.00	1	37699.2	Y-4
4	外牆	420	0.15	3.20	18.00	4.50	1	22302.0	Y-5
5	外牆	420	0.15	3.20	23.00	6.71	1	28095.9	X-1,2
6	外牆	420	0.15	3.20	23.00	8.00	1	27552.0	X-4
7	外牆	420	0.15	3.20	16.00	0.00	1	21504.0	X-6
8	隔間	400	0.15	3.40	26.58	2.00	1	35348.8	廁所
9	隔間	400	0.15	3.40	7.60	1.80	1	9616.0	電梯
10	隔間	400	0.15	3.40	22.10	7.20	1	27176.0	自習教室
11	隔間	400	0.15	3.40	3.90	1.80	1	4584.0	儲藏室
								261522.7	
				樓版平均	承載重量=			**314.9**	kg/m^2

樓層	2FL	(A=830.4)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	420	0.15	3.20	16.60	0.00	1	22310.4	Y-1
2	外牆	420	0.15	3.20	20.70	0.00	1	27820.8	Y-2
3	隔間牆	400	0.15	3.20	3.65	0.00	1	4672.0	Y-3
4	隔間牆	400	0.15	3.20	30.20	5.80	1	36336.0	Y-4
5	外牆	420	0.15	3.20	18.00	4.50	1	22302.0	Y-5
6	外牆	420	0.15	3.20	23.00	6.71	1	28095.9	X-1,2
7	外牆	420	0.15	3.20	3.35	0.00	1	4502.4	X-4
8	外牆	420	0.15	3.20	8.00	0.00	1	10752.0	X-6
9	隔間	400	0.15	3.40	26.58	2.00	1	35348.8	廁所
10	隔間	400	0.15	3.40	7.60	1.80	1	9616.0	電梯
11	隔間	400	0.15	3.40	58.43	11.60	1	74824.8	圖書教室
								276581.1	
				樓版平均	承載重量=			**333.1**	kg/m^2

樓層	1FL	(A=830.4)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	420	0.15	3.65	9.37	0.00	1	14364.2	Y-1
2	隔間牆	400	0.15	3.65	14.06	3.60	1	19087.6	Y-2
3	隔間牆	400	0.15	3.65	8.30	5.60	1	9878.0	Y-3
4	隔間牆	400	0.15	3.65	13.81	1.00	1	19762.6	Y-4
5	外牆	420	0.15	3.65	18.00	2.25	1	26649.0	Y-5
6	外牆	420	0.15	3.65	14.95	0.00	1	22918.4	X-1,2
7	隔間	400	0.15	3.65	11.35	0.90	1	16211.0	X-3
8	外牆	420	0.15	3.65	6.30	0.00	1	9657.9	X-4
9	隔間	400	0.15	3.85	15.93	0.00	1	24532.2	廁所電梯
10	隔間	400	0.15	3.85	9.30	0.00	1	14322.0	會計室
11	隔間	400	0.15	3.85	4.50	2.10	1	6090.0	樓梯管道間
								183472.9	
				樓版平均	承載重量=			**220.9**	kg/m^2

樓層	BFL	(A=1140.8)							
序號	部位	單位重	厚度(m)	高度(m)	寬度(m)	扣除開口	數量	重量(kg)	備註
1	外牆	660	0.25	2.90	43.85	0.00	1	83928.9	Y-1
2	外牆	660	0.25	2.90	11.45	0.00	1	21915.3	Y-3
3	外牆	660	0.25	2.90	15.40	0.00	1	29475.6	Y-4
4	外牆	660	0.25	2.90	18.00	0.00	1	34452.0	Y-5
5	外牆	660	0.25	2.90	23.00	0.00	1	44022.0	X-1,2
6	隔間	400	0.15	2.90	3.35	1.80	1	3166.0	X-3
7	外牆	660	0.25	2.90	3.35	0.00	1	6411.9	X-4
8	外牆	660	0.25	2.90	8.00	0.00	1	15312.0	X-6
9	外牆	660	0.25	2.90	30.30	0.00	1	57994.2	車道邊
10	隔間	400	0.15	3.10	57.80	14.20	1	65992.0	機房梯間
								362669.9	
				樓版平均	承載重量=			**317.9**	kg/m^2

二、各層樓版垂直載重、及總重量累計表

各層樓版垂直載重表

樓層	用途	靜載重（kg/m^2）	活載重（kg/m^2）
RF	屋頂平台	480	300
RF	水塔區	861	550
RF	鋼構屋頂	300	100
4F	準備區	921	300
4F	階梯教室	921	400
3F	教室	865	250
2F	閱覽室	883	400
2F	密集書庫	883	900
2F	辦公區	883	300
1F	辦公區	771	300
1F	室外	750	600
BF	停車場	868	500

樓層重量計算表

樓層	樓層面積	牆自重	版自重	梁柱自重	樓層載重
	(m^2)	A(t/m^2)	B(t/m^2)	C(t/m^2)	A+B+C+D(t/m^2)
PH	99.70	0.000	0.480	0.524	1.004
RFL	830.40	0.261	0.600	0.362	1.223
4FL	830.40	0.371	0.550	0.549	1.470
3FL	830.40	0.315	0.550	0.556	1.421
2FL	830.40	0.333	0.550	0.565	1.448
1FL	830.40	0.221	0.550	0.658	1.429
BFL	1140.80	0.318	0.550	1.044	1.912

三、建築物設計地震力

（一）考慮設計地震時之水平總橫力（VD）計算如下：

1. 震區短週期與一秒週期設計水平譜加速度係數

$Ss^D = 0.8$　　$S1^D = 0.45$

2. 短週期工址放大係數 $Fa = 1.0$

3. 長週期工址放大係數 $Fv = 1.0$

4. 工址短週期與一秒週期設計水平譜加速度係數

$SDs = Ss^D \times Fa \times Na = 0.8 \times 1.0 \times 1.23 = 0.984$

$SD1 = S1^D \times Fv \times Nv = 0.45 \times 1.0 \times 1.36 = 0.612$

5. 工址設計水平加速度反應譜短週期與中、長週期之分界

$$T0^D = \frac{SD1}{SDS} = \frac{0.612}{0.984} = 0.622\,\text{sec}$$

6. 建築物基本震動周期 $T=0.05hn^{3/4}=0.05\times20.15^{3/4}=0.476\text{sec}$

$T_{ana} = 0.742\,\text{sec}$　　$T = Min(0.742, 1.4 \times 0.476) = 0.666\,\text{sec}$

$$\frac{T}{T0^D} = \frac{0.666}{0.622} = 1.071$$　　$T = 1.071 T0^D$

7. 結構系統韌性容量　R＝4.0

8. 容許韌性容量

$$Ra = 1 + \frac{(R-1)}{1.5} = 1 + \frac{(4-1)}{1.5} = 3.0$$

9. 結構系統地震力折減係數

■ $T \geq T0^D$

$FuD = Ra = 3.0$

10. 工址設計水平譜加速度係數

$$SaD = \frac{S_{D1}}{T} = \frac{0.612}{0.666} = 0.919$$　　$$\frac{SaD}{FuD} = \frac{0.919}{3.0} = 0.306$$

11. 起始降伏地震力放大倍數　α_y＝1.0

■ $0.3 < \frac{SaD}{FuD} < 0.8$　　$\left(\frac{SaD}{FuD}\right)_m = 0.52 \times 0.306 + 0.144 = 0.303$

$$VD = \frac{I}{1.4\alpha y} \times \left(\frac{SaD}{FuD}\right)_m \times W = \frac{1.25}{1.4 \times 1.0} \times 0.303 \times W = 0.271W$$

（二）最大考量地震時之水平總橫力（VM）計算如下：

1. 震區短週期與一秒週期最大考量水平譜加速度係數

$Ss^M = 1.0$　　$S1^M = 0.55$

2. 短週期工址放大係數 $Fa = 1.0$

3. 長週期工址放大係數 $Fv = 1.0$

4. 工址短週期與一秒週期最大考量水平譜加速度係數

$$SMs = Ss^{M} \times Fa \times Na = 1.0 \times 1.0 \times 1.25 = 1.25$$

$$SM1 = S1^{M} \times Fv \times Nv = 0.55 \times 1.0 \times 1.5 = 0.825$$

5. 工址最大考量水平加速度反應譜短週期與中、長週期之分界

$$T0^{M} = \frac{SM1}{SMS} = \frac{0.825}{1.25} = 0.660\,\text{sec}$$

6. 建築物基本震動周期T=0.05hn$^{3/4}$=0.05×20.15$^{3/4}$＝0.476sec

$$T_{ana} = 0.742\,\text{sec} \qquad T = Min(0.742, 1.4 \times 0.476) = 0.666\,\text{sec}$$

$$\frac{T}{T0^{M}} = \frac{0.666}{0.660} = 1.01 \qquad T = 1.01T0^{M}$$

7. 結構系統韌性容量　R＝4.0

8. 結構系統地震力折減係數

■　$T \geq T0^{M}$　　$FuM = R = 4.0$

9. 工址最大水平譜加速度係數　αy＝1.0

$$SaM = \frac{S_{M1}}{T} = \frac{0.825}{0.666} = 1.24 \qquad \frac{SaM}{FuM} = \frac{1.24}{4.0} = 0.310$$

■ $0.3 < \frac{SaM}{FuM} < 0.8$　　$\left(\frac{SaM}{FuM}\right)_m = 0.52 \times 0.31 + 0.144 = 0.305$

$$VM = \frac{I}{1.4\alpha y} \times \left(\frac{SaM}{FuM}\right)_m \times W = \frac{1.25}{1.4 \times 1.0} \times 0.305 \times W = 0.272W$$

（三）避免中小度地震降伏之設計地震力V*

$$V^* = \frac{I \times FuD}{4.2\alpha y} \times \left(\frac{SaD}{FuD}\right)_m \times W = \frac{1.25 \times 3.0}{4.2 \times 1.0} \times 0.303 \times W = 0.271W$$

$$V = \max(VD, VM, V^*) = 0.272W$$

採用最小設計水平總橫力V=0.272W

（四）容許層間相對位移檢核地震力

$$V' = \frac{IFu}{4.2} \times \left(\frac{SaD}{Fu}\right)_m \times W = \frac{1.0 \times 3.0}{4.2} \times 0.303 \times W = 0.216W$$

（五）垂直地震力

$$Vz = \frac{I}{1.4\alpha y} \times \left(\frac{SaD,V}{Fuv}\right)_m \times W$$　式中 $\left(\frac{SaD,V}{Fuv}\right)_m$如下

垂直向設計譜加速度係數 $SaD,V = \frac{2}{3}SaD = \frac{2}{3} \times 0.919 = 0.613$

結構系統韌性容量　R＝3.0

容許韌性容量　　$Ra = 1 + \frac{(R-1)}{1.5} = 1 + \frac{(3-1)}{1.5} = 2.333$

結構系統地震力折減係數

■ $T \geq T0^{D}$ $FuD = Ra = 2.333$

$$\frac{SaD,V}{Fuv} = \frac{0.613}{2.333} = 0.263$$

■ $0.2 < \frac{SaD,V}{Fuv} \leq 0.53$ $\left(\frac{SaD,V}{Fuv}\right)_m = 0.52 \times 0.263 + 0.096 = 0.233$

$$VZ = \frac{I}{1.4\alpha y} \times \left(\frac{SaD,V}{Fuv}\right)_m \times W = \frac{1.25}{1.4 \times 1.0} \times 0.233 \times W = 0.208W$$

（六）豎向分配

樓層	各層自重	各層高程	總高度	W*H	Fx	Fy
RF	1117.0	4.000	16.450	18374.7	482.5	482.5
4F	1221.1	4.000	12.450	15202.7	399.2	399.2
3F	1179.8	4.000	8.450	9969.3	261.8	261.8
2F	1202.6	4.450	4.450	5351.6	140.5	140.5
合計	4720.5			48898.2	1284.0	1284.0

V=0.272W=0.272*4720.5= 1284.0

四、建築物設計風力

（一）基本設計條件

1. 基本設計風速$V_{10}(C) = 32.5m/s$（台中市太平區）
2. 建築物屬於第三類建築物，用途係數$I = 1.1$
3. 地表特性屬於地況B, 風速垂直分佈指數$\alpha = 0.25$, 梯度高度$z_g = 400$
4. 建築物基本資料
   A. 建築高度$H = 20.15m$
   B. 長向之建築物水平尺寸$L = 39.2m$
   C. 短向之建築物水平尺寸$B = 27.3m$
5. 建築物高度處設計風速

（二）順風向風力計算

1. 風速壓計算$q(z) = 0.06K(z)Kzt[IV_{10}(C)]^2$

$z < 5m$ $K(z) = 2.774\left(\frac{5}{z_g}\right)^{2\alpha} = 2.774\left(\frac{5}{400}\right)^{0.5} = 0.310kg/m^2$

$z \geq 5m$ $K(z) = 2.774\left(\frac{z}{z_g}\right)^{2\alpha} = 2.774\left(\frac{z}{400}\right)^{0.5}$

$$q(h)=0.06\times 2.774\left(\frac{20.15}{400}\right)^{0.5}[1.1\times 32.5]^2=47.7kg/m^2$$

$$q(z)=0.06\times 2.774\left(\frac{z}{400}\right)^{0.5}[1.1\times 32.5]^2=212.7\times\left(\frac{z}{400}\right)^{0.5}kg/m^2$$

2. 順風向陣風反應因子 $G=1.72$

3. 順風向單面之設計風壓 p ，依據規範2.2式計算

（a）長向受風計算

$p=qGfCp-q(GCpi)$

平行風向尺寸/垂直風向尺寸 $=27.3/39.2=0.696$　查表2.4

迎風面使用風速壓 $q=q(z)$　$Cp=0.8$

背風面使用風速壓 $q=q(h)$　$Cp=-0.5$

內風壓使用風速壓 $q_i=q(h)$　$GCpi=-0.375$

順風向迎風面設計風壓

$$p_1(z)=q(z)\times 1.72\times 0.8-q(h)\times(-0.375)$$

$$=212.7\times\left(\frac{z}{400}\right)^{0.5}\times 1.72\times 0.8-47.7\times(-0.375)=292.7\left(\frac{z}{400}\right)^{0.5}+17.9$$

順風向背風面設計風壓

$$p_2(z)=q(h)\times 1.72\times(-0.5)-q(h)\times(-0.375)$$

$$=47.7\times 1.72\times(-0.5)-47.7\times(-0.375)=-23.1kg/m^2$$

順風向設計風壓 $=p_1(z)-p_2(z)=292.7\left(\frac{z}{400}\right)^{0.5}+41.0$

樓層 Y-DIR	樓層頂部 高程(m)	單層 樓高(m)	受風面積 (m^2)	順風向設計 風壓(kg/m^2)	各層順風向 設計風力(kg)
RFL	16.45	4.00	78.40	100.18	7853.9
4FL	12.45	4.00	156.80	92.43	14493.2
3FL	8.45	4.00	156.80	83.29	13059.8
2FL	4.45	4.45	163.66	71.52	11705.6
				Σ=	47112.5

（b）短向受風計算

$p=qGfCp-q(GCpi)$　　$L/B=39.2/27.3=1.43$　查表2.4

迎風面使用風速壓 $q=q(z)$　$Cp=0.8$

背風面使用風速壓 $q=q(h)$　$Cp=-0.3$

內風壓使用風速壓 $q_i=q(h)$　$GCpi=-0.375$

順風向迎風面設計風壓

$$p_1(z)=q(z)\times 1.73\times 0.8-q(h)\times(-0.375)$$

$$=212.7\times\left(\frac{z}{400}\right)^{0.5}\times 1.72\times 0.8-47.7\times(-0.375)=292.7\left(\frac{z}{400}\right)^{0.5}+17.9$$

順風向背風面設計風壓

$$p_2(z)=q(h)\times 1.72\times(-0.3)-q(h)\times(-0.375)$$

$$=47.7\times 1.72\times(-0.3)-47.7\times(-0.375)=-6.7kg/m^2$$

順風向設計風壓 $=p_1(z)-p_2(z)=292.7\left(\frac{z}{400}\right)^{0.5}+24.6$

樓層 X-DIR	樓層頂部 高程(m)	單層 樓高(m)	受風面積 (m^2)	順風向設計 風壓(kg/m^2)	各層順風向 設計風力(kg)
RFL	16.45	4.00	54.60	83.78	4574.2
4FL	12.45	4.00	109.20	76.03	8302.6
3FL	8.45	4.00	109.20	66.89	7304.4
2FL	4.45	4.45	113.98	55.12	6282.9
				Σ=	26464.0

（三）橫風向設計風力

因 $\frac{h}{\sqrt{BL}}=\frac{20.15}{\sqrt{27.3\times 39.2}}=0.62<3$

$$Vh=1.666V_{10}(C)\left(\frac{h}{z_g}\right)^{\alpha}=1.666\times 32.5\times\left(\frac{20.15}{400}\right)^{0.25}=25.7m/s$$

$$\frac{fa\sqrt{BL}}{Vh}=\frac{1.1\sqrt{27.3\times 39.2}}{25.7}=1.4>0.4$$ 可不做橫風向風力及扭矩檢核

六、材料規格

（一）混凝土：

28天抗壓強度：fc’＝280kg/cm^2。

（二）鋼筋抗拉強度：

fy＝2800kg/cm^2（＃3－＃5） (SD280)－非梁柱構材。

fy＝4200kg/cm^2（＃4－＃5） (SD420W)－梁柱構材。

fy＝4200kg/cm^2（＃6－＃10）(SD420W)。

（三）採用鋼筋續接器應符合內政部鋼筋續接器續接施工規範，梁柱構材應達到SA性能等級。

（四）材料應符合我國國家標準（CNS）或美國材料試驗學會規範（ASTM）或日本工業規格（JIS）等之規定。

七、結構尺寸說明

柱尺寸：80×70cm　　C1,C2,C11
100×100cm　C3,~C9
90×150cm　C10
60×60cm　C12,C14
80×80cm　C13,C16
60×70cm　C15

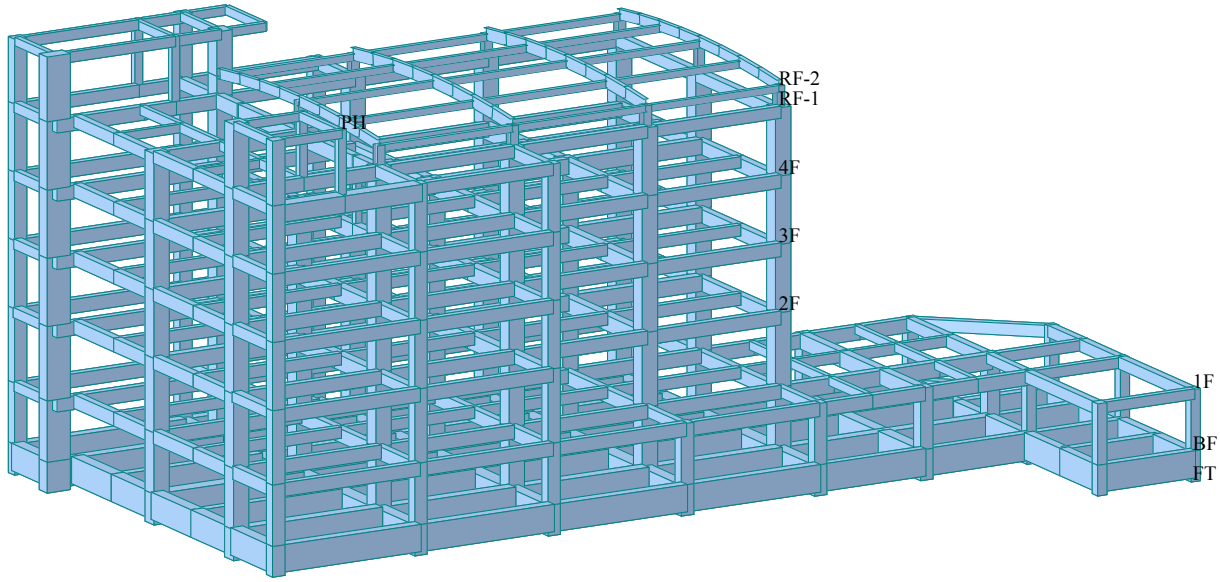
梁尺寸：45×60cm、50×70cm、50×80cm、 50×60cm、35×50cm
60×90㎝、65×90cm、45×70cm

地梁尺寸：80×160cm、70×160cm、60×160㎝、100×160cm、

樓版厚度：15cm、20cm

筏式基礎版厚度：50cm

八、結構配置說明：各層結構平面圖詳如設計圖說



電腦分析模型圖

參、結構設計說明

一、設計規範

（一）建築技術規則構造篇（最新版）。

（二）建築物耐震設計規範及解說(100年7月實施)。

（三）混凝土結構設計規範(106年7月實施)。

（四）建築物耐風設計規範及解說(104年1月實施)

（五）建築物基礎構造設計規範(91年7月實施)

（六）鋼結構設計規範(96年6月實施)

二、設計方法：極限強度設計法

三、結構設計程式：Midas Gen

四、靜力分析意外扭矩放大係數

X向地震力偏心5%之意外扭矩放大

樓層	第 1 點位移	第 2 點位移	平均位移 δ ave	最大位移 δ max	$(\delta max/1.2\delta ave)^2$
PH	0.0760	0.0730	0.0745	0.0760	0.723
RFL	0.0680	0.0640	0.0660	0.0680	0.737
4FL	0.0550	0.0540	0.0545	0.0550	0.707
3FL	0.0417	0.0416	0.0417	0.0417	0.696
2FL	0.0262	0.0252	0.0257	0.0262	0.722

如表所示意外扭矩不必放大　　Ax=1.0

Y向地震力偏心5%之意外扭矩放大

樓層	第 1 點位移	第 2 點位移	平均位移 δ ave	最大位移 δ max	$(\delta max/1.2\delta ave)^2$
PH	0.0680	0.0540	0.0610	0.0680	0.863
RFL	0.0940	0.0630	0.0785	0.0940	0.996
4FL	0.0740	0.0420	0.0580	0.0740	1.130
3FL	0.0550	0.0320	0.0435	0.0550	1.110
2FL	0.0332	0.0208	0.0270	0.0332	1.050

意外扭矩放大係數如上表所示

五、層間相對側向位移角

V'/V=	0.794	EX			
樓層名稱	樓層高	樓層最大位移	層間相對位移	相對位移角θ	相對位移角θ'
	(m)	(m)	(m)	(作用地震力V)	(作用地震力V')
RFL	4.00	0.06703	0.01358	0.00340	0.00270
4FL	4.00	0.05345	0.01306	0.00327	0.00259
3FL	4.00	0.04039	0.01529	0.00382	0.00304
2FL	4.45	0.02510	0.01620	0.00372	0.00296
1FL	-	0.00890	-	-	-
			θ'max=	0.00304	<0.005 (OK)

V'/V=	0.794	EY			
樓層名稱	樓層高	樓層最大位移	層間相對位移	相對位移角θ	相對位移角θ'
	(m)	(m)	(m)	(作用地震力V)	(作用地震力V')
RFL	4.00	0.07970	0.01624	0.00406	0.00322
4FL	4.00	0.06346	0.01559	0.00390	0.00310
3FL	4.00	0.04787	0.01811	0.00453	0.00360
2FL	4.45	0.02976	0.01943	0.00447	0.00355
1FL	-	0.01033	-	-	-
			θ'max=	0.00360	<0.005 (OK)

六、建築物之耐震間隔(碰撞距離)

X向 $S=0.6\times1.4\times\alpha_y\times Ra\times\triangle=0.6\times1.4\times1.0\times3.0\times6.70=16.9cm$

Y向 $S=0.6\times1.4\times\alpha_y\times Ra\times\triangle=0.6\times1.4\times1.0\times3.0\times7.97=20.1cm$

△：設計地震力作用下產生之最大位移

七、設計載重組合：

（一）1.4*DL*
（二）1.2*DL*+1.6*LL*
（三）1.2*DL*+1.0*LL*±1.6*WX*
（四）1.2*DL*+1.0*LL*±1.6*WY*
（五）0.9*DL*±1.6*WX*
（六）0.9*DL*±1.6*WY*
（七）1.2*DL*+1.0*LL*±1.0*EX* ±0.3*EZ*
（八）1.2*DL*+1.0*LL*±1.0*EY* ±0.3*EZ*
（九）1.2*DL*+1.0*LL*±1.0*EZ* ±0.3*EX*
（十）1.2*DL*+1.0*LL*±1.0*EZ* ±0.3*EY*
（十一）0.9*DL*±1.0*EX* ±0.3*EZ*
（十二）0.9*DL*±1.0*EY* ±0.3*EZ*
（十三）0.9*DL*±1.0*EZ* ±0.3*EX*
（十四）0.9*DL*±1.0*EZ* ±0.3*EY*

肆、結構分析設計報表

伍、其他項目檢核計算

一、地下室25cm外牆設計

單位重 $\gamma = 2\ t/m^3$　　地表加載q=1 t/m^2

摩擦角 $\varphi = 28°$　　凝聚力 $C = 0\ t/m^2$

靜止土壓係數$K_0 = (1-\sin\varphi) = 0.531$

土壓應力計算 $q_1 = \gamma HKa = 2.0\times 3.5\times 0.531 = 3.72t/m^2$

地表加載 $q_2 = qKa = 1.0\times 0.531 = 0.531t/m^2$

牆面設計彎矩

$$Mu = \frac{q_1 L^2}{46} + \frac{q_2 L^2}{24} = 1.6\times\left(\frac{3.73\times 3.5^2}{46} + \frac{0.531\times 3.5^2}{24}\right) = 2.02t-m/m$$

$$m = \frac{fy}{0.85\,fc'} = \frac{2800}{0.85\times 210} = 15.7$$

$$Rn = \frac{Mu}{\phi bd^2} = \frac{2.02\times 10^5}{0.9\times 100\times(25-4)^2} = 5.09$$

$$\rho = \frac{1}{m}\left(1-\sqrt{1-\frac{2Rnm}{fy}}\right) = \frac{1}{15.7}\left(1-\sqrt{1-\frac{2\times 5.09\times 15.7}{2800}}\right) = 1.84\times 10^{-3}$$

$$As = \rho bd = 1.84\times 10^{-3}\times 100\times(25-4) = 3.87cm^2$$

$$Ast = \rho bd = 0.002\times 100\times 25 = 5cm^2$$

採用#4@20 $As = 1.27\times\frac{100}{20} = 6.35cm^2 > 5.0cm^2$（ok）

二、樓梯設計

跨度 L=5.8m　　厚度 T=20cm

設計載重

$$Wd = (2.4\times 0.25 + 0.02\times 3)\times 1 = 0.66t/m$$

$$W_L = 0.4\times 1.0 = 0.4t/m^2$$

$$W_U = 1.2\times 0.66 + 1.6\times 0.4 = 1.43t/m$$

設計彎矩

$$Mu = \frac{WuL^2}{8} = \frac{1.43\times 5.8^2}{8} = 6.0t-m/m$$

設計配筋

$$m = \frac{fy}{0.85\,fc'} = \frac{2800}{0.85\times 210} = 15.7$$

$$Rn = \frac{Mu}{\phi bd^2} = \frac{6.0\times 10^5}{0.9\times 100\times(20-2.5)^2} = 21.8$$

$$\rho=\frac{1}{m}\left(1-\sqrt{1-\frac{2Rnm}{fy}}\right)=\frac{1}{15.7}\left(1-\sqrt{1-\frac{2\times21.8\times15.7}{2800}}\right)=0.0083$$

$$As=\rho bd=0.0083\times100\times(20-2.5)=14.5cm^2$$

$$Ast=\rho bd=0.0025\times100\times20=5cm^2$$

採用#5@12　$As=1.98\times\frac{100}{12}=16.5cm^2>14.5cm^2$（ok）

三、小樑設計

1.小樑b1：跨度 L=9.0m 斷面 40cm×60cm

承載面積 $A=2.25\times(\frac{4.5+9}{2})\times2=30.4m^2$

設計載重

靜載重 $W=2.4\times0.4\times0.45+(0.65\times30.4)/9.0=2.63t/m$

活載重 $W_L=0.3\times30.4/9.0=1.01t/m$

$W_U=1.2\times2.63+1.6\times1.01=4.8t/m$

設計彎矩

$$Mu=\frac{WuL^2}{8}=\frac{4.8\times9.0^2}{8}=48.6t-m$$

設計剪力

$$Vu=\frac{WuL}{2}=\frac{4.8\times9.0}{2}=21.6t$$

設計配筋

中央主筋

$$\mathrm{m}=\frac{fy}{0.85\,fc'}=\frac{4200}{0.85\times280}=17.6$$

$$\mathrm{Rn}=\frac{Mu}{\phi bd^2}=\frac{48.6\times10^5}{0.9\times40\times(60-5)^2}=40.8$$

$$\rho=\frac{1}{m}\left(1-\sqrt{1-\frac{2Rnm}{fy}}\right)=\frac{1}{17.6}\left(1-\sqrt{1-\frac{2\times40.8\times17.6}{4200}}\right)=0.0107$$

$$As=\rho bd=0.0107\times40\times(60-5)=23.6cm^2$$

採用5-#8　　$As=5.07\times5=25.4cm^2>23.6cm^2$（ok）

端部箍筋

$$Vc=0.53\sqrt{fc'}bd=0.53\times\sqrt{280}\times40\times(60-5)=19511kg=19.5t$$

$$Vs=\frac{Vu}{\phi}-Vc=\frac{21.6}{0.75}-19.5=9.3t$$

$$S=\frac{Av\times fy\times d}{Vs}=\frac{2\times1.27\times2800\times(60-5)}{9.3\times1000}=42.1cm$$

$$S=\min(\frac{d}{2}.42.1)=27.5cm$$

採用#4@20

2.小樑b2：跨度 L=9.0m 斷面 45cm×70cm

承載面積 $A=2.25\times(\frac{4.5+9}{2})\times2=30.4m^2$

設計載重

靜載重 $W=2.4\times0.45\times0.5+(0.65\times30.4)/9.0=2.74t/m$

活載重 $W_L=0.9\times30.4/9.0=3.04t/m$

$W_U=1.2\times2.74+1.6\times3.04=8.15t/m$

設計彎矩

$$Mu=\frac{WuL^2}{8}=\frac{8.15\times9.0^2}{8}=82.5t-m$$

設計剪力

$$Vu=\frac{WuL}{2}=\frac{8.15\times9.0}{2}=36.7t$$

設計配筋

中央主筋

$$\mathrm{m}=\frac{fy}{0.85\,fc'}=\frac{4200}{0.85\times280}=17.6$$

$$\mathrm{Rn}=\frac{Mu}{\phi bd^2}=\frac{82.5\times10^5}{0.9\times45\times(70-5)^2}=48.2$$

$$\rho=\frac{1}{m}\left(1-\sqrt{1-\frac{2Rnm}{fy}}\right)=\frac{1}{17.6}\left(1-\sqrt{1-\frac{2\times48.2\times17.6}{4200}}\right)=0.013$$

$$As=\rho bd=0.013\times45\times(70-5)=38.0cm^2$$

採用8-#8　　$As=5.07\times8=40.6cm^2>38.0cm^2$（ok）

端部箍筋

$$Vc=0.53\sqrt{fc'}bd=0.53\times\sqrt{280}\times45\times(70-5)=25941kg=25.9t$$

$$Vs=\frac{Vu}{\phi}-Vc=\frac{36.7}{0.75}-25.9=23.0t$$

$$S=\frac{Av\times fy\times d}{Vs}=\frac{2\times1.27\times2800\times(70-5)}{23.0\times1000}=20.0cm$$

$$S=\min(\frac{d}{2}.20.0)=20.0cm$$

採用#4@20

3.小樑b0：跨度 L=4.4m 斷面 35cm×50cm

設計載重

靜載重 $W = 2.4\times0.35\times0.35 + 0.65\times4.5 = 3.22t/m$

活載重 $W_L = 0.4\times4.5 = 1.8t/m$

$W_U = 1.2\times3.22 + 1.6\times1.8 = 6.74t/m$

設計彎矩

$$Mu = \frac{WuL^2}{8} = \frac{6.74\times4.4^2}{8} = 16.3t-m$$

設計剪力

$$Vu = \frac{WuL}{2} = \frac{6.74\times4.4}{2} = 14.8t$$

設計配筋

中央主筋

$$\mathrm{m} = \frac{fy}{0.85\,fc'} = \frac{4200}{0.85\times280} = 17.6$$

$$\mathrm{Rn} = \frac{Mu}{\phi bd^2} = \frac{16.3\times10^5}{0.9\times35\times(50-5)^2} = 25.6$$

$$\rho = \frac{1}{m}\left(1-\sqrt{1-\frac{2Rnm}{fy}}\right) = \frac{1}{17.6}\left(1-\sqrt{1-\frac{2\times25.6\times17.6}{4200}}\right) = 6.46\times10^{-3}$$

$$As = \rho bd = 6.46\times10^{-3}\times35\times(50-5) = 10.2cm^2$$

採用4-#6 $As = 2.85\times4 = 11.4cm^2 > 10.2cm^2$（ok）

端部箍筋

$$Vc = 0.53\sqrt{fc'}bd = 0.53\times\sqrt{280}\times35\times(50-5) = 13968kg = 14.0t$$

$$Vs = \frac{Vu}{\phi} - Vc = \frac{14.8}{0.75} - 14.0 = 5.7t$$

$$S = \frac{Av\times fy\times d}{Vs} = \frac{2\times1.27\times2800\times(50-5)}{5.7\times1000} = 56.1cm$$

$$S = \frac{d}{2} = \frac{(50-5)}{2} = 22.5cm$$

採用#4@20

4.鋼樑RSG1 L=9.0m 斷面 H-390×300×10×16

斷面係數 $Zx = 1980.0cm^3$ 斷面積 $A = 136cm^2$

設計載重

靜載重 $W = 107 + 200 \times 4.5 = 1007kg/m$

活載重 $W_L = 100 \times 4.5 = 450kg/m$

$W_U = 1007 + 450 = 1457kg/m$

設計彎矩

$$Mu = \frac{WuL^2}{8} = \frac{1457 \times 9.0^2}{8} = 14752kg - m$$

設計剪力

$$Vu = \frac{WuL}{2} = \frac{1457 \times 9.0}{2} = 6557kg$$

需要斷面係數 $Zx = \frac{Mu}{Fb} = \frac{14752 \times 100}{0.6 \times 2500} = 983.5cm^3 < 1980kg/cm^3 (OK)$

剪應力檢核 $fv = \frac{Vu}{dt} = \frac{6557}{39 \times 1} = 168.1kg/cm^2 < 0.4\,fy = 1000kg/cm^2 (OK)$

5.鋼樑Sb1 L=4.5m 斷面 H-300×150×6.5×9

斷面係數 $Zx = 481.0cm^3$ 斷面積 $A = 46.8cm^2$

設計載重

靜載重 $W = 37 + 200 \times 4.5 = 937kg/m$

活載重 $W_L = 100 \times 4.5 = 450kg/m$

$W_U = 937 + 450 = 1387kg/m$

設計彎矩

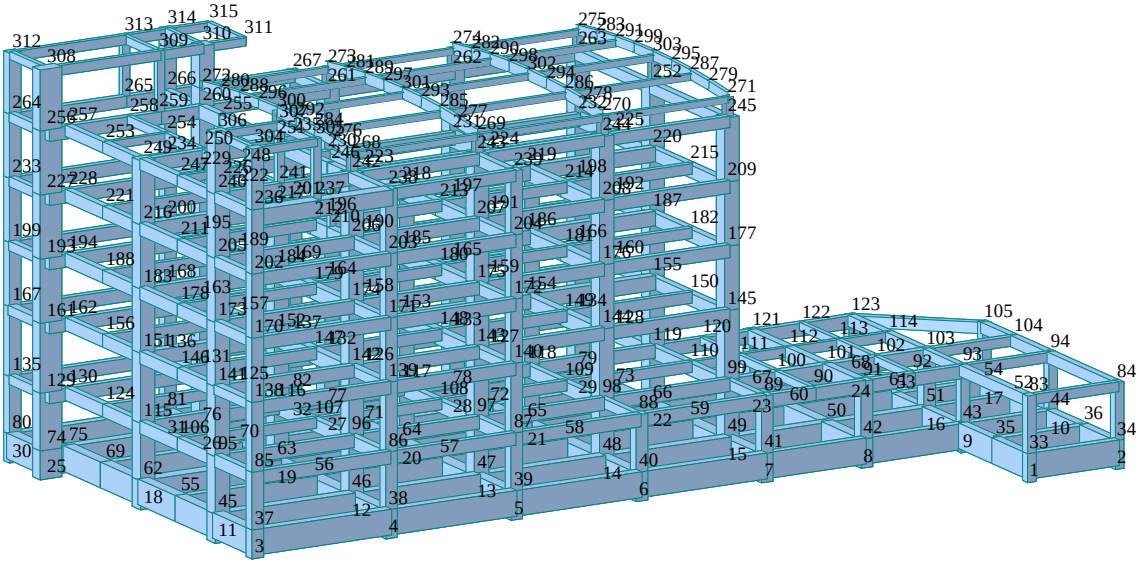
$$Mu = \frac{WuL^2}{8} = \frac{1387 \times 4.5^2}{8} = 3511kg - m$$

設計剪力

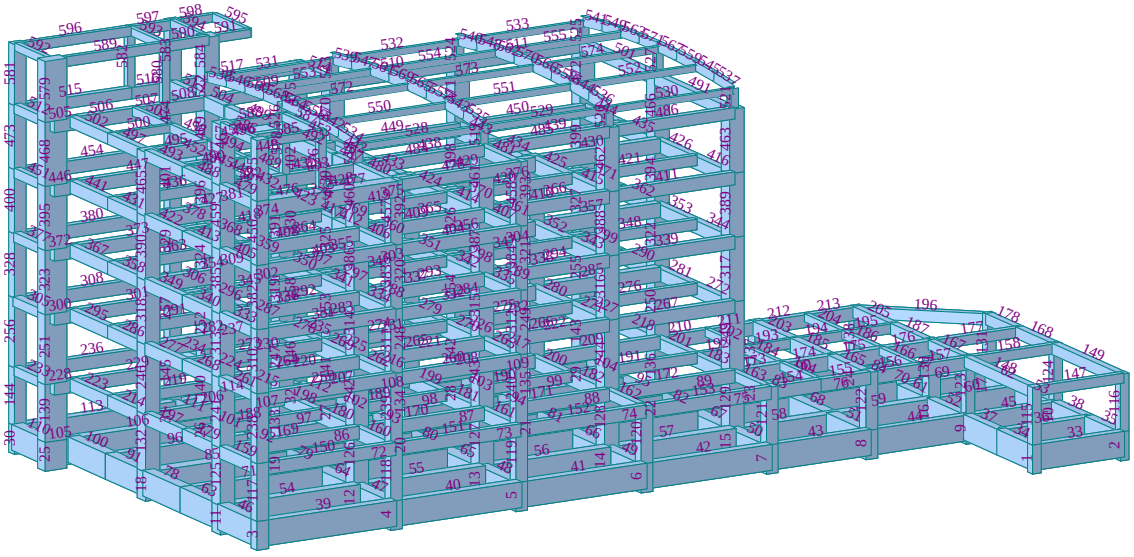
$$Vu = \frac{WuL}{2} = \frac{1387 \times 4.5}{2} = 3121kg$$

需要斷面係數 $Zx = \frac{Mu}{Fb} = \frac{3511 \times 100}{0.6 \times 2500} = 234.1cm^3 < 481.0cm^3 (OK)$

剪應力檢核 $fv = \frac{Vu}{dt} = \frac{3121}{30 \times 0.65} = 160.1kg/cm^2 < 0.4\,fy = 1000kg/cm^2 (OK)$



分析模型圖（節點編號）



分析模型圖（桿件編號）

*** PROJECT INFORMATION

Project Name : 長億高中圖書館
Date : 2021/5/31

*** CONTROL DATA

Panel Zone Effect : Do not Calculate
Unit System : KGF, CM
Definition of Frame
- X Direction of Frame : Unbraced I Sway
- Y Direction of Frame : Unbraced I Sway
- Design Type : 3-D

*** LOAD CASE DATA

NO	NAME	TYPE	SELF WEIGHT FACTOR X	Y	Z	DESCRIPTION
7	SW	D	0.000	0.000	-1.000	INPUT ELEMENT
1	DL	D	0.000	0.000	0.000	SLAB DEAD LOAD
2	LL	L	0.000	0.000	0.000	SLAB LIVE LOAD
3	EX	E	0.000	0.000	0.000	X-DIR
4	EY	E	0.000	0.000	0.000	Y-DIR
5	WX	W	0.000	0.000	0.000	Y-DIR
6	WY	W	0.000	0.000	0.000	Y-DIR
8	EZ	EVT	0.000	0.000	-0.104	Z-DIR

*** MATERIAL PROPERTY DATA

NO	NAME	TYPE	MODULUS OF ELASTICITY	SHEAR MODULUS	THERMAL COEFF.	POISSON RATIO	WEIGHT DENSITY
1	C280	CONC	2.492e+005	1.068e+005	5.556e-006	0.167	0.0024
2	STKR400	STEEL	2.1e+006	8.077e+005	6.667e-006	0.3	0.00785
3	SN400	STEEL	2.1e+006	8.077e+005	6.667e-006	0.3	0.00785
4	SS400	STEEL	2.1e+006	8.077e+005	6.667e-006	0.3	0.00785

NO	NAME	TYPE	STRENGTH OF DESIGN MATERIAL STEEL	CONCRETE	MAIN REBAR	SUB REBAR
1	C280	CONC	-	280	4200	4200
2	STKR400	STEEL	2500	-	-	-
3	SN400	STEEL	2400	-	-	-
4	SS400	STEEL	2500	-	-	-

*** STORY DATA

Ground Level = 0

NAME	LEVEL	HEIGHT	FLOOR DIAPHRAGM
PH	2015.000	0.000	Consider
RF-2	1755.000	260.000	Do not consider
RF-1	1635.000	120.000	Consider
4F	1235.000	400.000	Consider
3F	835.000	400.000	Consider
2F	435.000	400.000	Consider
1F	0.000	435.000	Do not consider
BF	-360.000	360.000	Do not consider

FT -540.000 180.000 Do not consider

*** NODE DATA

NO	X	Y	Z	TEMPERATURE
1	5070	-800	-540	0
2	5680	-800	-540	0
3	150	0	-540	0
4	1080	0	-540	0
5	1950	0	-540	0
6	2820	0	-540	0
7	3690	0	-540	0
8	4380	0	-540	0
9	5070	0	-540	0
10	5680	0	-540	0
11	150	440	-540	0
12	1080	440	-540	0
13	1950	440	-540	0
14	2820	440	-540	0
15	3690	440	-540	0
16	5070	440	-540	0
17	5680	800	-540	0
18	150	1340	-540	0
19	1080	1340	-540	0
20	1950	1340	-540	0
21	2820	1340	-540	0
22	3690	1340	-540	0
23	4380	1340	-540	0
24	5070	1340	-540	0
25	0	2240	-540	0
26	1080	2240	-540	0
27	1950	2240	-540	0
28	2820	2240	-540	0
29	3690	2240	-540	0
30	0	2660	-540	0
31	1080	2660	-540	0
32	1950	2660	-540	0
33	5070	-800	-360	0
34	5680	-800	-360	0
35	5070	-400	-360	0
36	5680	-400	-360	0
37	150	0	-360	0
38	1080	0	-360	0
39	1950	0	-360	0
40	2820	0	-360	0
41	3690	0	-360	0
42	4380	0	-360	0
43	5070	0	-360	0
44	5680	0	-360	0
45	150	440	-360	0
46	1080	440	-360	0
47	1950	440	-360	0
48	2820	440	-360	0
49	3690	440	-360	0
50	4380	440	-360	0
51	5070	440	-360	0
52	5680	440	-360	0
53	5070	800	-360	0

54	5680	800	-360	0
55	150	890	-360	0
56	1080	890	-360	0
57	1950	890	-360	0
58	2820	890	-360	0
59	3690	890	-360	0
60	4380	890	-360	0
61	5070	890	-360	0
62	150	1340	-360	0
63	1080	1340	-360	0
64	1950	1340	-360	0
65	2820	1340	-360	0
66	3690	1340	-360	0
67	4380	1340	-360	0
68	5070	1340	-360	0
69	150	1790	-360	0
70	1080	1790	-360	0
71	1950	1790	-360	0
72	2820	1790	-360	0
73	3690	1790	-360	0
74	0	2240	-360	0
75	150	2240	-360	0
76	1080	2240	-360	0
77	1950	2240	-360	0
78	2820	2240	-360	0
79	3690	2240	-360	0
80	0	2660	-360	0
81	1080	2660	-360	0
82	1950	2660	-360	0
83	5070	-800	0	0
84	5680	-800	0	0
85	150	0	0	0
86	1080	0	0	0
87	1950	0	0	0
88	2820	0	0	0
89	3690	0	0	0
90	4035	0	0	0
91	4380	0	0	0
92	4725	0	0	0
93	5070	0	0	0
94	5680	0	0	0
95	150	440	0	0
96	1080	440	0	0
97	1950	440	0	0
98	2820	440	0	0
99	3690	440	0	0
100	4035	440	0	0
101	4380	440	0	0
102	4725	440	0	0
103	5070	440	0	0
104	5680	440	0	0
105	5680	800	0	0
106	150	890	0	0
107	1080	890	0	0
108	1950	890	0	0
109	2820	890	0	0
110	3690	890	0	0
111	4035	890	0	0
112	4380	890	0	0

113	4725	890	0	0
114	5070	890	0	0
115	150	1340	0	0
116	1080	1340	0	0
117	1950	1340	0	0
118	2820	1340	0	0
119	3690	1340	0	0
120	4035	1340	0	0
121	4380	1340	0	0
122	4725	1340	0	0
123	5070	1340	0	0
124	150	1790	0	0
125	1080	1790	0	0
126	1950	1790	0	0
127	2820	1790	0	0
128	3690	1790	0	0
129	0	2240	0	0
130	150	2240	0	0
131	1080	2240	0	0
132	1950	2240	0	0
133	2820	2240	0	0
134	3690	2240	0	0
135	0	2660	0	0
136	1080	2660	0	0
137	1950	2660	0	0
138	150	0	435	0
139	1080	0	435	0
140	1950	0	435	0
141	150	440	435	0
142	1080	440	435	0
143	1950	440	435	0
144	2820	440	435	0
145	3690	440	435	0
146	150	890	435	0
147	1080	890	435	0
148	1950	890	435	0
149	2820	890	435	0
150	3690	890	435	0
151	150	1340	435	0
152	1080	1340	435	0
153	1950	1340	435	0
154	2820	1340	435	0
155	3690	1340	435	0
156	150	1790	435	0
157	1080	1790	435	0
158	1950	1790	435	0
159	2820	1790	435	0
160	3690	1790	435	0
161	0	2240	435	0
162	150	2240	435	0
163	1080	2240	435	0
164	1950	2240	435	0
165	2820	2240	435	0
166	3690	2240	435	0
167	0	2660	435	0
168	1080	2660	435	0
169	1950	2660	435	0
170	150	0	835	0
171	1080	0	835	0

172	1950	0	835	0
173	150	440	835	0
174	1080	440	835	0
175	1950	440	835	0
176	2820	440	835	0
177	3690	440	835	0
178	150	890	835	0
179	1080	890	835	0
180	1950	890	835	0
181	2820	890	835	0
182	3690	890	835	0
183	150	1340	835	0
184	1080	1340	835	0
185	1950	1340	835	0
186	2820	1340	835	0
187	3690	1340	835	0
188	150	1790	835	0
189	1080	1790	835	0
190	1950	1790	835	0
191	2820	1790	835	0
192	3690	1790	835	0
193	0	2240	835	0
194	150	2240	835	0
195	1080	2240	835	0
196	1950	2240	835	0
197	2820	2240	835	0
198	3690	2240	835	0
199	0	2660	835	0
200	1080	2660	835	0
201	1950	2660	835	0
202	150	0	1235	0
203	1080	0	1235	0
204	1950	0	1235	0
205	150	440	1235	0
206	1080	440	1235	0
207	1950	440	1235	0
208	2820	440	1235	0
209	3690	440	1235	0
210	1080	690	1235	0
211	150	890	1235	0
212	1080	890	1235	0
213	1950	890	1235	0
214	2820	890	1235	0
215	3690	890	1235	0
216	150	1340	1235	0
217	1080	1340	1235	0
218	1950	1340	1235	0
219	2820	1340	1235	0
220	3690	1340	1235	0
221	150	1790	1235	0
222	1080	1790	1235	0
223	1950	1790	1235	0
224	2820	1790	1235	0
225	3690	1790	1235	0
226	1080	1990	1235	0
227	0	2240	1235	0
228	150	2240	1235	0
229	1080	2240	1235	0
230	1950	2240	1235	0

231	2820	2240	1235	0
232	3690	2240	1235	0
233	0	2660	1235	0
234	1080	2660	1235	0
235	1950	2660	1235	0
236	150	0	1635	0
237	575	0	1635	0
238	1080	0	1635	0
239	1950	0	1635	0
240	150	440	1635	0
241	575	440	1635	0
242	1080	440	1635	0
243	1950	440	1635	0
244	2820	440	1635	0
245	3690	440	1635	0
246	1080	690	1635	0
247	150	890	1635	0
248	575	890	1635	0
249	150	1340	1635	0
250	575	1340	1635	0
251	1080	1340	1635	0
252	3690	1340	1635	0
253	150	1790	1635	0
254	575	1790	1635	0
255	1080	1990	1635	0
256	0	2240	1635	0
257	150	2240	1635	0
258	575	2240	1635	0
259	780	2240	1635	0
260	1080	2240	1635	0
261	1950	2240	1635	0
262	2820	2240	1635	0
263	3690	2240	1635	0
264	0	2660	1635	0
265	780	2660	1635	0
266	1080	2660	1635	0
267	1950	2660	1635	0
268	1080	440	1755	0
269	1950	440	1755	0
270	2820	440	1755	0
271	3690	440	1755	0
272	1080	2240	1755	0
273	1950	2240	1755	0
274	2820	2240	1755	0
275	3690	2240	1755	0
276	1080	664.4	1777	0
277	1950	664.4	1777	0
278	2820	664.4	1777	0
279	3690	664.4	1777	0
280	1080	2016	1777	0
281	1950	2016	1777	0
282	2820	2016	1777	0
283	3690	2016	1777	0
284	1080	889.3	1792	0
285	1950	889.3	1792	0
286	2820	889.3	1792	0
287	3690	889.3	1792	0
288	1080	1791	1792	0
289	1950	1791	1792	0

290	2820	1791	1792	0
291	3690	1791	1792	0
292	1080	1115	1802	0
293	1950	1115	1802	0
294	2820	1115	1802	0
295	3690	1115	1802	0
296	1080	1565	1802	0
297	1950	1565	1802	0
298	2820	1565	1802	0
299	3690	1565	1802	0
300	1080	1340	1805	0
301	1950	1340	1805	0
302	2820	1340	1805	0
303	3690	1340	1805	0
304	150	0	2015	0
305	575	0	2015	0
306	150	440	2015	0
307	575	440	2015	0
308	0	2240	2015	0
309	780	2240	2015	0
310	1080	2240	2015	0
311	1370	2240	2015	0
312	0	2660	2015	0
313	780	2660	2015	0
314	1080	2660	2015	0
315	1370	2660	2015	0

*** SUPPORT / SPECIFIED DISPLACEMENT / POINT SPRING SUPPORT

** SUPPORT / SPECIFIED DISPLACEMENT

NODE	SUPPORT	SPECIFIED DISPLACEMENT					
	DDDRRR	Dx	Dy	Dz	Rx	Ry	Rz
1	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
4	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
5	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
6	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
7	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
8	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
9	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
10	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
11	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
12	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
13	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
14	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
15	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
16	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
17	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
18	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
19	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
20	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
21	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
22	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
23	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
24	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
25	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

26	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
27	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
28	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
29	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
30	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
31	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
32	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

*** FLOOR DIAPHRAGM / RIGID LINK DATA

MASTER	DDDRRR	NODES OF SAME DISPLACEMENT
PH	Floor Diaphragm	304to315
RF-1	Floor Diaphragm	236to243 246to251 253to261 264to267
4F	Floor Diaphragm	202to235
3F	Floor Diaphragm	170to201
2F	Floor Diaphragm	138to169

*** SECTION PROPERTY DATA

NO	NAME	SHAPE	H	B	tw	tf1	r1
101	1C1	SB	80	70	0	0	0
102	1C2	SB	80	70	0	0	0
103	1C3	SB	100	100	0	0	0
104	1C4	SB	100	100	0	0	0
105	1C5	SB	100	100	0	0	0
106	1C6	SB	100	100	0	0	0
107	1C7	SB	100	100	0	0	0
108	1C8	SB	100	100	0	0	0
109	1C9	SB	100	100	0	0	0
110	1C10	SB	150	90	0	0	0
111	2C1	SB	80	70	0	0	0
112	2C2	SB	80	70	0	0	0
113	2C3	SB	100	100	0	0	0
114	2C4	SB	100	100	0	0	0
115	2C5	SB	100	100	0	0	0
116	2C6	SB	100	100	0	0	0
117	2C7	SB	100	100	0	0	0
118	2C8	SB	100	100	0	0	0
119	2C9	SB	100	100	0	0	0
120	2C10	SB	150	90	0	0	0
121	3C1	SB	80	70	0	0	0
122	3C2	SB	80	70	0	0	0
123	3C3	SB	100	100	0	0	0
124	3C4	SB	100	100	0	0	0
125	3C5	SB	100	100	0	0	0
126	3C6	SB	100	100	0	0	0
127	3C7	SB	100	100	0	0	0
128	3C8	SB	100	100	0	0	0
129	3C9	SB	100	100	0	0	0
130	3C10	SB	150	90	0	0	0
131	4C1	SB	80	70	0	0	0
132	4C2	SB	80	70	0	0	0
133	4C3	SB	100	100	0	0	0
134	4C4	SB	100	100	0	0	0
135	4C5	SB	100	100	0	0	0
136	4C6	SB	100	100	0	0	0

137	4C7	SB	100	100	0	0	0
138	4C8	SB	100	100	0	0	0
139	4C9	SB	100	100	0	0	0
140	4C10	SB	150	90	0	0	0
141	BC1	SB	80	70	0	0	0
142	BC2	SB	80	70	0	0	0
143	BC3	SB	100	100	0	0	0
144	BC4	SB	100	100	0	0	0
145	BC5	SB	100	100	0	0	0
146	BC6	SB	100	100	0	0	0
147	BC7	SB	100	100	0	0	0
148	BC8	SB	100	100	0	0	0
149	BC9	SB	100	100	0	0	0
150	BC10	SB	150	90	0	0	0
151	BC11	SB	80	70	0	0	0
152	BC12	SB	60	60	0	0	0
153	BC13	SB	80	80	0	0	0
154	BC14	SB	60	60	0	0	0
155	BC15	SB	70	60	0	0	0
156	BC16	SB	80	80	0	0	0
161	PC1	SB	80	70	0	0	0
162	PC2	SB	80	70	0	0	0
163	PC3	SB	100	100	0	0	0
164	PC4	SB	100	100	0	0	0
165	PC10	SB	150	90	0	0	0
166	PC0	SB	50	40	0	0	0
171	4C11	B	40	40	1.6	1.6	0
201	FG1	SB	160	70	0	0	0
202	FG2	SB	160	70	0	0	0
203	FG3	SB	160	80	0	0	0
204	FG4	SB	160	80	0	0	0
205	FG5	SB	160	80	0	0	0
206	FG6	SB	160	100	0	0	0
207	FG7	SB	160	100	0	0	0
208	FG8	SB	160	100	0	0	0
209	FG9	SB	160	70	0	0	0
210	FG10	SB	160	70	0	0	0
211	FG11	SB	160	70	0	0	0
212	FG12	SB	160	60	0	0	0
213	FG13	SB	160	60	0	0	0
214	FG14	SB	160	80	0	0	0
215	FG15	SB	160	60	0	0	0
216	FG16	SB	160	60	0	0	0
251	FB1	SB	160	80	0	0	0
252	FB2	SB	160	80	0	0	0
253	FB3	SB	160	80	0	0	0
254	FB4	SB	160	80	0	0	0
255	FB5	SB	160	80	0	0	0
256	FB6	SB	160	100	0	0	0
257	FB7	SB	160	100	0	0	0
258	FB8	SB	160	100	0	0	0
259	FB9	SB	160	80	0	0	0
260	FB10	SB	160	60	0	0	0
261	FB11	SB	160	60	0	0	0
262	FB12	SB	160	60	0	0	0
263	FB13	SB	160	60	0	0	0
264	FB14	SB	160	80	0	0	0
265	Fb1	SB	160	60	0	0	0
266	Fb2	SB	160	60	0	0	0

301	1G1	SB	80	50	0	0	0
302	1G2	SB	80	50	0	0	0
303	1G3	SB	80	50	0	0	0
304	1G4	SB	80	50	0	0	0
305	1G5	SB	80	50	0	0	0
306	1G6	SB	80	50	0	0	0
307	1G7	SB	80	50	0	0	0
308	1G8	SB	80	50	0	0	0
309	1G9	SB	80	50	0	0	0
310	1G10	SB	80	50	0	0	0
311	1G11	SB	80	50	0	0	0
312	1G12	SB	80	50	0	0	0
313	1G13	SB	60	45	0	0	0
314	1G14	SB	90	60	0	0	0
315	1G15	SB	60	45	0	0	0
316	1G16	SB	60	45	0	0	0
351	1B1	SB	80	50	0	0	0
352	1B2	SB	80	50	0	0	0
353	1B3	SB	80	50	0	0	0
354	1B4	SB	60	45	0	0	0
355	1B5	SB	80	50	0	0	0
356	1B6	SB	80	50	0	0	0
357	1B7	SB	80	50	0	0	0
358	1B8	SB	80	50	0	0	0
359	1B9	SB	60	45	0	0	0
360	1B10	SB	60	45	0	0	0
361	1B11	SB	70	50	0	0	0
362	1B12	SB	70	50	0	0	0
363	1B13	SB	70	50	0	0	0
364	1B14	SB	90	60	0	0	0
401	2G1	SB	80	50	0	0	0
402	2G2	SB	80	50	0	0	0
403	2G3	SB	80	50	0	0	0
404	2G4	SB	80	50	0	0	0
405	2G5	SB	80	50	0	0	0
406	2G6	SB	90	65	0	0	0
407	2G7	SB	90	65	0	0	0
408	2G8	SB	90	65	0	0	0
409	2G9	SB	80	50	0	0	0
410	2G10	SB	80	50	0	0	0
451	2B1	SB	60	60	0	0	0
452	2B2	SB	80	50	0	0	0
453	2B3	SB	80	50	0	0	0
454	2B4	SB	60	60	0	0	0
455	2B5	SB	60	60	0	0	0
456	2B6	SB	90	65	0	0	0
457	2B7	SB	90	65	0	0	0
458	2B8	SB	80	50	0	0	0
501	3G1	SB	80	50	0	0	0
502	3G2	SB	80	50	0	0	0
503	3G3	SB	80	50	0	0	0
504	3G4	SB	80	50	0	0	0
505	3G5	SB	80	50	0	0	0
506	3G6	SB	90	60	0	0	0
507	3G7	SB	90	60	0	0	0
508	3G8	SB	90	60	0	0	0
509	3G9	SB	80	50	0	0	0
510	3G10	SB	80	50	0	0	0
511	3G11	SB	80	50	0	0	0

551	3B1	SB	70	50	0	0	0
552	3B2	SB	80	50	0	0	0
553	3B3	SB	80	50	0	0	0
554	3B4	SB	60	50	0	0	0
555	3B5	SB	70	50	0	0	0
556	3B6	SB	90	65	0	0	0
557	3B7	SB	90	65	0	0	0
558	3B8	SB	80	50	0	0	0
601	4G1	SB	80	50	0	0	0
602	4G2	SB	80	50	0	0	0
603	4G3	SB	80	50	0	0	0
604	4G4	SB	80	50	0	0	0
605	4G5	SB	80	50	0	0	0
606	4G6	SB	90	60	0	0	0
607	4G7	SB	90	60	0	0	0
608	4G8	SB	90	60	0	0	0
609	4G9	SB	80	50	0	0	0
610	4G10	SB	80	50	0	0	0
611	4G11	SB	80	50	0	0	0
651	4B1	SB	70	50	0	0	0
652	4B2	SB	80	50	0	0	0
653	4B3	SB	80	50	0	0	0
654	4B4	SB	60	45	0	0	0
655	4B5	SB	70	50	0	0	0
656	4B6	SB	90	60	0	0	0
657	4B7	SB	90	60	0	0	0
658	4B8	SB	80	50	0	0	0
701	RG1	SB	70	50	0	0	0
702	RG2	SB	70	50	0	0	0
703	RG3	SB	80	50	0	0	0
704	RG4	SB	70	50	0	0	0
705	RG5	SB	70	50	0	0	0
706	RG6	SB	80	50	0	0	0
707	RG7	SB	80	50	0	0	0
708	RG8	SB	80	50	0	0	0
709	RG9	SB	70	50	0	0	0
710	RG10	SB	70	50	0	0	0
711	RG11	SB	80	50	0	0	0
751	RB1	SB	70	50	0	0	0
752	RB2	SB	70	50	0	0	0
753	RB3	SB	70	50	0	0	0
754	RB4	SB	60	45	0	0	0
755	RB5	SB	70	50	0	0	0
756	RB6	SB	85	50	0	0	0
757	RB7	SB	85	50	0	0	0
758	RB8	SB	70	50	0	0	0
801	PG1	SB	50	35	0	0	0
802	PG2	SB	60	35	0	0	0
803	PG3	SB	60	35	0	0	0
804	PCG3	SB	50	35	0	0	0
811	PB1	SB	50	35	0	0	0
812	PB2	SB	50	35	0	0	0
813	Pb1	SB	50	30	0	0	0
1001	b1a	SB	55	35	0	0	0
1002	b0	SB	50	30	0	0	0
1003	b3	SB	60	35	0	0	0
1004	b4	SB	50	35	0	0	0
1005	b1	SB	65	40	0	0	0
1006	4b2	SB	65	45	0	0	0

1007	2b2	SB	70	45	0	0	0
1008	1b1	SB	65	45	0	0	0
1009	1b2	SB	65	45	0	0	0
3001	RSB1	H	70	30	1.3	2.4	1.8
3002	RSB2	H	58.8	30	1.2	2	1.3
3101	RSG1	H	39	30	1	1.6	1.3
3151	RSC1	H	50	50	1.6	2.5	0
3152	RSC2	H	45	45	1.2	1.6	0

NO	NAME	AREA	MOMENT OF INERTIA			SHAPE FACTOR	
		[SRC:EQIV.]	Ix	Iy	Iz	k-Y	k-Z
101	1C1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
102	1C2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
103	1C3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
104	1C4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
105	1C5	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
106	1C6	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
107	1C7	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
108	1C8	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
109	1C9	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
110	1C10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
111	2C1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
112	2C2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
113	2C3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
114	2C4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
115	2C5	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
116	2C6	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
117	2C7	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
118	2C8	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
119	2C9	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
120	2C10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
121	3C1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
122	3C2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
123	3C3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
124	3C4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
125	3C5	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
126	3C6	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
127	3C7	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
128	3C8	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
129	3C9	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
130	3C10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
131	4C1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
132	4C2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
133	4C3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
134	4C4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
135	4C5	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
136	4C6	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
137	4C7	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
138	4C8	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
139	4C9	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
140	4C10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
141	BC1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
142	BC2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
143	BC3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
144	BC4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
145	BC5	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
146	BC6	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333

147	BC7	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
148	BC8	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
149	BC9	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
150	BC10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
151	BC11	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
152	BC12	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
153	BC13	6400	5.76e+006	3.413e+006	3.413e+006	0.8333	0.8333
154	BC14	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
155	BC15	4200	2.441e+006	1.715e+006	1.26e+006	0.8333	0.8333
156	BC16	6400	5.76e+006	3.413e+006	3.413e+006	0.8333	0.8333
161	PC1	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
162	PC2	5600	4.351e+006	2.987e+006	2.287e+006	0.8333	0.8333
163	PC3	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
164	PC4	1e+004	1.406e+007	8.333e+006	8.333e+006	0.8333	0.8333
165	PC10	1.35e+004	2.282e+007	2.531e+007	9.113e+006	0.8333	0.8333
166	PC0	2000	5.474e+005	4.167e+005	2.667e+005	0.8333	0.8333
171	4C11	246	9.06e+004	6.05e+004	6.05e+004	0.5203	0.5203
201	FG1	1.12e+004	1.327e+007	2.389e+007	4.573e+006	0.8333	0.8333
202	FG2	1.12e+004	1.327e+007	2.389e+007	4.573e+006	0.8333	0.8333
203	FG3	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
204	FG4	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
205	FG5	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
206	FG6	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
207	FG7	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
208	FG8	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
209	FG9	1.12e+004	1.327e+007	2.389e+007	4.573e+006	0.8333	0.8333
210	FG10	1.12e+004	1.327e+007	2.389e+007	4.573e+006	0.8333	0.8333
211	FG11	1.12e+004	1.327e+007	2.389e+007	4.573e+006	0.8333	0.8333
212	FG12	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
213	FG13	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
214	FG14	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
215	FG15	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
216	FG16	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
251	FB1	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
252	FB2	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
253	FB3	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
254	FB4	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
255	FB5	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
256	FB6	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
257	FB7	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
258	FB8	1.6e+004	3.26e+007	3.413e+007	1.333e+007	0.8333	0.8333
259	FB9	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
260	FB10	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
261	FB11	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
262	FB12	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
263	FB13	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
264	FB14	1.28e+004	1.875e+007	2.731e+007	6.827e+006	0.8333	0.8333
265	Fb1	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
266	Fb2	9600	8.803e+006	2.048e+007	2.88e+006	0.8333	0.8333
301	1G1	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
302	1G2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
303	1G3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
304	1G4	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
305	1G5	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
306	1G6	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
307	1G7	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
308	1G8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
309	1G9	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
310	1G10	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333

311	1G11	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
312	1G12	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
313	1G13	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
314	1G14	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
315	1G15	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
316	1G16	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
351	1B1	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
352	1B2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
353	1B3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
354	1B4	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
355	1B5	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
356	1B6	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
357	1B7	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
358	1B8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
359	1B9	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
360	1B10	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
361	1B11	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
362	1B12	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
363	1B13	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
364	1B14	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
401	2G1	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
402	2G2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
403	2G3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
404	2G4	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
405	2G5	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
406	2G6	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
407	2G7	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
408	2G8	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
409	2G9	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
410	2G10	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
451	2B1	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
452	2B2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
453	2B3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
454	2B4	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
455	2B5	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
456	2B6	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
457	2B7	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
458	2B8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
501	3G1	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
502	3G2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
503	3G3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
504	3G4	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
505	3G5	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
506	3G6	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
507	3G7	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
508	3G8	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
509	3G9	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
510	3G10	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
511	3G11	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
551	3B1	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
552	3B2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
553	3B3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
554	3B4	3000	1.24e+006	9e+005	6.25e+005	0.8333	0.8333
555	3B5	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
556	3B6	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
557	3B7	5850	4.575e+006	3.949e+006	2.06e+006	0.8333	0.8333
558	3B8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
601	4G1	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
602	4G2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333

603	4G3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
604	4G4	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
605	4G5	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
606	4G6	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
607	4G7	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
608	4G8	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
609	4G9	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
610	4G10	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
611	4G11	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
651	4B1	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
652	4B2	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
653	4B3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
654	4B4	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
655	4B5	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
656	4B6	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
657	4B7	5400	3.803e+006	3.645e+006	1.62e+006	0.8333	0.8333
658	4B8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
701	RG1	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
702	RG2	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
703	RG3	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
704	RG4	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
705	RG5	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
706	RG6	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
707	RG7	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
708	RG8	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
709	RG9	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
710	RG10	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
711	RG11	4000	2.038e+006	2.133e+006	8.333e+005	0.8333	0.8333
751	RB1	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
752	RB2	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
753	RB3	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
754	RB4	2700	9.841e+005	8.1e+005	4.556e+005	0.8333	0.8333
755	RB5	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
756	RB6	4250	2.242e+006	2.559e+006	8.854e+005	0.8333	0.8333
757	RB7	4250	2.242e+006	2.559e+006	8.854e+005	0.8333	0.8333
758	RB8	3500	1.633e+006	1.429e+006	7.292e+005	0.8333	0.8333
801	PG1	1750	4.058e+005	3.646e+005	1.786e+005	0.8333	0.8333
802	PG2	2100	5.454e+005	6.3e+005	2.144e+005	0.8333	0.8333
803	PG3	2100	5.454e+005	6.3e+005	2.144e+005	0.8333	0.8333
804	PCG3	1750	4.058e+005	3.646e+005	1.786e+005	0.8333	0.8333
811	PB1	1750	4.058e+005	3.646e+005	1.786e+005	0.8333	0.8333
812	PB2	1750	4.058e+005	3.646e+005	1.786e+005	0.8333	0.8333
813	Pb1	1500	2.817e+005	3.125e+005	1.125e+005	0.8333	0.8333
1001	b1a	1925	4.752e+005	4.853e+005	1.965e+005	0.8333	0.8333
1002	b0	1500	2.817e+005	3.125e+005	1.125e+005	0.8333	0.8333
1003	b3	2100	5.454e+005	6.3e+005	2.144e+005	0.8333	0.8333
1004	b4	1750	4.058e+005	3.646e+005	1.786e+005	0.8333	0.8333
1005	b1	2600	8.555e+005	9.154e+005	3.467e+005	0.8333	0.8333
1006	4b2	2925	1.13e+006	1.03e+006	4.936e+005	0.8333	0.8333
1007	2b2	3150	1.277e+006	1.286e+006	5.316e+005	0.8333	0.8333
1008	1b1	2925	1.13e+006	1.03e+006	4.936e+005	0.8333	0.8333
1009	1b2	2925	1.13e+006	1.03e+006	4.936e+005	0.8333	0.8333
3001	RSB1	232	326	1.97e+005	1.08e+004	0.5172	0.3922
3002	RSB2	187	192.7	1.14e+005	9010	0.5348	0.3773
3101	RSG1	133	94.39	3.79e+004	7200	0.6015	0.2932
3151	RSC1	322	585.7	1.533e+005	5.21e+004	0.647	0.2484
3152	RSC2	194.2	147.9	7.514e+004	2.431e+004	0.618	0.2781

NO	NAME	SECTION MODULUS Sy	SECTION MODULUS Sz

		I or CONC.	J or STEEL	I or CONC.	J or STEEL
101	1C1	7.467e+004	7.467e+004	6.533e+004	6.533e+004
102	1C2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
103	1C3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
104	1C4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
105	1C5	1.667e+005	1.667e+005	1.667e+005	1.667e+005
106	1C6	1.667e+005	1.667e+005	1.667e+005	1.667e+005
107	1C7	1.667e+005	1.667e+005	1.667e+005	1.667e+005
108	1C8	1.667e+005	1.667e+005	1.667e+005	1.667e+005
109	1C9	1.667e+005	1.667e+005	1.667e+005	1.667e+005
110	1C10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
111	2C1	7.467e+004	7.467e+004	6.533e+004	6.533e+004
112	2C2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
113	2C3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
114	2C4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
115	2C5	1.667e+005	1.667e+005	1.667e+005	1.667e+005
116	2C6	1.667e+005	1.667e+005	1.667e+005	1.667e+005
117	2C7	1.667e+005	1.667e+005	1.667e+005	1.667e+005
118	2C8	1.667e+005	1.667e+005	1.667e+005	1.667e+005
119	2C9	1.667e+005	1.667e+005	1.667e+005	1.667e+005
120	2C10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
121	3C1	7.467e+004	7.467e+004	6.533e+004	6.533e+004
122	3C2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
123	3C3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
124	3C4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
125	3C5	1.667e+005	1.667e+005	1.667e+005	1.667e+005
126	3C6	1.667e+005	1.667e+005	1.667e+005	1.667e+005
127	3C7	1.667e+005	1.667e+005	1.667e+005	1.667e+005
128	3C8	1.667e+005	1.667e+005	1.667e+005	1.667e+005
129	3C9	1.667e+005	1.667e+005	1.667e+005	1.667e+005
130	3C10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
131	4C1	7.467e+004	7.467e+004	6.533e+004	6.533e+004
132	4C2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
133	4C3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
134	4C4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
135	4C5	1.667e+005	1.667e+005	1.667e+005	1.667e+005
136	4C6	1.667e+005	1.667e+005	1.667e+005	1.667e+005
137	4C7	1.667e+005	1.667e+005	1.667e+005	1.667e+005
138	4C8	1.667e+005	1.667e+005	1.667e+005	1.667e+005
139	4C9	1.667e+005	1.667e+005	1.667e+005	1.667e+005
140	4C10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
141	BC1	7.467e+004	7.467e+004	6.533e+004	6.533e+004
142	BC2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
143	BC3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
144	BC4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
145	BC5	1.667e+005	1.667e+005	1.667e+005	1.667e+005
146	BC6	1.667e+005	1.667e+005	1.667e+005	1.667e+005
147	BC7	1.667e+005	1.667e+005	1.667e+005	1.667e+005
148	BC8	1.667e+005	1.667e+005	1.667e+005	1.667e+005
149	BC9	1.667e+005	1.667e+005	1.667e+005	1.667e+005
150	BC10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
151	BC11	7.467e+004	7.467e+004	6.533e+004	6.533e+004
152	BC12	3.6e+004	3.6e+004	3.6e+004	3.6e+004
153	BC13	8.533e+004	8.533e+004	8.533e+004	8.533e+004
154	BC14	3.6e+004	3.6e+004	3.6e+004	3.6e+004
155	BC15	4.9e+004	4.9e+004	4.2e+004	4.2e+004
156	BC16	8.533e+004	8.533e+004	8.533e+004	8.533e+004
161	PC1	7.467e+004	7.467e+004	6.533e+004	6.533e+004

162	PC2	7.467e+004	7.467e+004	6.533e+004	6.533e+004
163	PC3	1.667e+005	1.667e+005	1.667e+005	1.667e+005
164	PC4	1.667e+005	1.667e+005	1.667e+005	1.667e+005
165	PC10	3.375e+005	3.375e+005	2.025e+005	2.025e+005
166	PC0	1.667e+004	1.667e+004	1.333e+004	1.333e+004
171	4C11	3030	3030	3030	3030
201	FG1	2.987e+005	2.987e+005	1.307e+005	1.307e+005
202	FG2	2.987e+005	2.987e+005	1.307e+005	1.307e+005
203	FG3	3.413e+005	3.413e+005	1.707e+005	1.707e+005
204	FG4	3.413e+005	3.413e+005	1.707e+005	1.707e+005
205	FG5	3.413e+005	3.413e+005	1.707e+005	1.707e+005
206	FG6	4.267e+005	4.267e+005	2.667e+005	2.667e+005
207	FG7	4.267e+005	4.267e+005	2.667e+005	2.667e+005
208	FG8	4.267e+005	4.267e+005	2.667e+005	2.667e+005
209	FG9	2.987e+005	2.987e+005	1.307e+005	1.307e+005
210	FG10	2.987e+005	2.987e+005	1.307e+005	1.307e+005
211	FG11	2.987e+005	2.987e+005	1.307e+005	1.307e+005
212	FG12	2.56e+005	2.56e+005	9.6e+004	9.6e+004
213	FG13	2.56e+005	2.56e+005	9.6e+004	9.6e+004
214	FG14	3.413e+005	3.413e+005	1.707e+005	1.707e+005
215	FG15	2.56e+005	2.56e+005	9.6e+004	9.6e+004
216	FG16	2.56e+005	2.56e+005	9.6e+004	9.6e+004
251	FB1	3.413e+005	3.413e+005	1.707e+005	1.707e+005
252	FB2	3.413e+005	3.413e+005	1.707e+005	1.707e+005
253	FB3	3.413e+005	3.413e+005	1.707e+005	1.707e+005
254	FB4	3.413e+005	3.413e+005	1.707e+005	1.707e+005
255	FB5	3.413e+005	3.413e+005	1.707e+005	1.707e+005
256	FB6	4.267e+005	4.267e+005	2.667e+005	2.667e+005
257	FB7	4.267e+005	4.267e+005	2.667e+005	2.667e+005
258	FB8	4.267e+005	4.267e+005	2.667e+005	2.667e+005
259	FB9	3.413e+005	3.413e+005	1.707e+005	1.707e+005
260	FB10	2.56e+005	2.56e+005	9.6e+004	9.6e+004
261	FB11	2.56e+005	2.56e+005	9.6e+004	9.6e+004
262	FB12	2.56e+005	2.56e+005	9.6e+004	9.6e+004
263	FB13	2.56e+005	2.56e+005	9.6e+004	9.6e+004
264	FB14	3.413e+005	3.413e+005	1.707e+005	1.707e+005
265	Fb1	2.56e+005	2.56e+005	9.6e+004	9.6e+004
266	Fb2	2.56e+005	2.56e+005	9.6e+004	9.6e+004
301	1G1	5.333e+004	5.333e+004	3.333e+004	3.333e+004
302	1G2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
303	1G3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
304	1G4	5.333e+004	5.333e+004	3.333e+004	3.333e+004
305	1G5	5.333e+004	5.333e+004	3.333e+004	3.333e+004
306	1G6	5.333e+004	5.333e+004	3.333e+004	3.333e+004
307	1G7	5.333e+004	5.333e+004	3.333e+004	3.333e+004
308	1G8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
309	1G9	5.333e+004	5.333e+004	3.333e+004	3.333e+004
310	1G10	5.333e+004	5.333e+004	3.333e+004	3.333e+004
311	1G11	5.333e+004	5.333e+004	3.333e+004	3.333e+004
312	1G12	5.333e+004	5.333e+004	3.333e+004	3.333e+004
313	1G13	2.7e+004	2.7e+004	2.025e+004	2.025e+004
314	1G14	8.1e+004	8.1e+004	5.4e+004	5.4e+004
315	1G15	2.7e+004	2.7e+004	2.025e+004	2.025e+004
316	1G16	2.7e+004	2.7e+004	2.025e+004	2.025e+004
351	1B1	5.333e+004	5.333e+004	3.333e+004	3.333e+004
352	1B2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
353	1B3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
354	1B4	2.7e+004	2.7e+004	2.025e+004	2.025e+004
355	1B5	5.333e+004	5.333e+004	3.333e+004	3.333e+004

356	1B6	5.333e+004	5.333e+004	3.333e+004	3.333e+004
357	1B7	5.333e+004	5.333e+004	3.333e+004	3.333e+004
358	1B8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
359	1B9	2.7e+004	2.7e+004	2.025e+004	2.025e+004
360	1B10	2.7e+004	2.7e+004	2.025e+004	2.025e+004
361	1B11	4.083e+004	4.083e+004	2.917e+004	2.917e+004
362	1B12	4.083e+004	4.083e+004	2.917e+004	2.917e+004
363	1B13	4.083e+004	4.083e+004	2.917e+004	2.917e+004
364	1B14	8.1e+004	8.1e+004	5.4e+004	5.4e+004
401	2G1	5.333e+004	5.333e+004	3.333e+004	3.333e+004
402	2G2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
403	2G3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
404	2G4	5.333e+004	5.333e+004	3.333e+004	3.333e+004
405	2G5	5.333e+004	5.333e+004	3.333e+004	3.333e+004
406	2G6	8.775e+004	8.775e+004	6.338e+004	6.338e+004
407	2G7	8.775e+004	8.775e+004	6.338e+004	6.338e+004
408	2G8	8.775e+004	8.775e+004	6.338e+004	6.338e+004
409	2G9	5.333e+004	5.333e+004	3.333e+004	3.333e+004
410	2G10	5.333e+004	5.333e+004	3.333e+004	3.333e+004
451	2B1	3.6e+004	3.6e+004	3.6e+004	3.6e+004
452	2B2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
453	2B3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
454	2B4	3.6e+004	3.6e+004	3.6e+004	3.6e+004
455	2B5	3.6e+004	3.6e+004	3.6e+004	3.6e+004
456	2B6	8.775e+004	8.775e+004	6.338e+004	6.338e+004
457	2B7	8.775e+004	8.775e+004	6.338e+004	6.338e+004
458	2B8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
501	3G1	5.333e+004	5.333e+004	3.333e+004	3.333e+004
502	3G2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
503	3G3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
504	3G4	5.333e+004	5.333e+004	3.333e+004	3.333e+004
505	3G5	5.333e+004	5.333e+004	3.333e+004	3.333e+004
506	3G6	8.1e+004	8.1e+004	5.4e+004	5.4e+004
507	3G7	8.1e+004	8.1e+004	5.4e+004	5.4e+004
508	3G8	8.1e+004	8.1e+004	5.4e+004	5.4e+004
509	3G9	5.333e+004	5.333e+004	3.333e+004	3.333e+004
510	3G10	5.333e+004	5.333e+004	3.333e+004	3.333e+004
511	3G11	5.333e+004	5.333e+004	3.333e+004	3.333e+004
551	3B1	4.083e+004	4.083e+004	2.917e+004	2.917e+004
552	3B2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
553	3B3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
554	3B4	3e+004	3e+004	2.5e+004	2.5e+004
555	3B5	4.083e+004	4.083e+004	2.917e+004	2.917e+004
556	3B6	8.775e+004	8.775e+004	6.338e+004	6.338e+004
557	3B7	8.775e+004	8.775e+004	6.338e+004	6.338e+004
558	3B8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
601	4G1	5.333e+004	5.333e+004	3.333e+004	3.333e+004
602	4G2	5.333e+004	5.333e+004	3.333e+004	3.333e+004
603	4G3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
604	4G4	5.333e+004	5.333e+004	3.333e+004	3.333e+004
605	4G5	5.333e+004	5.333e+004	3.333e+004	3.333e+004
606	4G6	8.1e+004	8.1e+004	5.4e+004	5.4e+004
607	4G7	8.1e+004	8.1e+004	5.4e+004	5.4e+004
608	4G8	8.1e+004	8.1e+004	5.4e+004	5.4e+004
609	4G9	5.333e+004	5.333e+004	3.333e+004	3.333e+004
610	4G10	5.333e+004	5.333e+004	3.333e+004	3.333e+004
611	4G11	5.333e+004	5.333e+004	3.333e+004	3.333e+004
651	4B1	4.083e+004	4.083e+004	2.917e+004	2.917e+004
652	4B2	5.333e+004	5.333e+004	3.333e+004	3.333e+004

653	4B3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
654	4B4	2.7e+004	2.7e+004	2.025e+004	2.025e+004
655	4B5	4.083e+004	4.083e+004	2.917e+004	2.917e+004
656	4B6	8.1e+004	8.1e+004	5.4e+004	5.4e+004
657	4B7	8.1e+004	8.1e+004	5.4e+004	5.4e+004
658	4B8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
701	RG1	4.083e+004	4.083e+004	2.917e+004	2.917e+004
702	RG2	4.083e+004	4.083e+004	2.917e+004	2.917e+004
703	RG3	5.333e+004	5.333e+004	3.333e+004	3.333e+004
704	RG4	4.083e+004	4.083e+004	2.917e+004	2.917e+004
705	RG5	4.083e+004	4.083e+004	2.917e+004	2.917e+004
706	RG6	5.333e+004	5.333e+004	3.333e+004	3.333e+004
707	RG7	5.333e+004	5.333e+004	3.333e+004	3.333e+004
708	RG8	5.333e+004	5.333e+004	3.333e+004	3.333e+004
709	RG9	4.083e+004	4.083e+004	2.917e+004	2.917e+004
710	RG10	4.083e+004	4.083e+004	2.917e+004	2.917e+004
711	RG11	5.333e+004	5.333e+004	3.333e+004	3.333e+004
751	RB1	4.083e+004	4.083e+004	2.917e+004	2.917e+004
752	RB2	4.083e+004	4.083e+004	2.917e+004	2.917e+004
753	RB3	4.083e+004	4.083e+004	2.917e+004	2.917e+004
754	RB4	2.7e+004	2.7e+004	2.025e+004	2.025e+004
755	RB5	4.083e+004	4.083e+004	2.917e+004	2.917e+004
756	RB6	6.021e+004	6.021e+004	3.542e+004	3.542e+004
757	RB7	6.021e+004	6.021e+004	3.542e+004	3.542e+004
758	RB8	4.083e+004	4.083e+004	2.917e+004	2.917e+004
801	PG1	1.458e+004	1.458e+004	1.021e+004	1.021e+004
802	PG2	2.1e+004	2.1e+004	1.225e+004	1.225e+004
803	PG3	2.1e+004	2.1e+004	1.225e+004	1.225e+004
804	PCG3	1.458e+004	1.458e+004	1.021e+004	1.021e+004
811	PB1	1.458e+004	1.458e+004	1.021e+004	1.021e+004
812	PB2	1.458e+004	1.458e+004	1.021e+004	1.021e+004
813	Pb1	1.25e+004	1.25e+004	7500	7500
1001	b1a	1.765e+004	1.765e+004	1.123e+004	1.123e+004
1002	b0	1.25e+004	1.25e+004	7500	7500
1003	b3	2.1e+004	2.1e+004	1.225e+004	1.225e+004
1004	b4	1.458e+004	1.458e+004	1.021e+004	1.021e+004
1005	b1	2.817e+004	2.817e+004	1.733e+004	1.733e+004
1006	4b2	3.169e+004	3.169e+004	2.194e+004	2.194e+004
1007	2b2	3.675e+004	3.675e+004	2.363e+004	2.363e+004
1008	1b1	3.169e+004	3.169e+004	2.194e+004	2.194e+004
1009	1b2	3.169e+004	3.169e+004	2.194e+004	2.194e+004
3001	RSB1	5640	5640	721	721
3002	RSB2	3890	3890	601	601
3101	RSG1	1940	1940	480	480
3151	RSC1	6132	6132	2084	2084
3152	RSC2	3340	3340	1080	1080

*** BEAM MEMBER DATA

NO	NODAL CONNECTIVITY		BEAM END RELEASE		MATERIAL	SECTION	LENGTH
	I	J	I	J			
1	1	33	-	-	C280	BC14	180
2	2	34	-	-	C280	BC14	180
3	3	37	-	-	C280	BC1	180
4	4	38	-	-	C280	BC2	180
5	5	39	-	-	C280	BC1	180
6	6	40	-	-	C280	BC11	180
7	7	41	-	-	C280	BC11	180

8	8	42	-	-	C280	BC16	180
9	9	43	-	-	C280	BC12	180
10	10	44	-	-	C280	BC14	180
11	11	45	-	-	C280	BC3	180
12	12	46	-	-	C280	BC4	180
13	13	47	-	-	C280	BC5	180
14	14	48	-	-	C280	BC5	180
15	15	49	-	-	C280	BC6	180
16	16	51	-	-	C280	BC13	180
17	17	54	-	-	C280	BC14	180
18	18	62	-	-	C280	BC7	180
19	19	63	-	-	C280	BC8	180
20	20	64	-	-	C280	BC8	180
21	21	65	-	-	C280	BC8	180
22	22	66	-	-	C280	BC9	180
23	23	67	-	-	C280	BC16	180
24	24	68	-	-	C280	BC15	180
25	25	74	-	-	C280	BC10	180
26	26	76	-	-	C280	BC4	180
27	27	77	-	-	C280	BC5	180
28	28	78	-	-	C280	BC5	180
29	29	79	-	-	C280	BC6	180
30	30	80	-	-	C280	BC1	180
31	31	81	-	-	C280	BC2	180
32	32	82	-	-	C280	BC1	180
33	33	34	-	-	C280	FG16	610
34	35	33	-	-	C280	FB13	400
35	36	34	-	-	C280	FB13	400
36	35	36	-	-	C280	Fb2	610
37	43	35	-	-	C280	FB13	400
38	44	36	-	-	C280	FB13	400
39	37	38	-	-	C280	FG1	930
40	38	39	-	-	C280	FG2	870
41	39	40	-	-	C280	FG11	870
42	40	41	-	-	C280	FG11	870
43	41	42	-	-	C280	FG12	690
44	42	43	-	-	C280	FG12	690
45	43	44	-	-	C280	FG13	610
46	37	45	-	-	C280	FB1	440
47	38	46	-	-	C280	FB5	440
48	39	47	-	-	C280	FB5	440
49	40	48	-	-	C280	FB9	440
50	41	49	-	-	C280	FB9	440
51	42	50	-	-	C280	FB14	440
52	51	43	-	-	C280	FB10	440
53	52	44	-	-	C280	FB12	440
54	45	46	-	-	C280	FG3	930
55	46	47	-	-	C280	FG4	870
56	47	48	-	-	C280	FG4	870
57	48	49	-	-	C280	FG5	870
58	49	50	-	-	C280	FG14	690
59	50	51	-	-	C280	FG14	690
60	51	52	-	-	C280	FG15	610
61	51	53	-	-	C280	FB11	360
62	54	52	-	-	C280	FB12	360
63	45	55	-	-	C280	FB2	450
64	46	56	-	-	C280	FB6	450
65	47	57	-	-	C280	FB6	450
66	48	58	-	-	C280	FB7	450

67	49	59	-	-	C280	FB8	450
68	50	60	-	-	C280	FB14	450
69	53	54	-	-	C280	Fb2	610
70	53	61	-	-	C280	FB11	90
71	55	56	-	-	C280	Fb1	930
72	56	57	-	-	C280	Fb1	870
73	57	58	-	-	C280	Fb1	870
74	58	59	-	-	C280	Fb1	870
75	59	60	-	-	C280	Fb2	690
76	60	61	-	-	C280	Fb2	690
77	68	54	-	-	C280	FB12	814.7
78	55	62	-	-	C280	FB2	450
79	56	63	-	-	C280	FB6	450
80	57	64	-	-	C280	FB6	450
81	58	65	-	-	C280	FB7	450
82	59	66	-	-	C280	FB8	450
83	60	67	-	-	C280	FB14	450
84	61	68	-	-	C280	FB11	450
85	62	63	-	-	C280	FG6	930
86	63	64	-	-	C280	FG7	870
87	64	65	-	-	C280	FG7	870
88	65	66	-	-	C280	FG8	870
89	66	67	-	-	C280	FG12	690
90	67	68	-	-	C280	FG12	690
91	62	69	-	-	C280	FB3	450
92	63	70	-	-	C280	FB6	450
93	64	71	-	-	C280	FB6	450
94	65	72	-	-	C280	FB7	450
95	66	73	-	-	C280	FB8	450
96	69	70	-	-	C280	Fb1	930
97	70	71	-	-	C280	Fb1	870
98	71	72	-	-	C280	Fb1	870
99	72	73	-	-	C280	Fb1	870
100	69	75	-	-	C280	FB3	450
101	70	76	-	-	C280	FB6	450
102	71	77	-	-	C280	FB6	450
103	72	78	-	-	C280	FB7	450
104	73	79	-	-	C280	FB8	450
105	74	75	-	-	C280	FG3	150
106	75	76	-	-	C280	FG3	930
107	76	77	-	-	C280	FG4	870
108	77	78	-	-	C280	FG4	870
109	78	79	-	-	C280	FG5	870
110	74	80	-	-	C280	FB4	420
111	76	81	-	-	C280	FB5	420
112	77	82	-	-	C280	FB4	420
113	80	81	-	-	C280	FG9	1080
114	81	82	-	-	C280	FG10	870
115	33	83	-	-	C280	BC14	360
116	34	84	-	-	C280	BC14	360
117	37	85	-	-	C280	BC1	360
118	38	86	-	-	C280	BC2	360
119	39	87	-	-	C280	BC1	360
120	40	88	-	-	C280	BC11	360
121	41	89	-	-	C280	BC11	360
122	42	91	-	-	C280	BC16	360
123	43	93	-	-	C280	BC12	360
124	44	94	-	-	C280	BC14	360
125	45	95	-	-	C280	BC3	360

126	46	96	-	-	C280	BC4	360
127	47	97	-	-	C280	BC5	360
128	48	98	-	-	C280	BC5	360
129	49	99	-	-	C280	BC6	360
130	51	103	-	-	C280	BC13	360
131	54	105	-	-	C280	BC14	360
132	62	115	-	-	C280	BC7	360
133	63	116	-	-	C280	BC8	360
134	64	117	-	-	C280	BC8	360
135	65	118	-	-	C280	BC8	360
136	66	119	-	-	C280	BC9	360
137	67	121	-	-	C280	BC16	360
138	68	123	-	-	C280	BC15	360
139	74	129	-	-	C280	BC10	360
140	76	131	-	-	C280	BC4	360
141	77	132	-	-	C280	BC5	360
142	78	133	-	-	C280	BC5	360
143	79	134	-	-	C280	BC6	360
144	80	135	-	-	C280	BC1	360
145	81	136	-	-	C280	BC2	360
146	82	137	-	-	C280	BC1	360
147	83	84	-	-	C280	1G16	610
148	93	83	-	-	C280	1B13	800
149	94	84	-	-	C280	1B13	800
150	85	86	-	-	C280	1G1	930
151	86	87	-	-	C280	1G2	870
152	87	88	-	-	C280	1G11	870
153	88	89	-	-	C280	1G11	870
154	89	90	-	-	C280	1G12	345
155	90	91	-	-	C280	1G12	345
156	91	92	-	-	C280	1G12	345
157	92	93	-	-	C280	1G12	345
158	93	94	-	-	C280	1G13	610
159	85	95	-	-	C280	1B1	440
160	86	96	-	-	C280	1B5	440
161	87	97	-	-	C280	1B5	440
162	88	98	-	-	C280	1B9	440
163	89	99	-	-	C280	1B9	440
164	100	90	-	-	C280	b4	440
165	101	91	-	-	C280	1B14	440
166	102	92	-	-	C280	b4	440
167	103	93	-	-	C280	1B10	440
168	104	94	-	-	C280	1B12	440
169	95	96	-	-	C280	1G3	930
170	96	97	-	-	C280	1G4	870
171	97	98	-	-	C280	1G4	870
172	98	99	-	-	C280	1G5	870
173	99	100	-	-	C280	1G14	345
174	100	101	-	-	C280	1G14	345
175	101	102	-	-	C280	1G14	345
176	102	103	-	-	C280	1G14	345
177	103	104	-	-	C280	1G15	610
178	105	104	-	-	C280	1B12	360
179	95	106	-	-	C280	1B2	450
180	96	107	-	-	C280	1B6	450
181	97	108	-	-	C280	1B6	450
182	98	109	-	-	C280	1B7	450
183	99	110	-	-	C280	1B8	450
184	111	100	-	-	C280	b4	450

185	112	101	-	-	C280	1B14	450
186	113	102	-	-	C280	b4	450
187	103	114	-	-	C280	1B11	450
188	107	106	000011	000011	C280	1b1	930
189	108	107	000011	000011	C280	1b2	870
190	109	108	000011	000011	C280	1b2	870
191	110	109	000011	000011	C280	1b2	870
192	110	111	-	-	C280	b3	345
193	111	112	-	-	C280	b3	345
194	112	113	-	-	C280	b3	345
195	113	114	-	-	C280	b3	345
196	123	105	-	-	C280	1B12	814.7
197	106	115	-	-	C280	1B2	450
198	107	116	-	-	C280	1B6	450
199	108	117	-	-	C280	1B6	450
200	109	118	-	-	C280	1B7	450
201	110	119	-	-	C280	1B8	450
202	120	111	-	-	C280	b4	450
203	121	112	-	-	C280	1B14	450
204	122	113	-	-	C280	b4	450
205	114	123	-	-	C280	1B11	450
206	115	116	-	-	C280	1G6	930
207	116	117	-	-	C280	1G7	870
208	117	118	-	-	C280	1G7	870
209	118	119	-	-	C280	1G8	870
210	119	120	-	-	C280	1G12	345
211	120	121	-	-	C280	1G12	345
212	121	122	-	-	C280	1G12	345
213	122	123	-	-	C280	1G12	345
214	115	124	-	-	C280	1B3	450
215	116	125	-	-	C280	1B6	450
216	117	126	-	-	C280	1B6	450
217	118	127	-	-	C280	1B7	450
218	119	128	-	-	C280	1B8	450
219	124	125	000011	000011	C280	1b1	930
220	125	126	000011	000011	C280	1b2	870
221	126	127	000011	000011	C280	1b2	870
222	127	128	000011	000011	C280	1b2	870
223	124	130	-	-	C280	1B3	450
224	125	131	-	-	C280	1B6	450
225	126	132	-	-	C280	1B6	450
226	127	133	-	-	C280	1B7	450
227	128	134	-	-	C280	1B8	450
228	129	130	-	-	C280	1G3	150
229	130	131	-	-	C280	1G3	930
230	131	132	-	-	C280	1G4	870
231	132	133	-	-	C280	1G4	870
232	133	134	-	-	C280	1G5	870
233	129	135	-	-	C280	1B4	420
234	131	136	-	-	C280	1B5	420
235	132	137	-	-	C280	1B4	420
236	135	136	-	-	C280	1G9	1080
237	136	137	-	-	C280	1G10	870
238	85	138	-	-	C280	1C1	435
239	86	139	-	-	C280	1C2	435
240	87	140	-	-	C280	1C1	435
241	95	141	-	-	C280	1C3	435
242	96	142	-	-	C280	1C4	435
243	97	143	-	-	C280	1C5	435

244	98	144	-	-	C280	1C5	435
245	99	145	-	-	C280	1C6	435
246	115	151	-	-	C280	1C7	435
247	116	152	-	-	C280	1C8	435
248	117	153	-	-	C280	1C8	435
249	118	154	-	-	C280	1C8	435
250	119	155	-	-	C280	1C9	435
251	129	161	-	-	C280	1C10	435
252	131	163	-	-	C280	1C4	435
253	132	164	-	-	C280	1C5	435
254	133	165	-	-	C280	1C5	435
255	134	166	-	-	C280	1C6	435
256	135	167	-	-	C280	1C1	435
257	136	168	-	-	C280	1C2	435
258	137	169	-	-	C280	1C1	435
259	138	139	-	-	C280	2G1	930
260	139	140	-	-	C280	2G2	870
261	138	141	-	-	C280	2B1	440
262	139	142	-	-	C280	2B5	440
263	140	143	-	-	C280	2B1	440
264	141	142	-	-	C280	2G3	930
265	142	143	-	-	C280	2G4	870
266	143	144	-	-	C280	2G4	870
267	144	145	-	-	C280	2G5	870
268	141	146	-	-	C280	2B2	450
269	142	147	-	-	C280	2B6	450
270	143	148	-	-	C280	2B6	450
271	144	149	-	-	C280	2B7	450
272	145	150	-	-	C280	2B8	450
273	146	147	000011	000011	C280	2b2	930
274	147	148	000011	000011	C280	2b2	870
275	148	149	000011	000011	C280	2b2	870
276	149	150	000011	000011	C280	2b2	870
277	146	151	-	-	C280	2B2	450
278	147	152	-	-	C280	2B6	450
279	148	153	-	-	C280	2B6	450
280	149	154	-	-	C280	2B7	450
281	150	155	-	-	C280	2B8	450
282	151	152	-	-	C280	2G6	930
283	152	153	-	-	C280	2G7	870
284	153	154	-	-	C280	2G7	870
285	154	155	-	-	C280	2G8	870
286	151	156	-	-	C280	2B3	450
287	152	157	-	-	C280	2B6	450
288	153	158	-	-	C280	2B6	450
289	154	159	-	-	C280	2B7	450
290	155	160	-	-	C280	2B8	450
291	156	157	000011	000011	C280	2b2	930
292	157	158	000011	000011	C280	2b2	870
293	158	159	000011	000011	C280	2b2	870
294	159	160	000011	000011	C280	2b2	870
295	156	162	-	-	C280	2B3	450
296	157	163	-	-	C280	2B6	450
297	158	164	-	-	C280	2B6	450
298	159	165	-	-	C280	2B7	450
299	160	166	-	-	C280	2B8	450
300	161	162	-	-	C280	2G3	150
301	162	163	-	-	C280	2G3	930
302	163	164	-	-	C280	2G4	870

303	164	165	-	-	C280	2G4	870
304	165	166	-	-	C280	2G5	870
305	161	167	-	-	C280	2B4	420
306	163	168	-	-	C280	2B5	420
307	164	169	-	-	C280	2B4	420
308	167	168	-	-	C280	2G9	1080
309	168	169	-	-	C280	2G10	870
310	138	170	-	-	C280	2C1	400
311	139	171	-	-	C280	2C2	400
312	140	172	-	-	C280	2C1	400
313	141	173	-	-	C280	2C3	400
314	142	174	-	-	C280	2C4	400
315	143	175	-	-	C280	2C5	400
316	144	176	-	-	C280	2C5	400
317	145	177	-	-	C280	2C6	400
318	151	183	-	-	C280	2C7	400
319	152	184	-	-	C280	2C8	400
320	153	185	-	-	C280	2C8	400
321	154	186	-	-	C280	2C8	400
322	155	187	-	-	C280	2C9	400
323	161	193	-	-	C280	2C10	400
324	163	195	-	-	C280	2C4	400
325	164	196	-	-	C280	2C5	400
326	165	197	-	-	C280	2C5	400
327	166	198	-	-	C280	2C6	400
328	167	199	-	-	C280	2C1	400
329	168	200	-	-	C280	2C2	400
330	169	201	-	-	C280	2C1	400
331	170	171	-	-	C280	3G1	930
332	171	172	-	-	C280	3G2	870
333	170	173	-	-	C280	3B1	440
334	171	174	-	-	C280	3B5	440
335	172	175	-	-	C280	3B1	440
336	173	174	-	-	C280	3G3	930
337	174	175	-	-	C280	3G4	870
338	175	176	-	-	C280	3G4	870
339	176	177	-	-	C280	3G5	870
340	173	178	-	-	C280	3B2	450
341	174	179	-	-	C280	3B6	450
342	175	180	-	-	C280	3B6	450
343	176	181	-	-	C280	3B7	450
344	177	182	-	-	C280	3B8	450
345	178	179	000011	000011	C280	b1	930
346	179	180	000011	000011	C280	b1	870
347	180	181	000011	000011	C280	b1	870
348	181	182	000011	000011	C280	b1	870
349	178	183	-	-	C280	3B2	450
350	179	184	-	-	C280	3B6	450
351	180	185	-	-	C280	3B6	450
352	181	186	-	-	C280	3B7	450
353	182	187	-	-	C280	3B8	450
354	183	184	-	-	C280	3G6	930
355	184	185	-	-	C280	3G7	870
356	185	186	-	-	C280	3G7	870
357	186	187	-	-	C280	3G8	870
358	183	188	-	-	C280	3B3	450
359	184	189	-	-	C280	3B6	450
360	185	190	-	-	C280	3B6	450
361	186	191	-	-	C280	3B7	450

362	187	192	-	-	C280	3B8	450
363	188	189	000011	000011	C280	b1	930
364	189	190	000011	000011	C280	b1	870
365	190	191	000011	000011	C280	b1	870
366	191	192	000011	000011	C280	b1	870
367	188	194	-	-	C280	3B3	450
368	189	195	-	-	C280	3B6	450
369	190	196	-	-	C280	3B6	450
370	191	197	-	-	C280	3B7	450
371	192	198	-	-	C280	3B8	450
372	193	194	-	-	C280	3G11	150
373	194	195	-	-	C280	3G11	930
374	195	196	-	-	C280	3G4	870
375	196	197	-	-	C280	3G4	870
376	197	198	-	-	C280	3G5	870
377	193	199	-	-	C280	3B4	420
378	195	200	-	-	C280	3B5	420
379	196	201	-	-	C280	3B4	420
380	199	200	-	-	C280	3G9	1080
381	200	201	-	-	C280	3G10	870
382	170	202	-	-	C280	3C1	400
383	171	203	-	-	C280	3C2	400
384	172	204	-	-	C280	3C1	400
385	173	205	-	-	C280	3C3	400
386	174	206	-	-	C280	3C4	400
387	175	207	-	-	C280	3C5	400
388	176	208	-	-	C280	3C5	400
389	177	209	-	-	C280	3C6	400
390	183	216	-	-	C280	3C7	400
391	184	217	-	-	C280	3C8	400
392	185	218	-	-	C280	3C8	400
393	186	219	-	-	C280	3C8	400
394	187	220	-	-	C280	3C9	400
395	193	227	-	-	C280	3C10	400
396	195	229	-	-	C280	3C4	400
397	196	230	-	-	C280	3C5	400
398	197	231	-	-	C280	3C5	400
399	198	232	-	-	C280	3C6	400
400	199	233	-	-	C280	3C1	400
401	200	234	-	-	C280	3C2	400
402	201	235	-	-	C280	3C1	400
403	202	203	-	-	C280	4G1	930
404	203	204	-	-	C280	4G2	870
405	202	205	-	-	C280	4B1	440
406	203	206	-	-	C280	4B5	440
407	204	207	-	-	C280	4B1	440
408	205	206	-	-	C280	4G3	930
409	206	207	-	-	C280	4G4	870
410	207	208	-	-	C280	4G4	870
411	208	209	-	-	C280	4G5	870
412	206	210	-	-	C280	4B6	250
413	205	211	-	-	C280	4B2	450
414	207	213	-	-	C280	4B7	450
415	208	214	-	-	C280	4B7	450
416	209	215	-	-	C280	4B8	450
417	210	212	-	-	C280	4B6	200
418	211	212	000011	000011	C280	b1	930
419	212	213	000011	000011	C280	4b2	870
420	213	214	000011	000011	C280	4b2	870

421	214	215	000011	000011	C280	4b2	870
422	211	216	-	-	C280	4B2	450
423	212	217	-	-	C280	4B6	450
424	213	218	-	-	C280	4B7	450
425	214	219	-	-	C280	4B7	450
426	215	220	-	-	C280	4B8	450
427	216	217	-	-	C280	4G6	930
428	217	218	-	-	C280	4G7	870
429	218	219	-	-	C280	4G7	870
430	219	220	-	-	C280	4G8	870
431	216	221	-	-	C280	4B3	450
432	217	222	-	-	C280	4B6	450
433	218	223	-	-	C280	4B7	450
434	219	224	-	-	C280	4B7	450
435	220	225	-	-	C280	4B8	450
436	221	222	000011	000011	C280	b1	930
437	222	223	000011	000011	C280	4b2	870
438	223	224	000011	000011	C280	4b2	870
439	224	225	000011	000011	C280	4b2	870
440	222	226	-	-	C280	4B6	200
441	221	228	-	-	C280	4B3	450
442	223	230	-	-	C280	4B7	450
443	224	231	-	-	C280	4B7	450
444	225	232	-	-	C280	4B8	450
445	226	229	-	-	C280	4B6	250
446	227	228	-	-	C280	4G11	150
447	228	229	-	-	C280	4G11	930
448	229	230	-	-	C280	4G4	870
449	230	231	-	-	C280	4G4	870
450	231	232	-	-	C280	4G5	870
451	227	233	-	-	C280	4B4	420
452	229	234	-	-	C280	4B5	420
453	230	235	-	-	C280	4B4	420
454	233	234	-	-	C280	4G9	1080
455	234	235	-	-	C280	4G10	870
456	202	236	-	-	C280	4C1	400
457	203	238	-	-	C280	4C2	400
458	204	239	-	-	C280	4C1	400
459	205	240	-	-	C280	4C3	400
460	206	242	-	-	C280	4C4	400
461	207	243	-	-	C280	4C5	400
462	208	244	-	-	C280	4C5	400
463	209	245	-	-	C280	4C6	400
464	210	246	000011	000011	STKR400	4C11	400
465	216	249	-	-	C280	4C7	400
466	220	252	-	-	C280	4C9	400
467	226	255	000011	000011	STKR400	4C11	400
468	227	256	-	-	C280	4C10	400
469	229	260	-	-	C280	4C4	400
470	230	261	-	-	C280	4C5	400
471	231	262	-	-	C280	4C5	400
472	232	263	-	-	C280	4C6	400
473	233	264	-	-	C280	4C1	400
474	234	266	-	-	C280	4C2	400
475	235	267	-	-	C280	4C1	400
476	236	237	-	-	C280	RG1	425
477	237	238	-	-	C280	RG1	505
478	238	239	-	-	C280	RG2	870
479	236	240	-	-	C280	RB1	440

480	238	242	-	-	C280	RB5	440
481	239	243	-	-	C280	RB1	440
482	240	241	-	-	C280	RG3	425
483	241	242	-	-	C280	RG3	505
484	242	243	-	-	C280	RG4	870
485	243	244	-	-	C280	RG4	870
486	244	245	-	-	C280	RG5	870
487	242	246	-	-	C280	RB6	250
488	240	247	-	-	C280	RB2	450
489	241	248	-	-	C280	b1a	450
490	247	248	-	-	C280	b0	425
491	245	252	-	-	C280	RB8	900
492	246	251	-	-	C280	RB7	650
493	247	249	-	-	C280	RB2	450
494	248	250	-	-	C280	b1a	450
495	249	250	-	-	C280	RG6	425
496	250	251	-	-	C280	RG6	505
497	249	253	-	-	C280	RB3	450
498	250	254	-	-	C280	b1a	450
499	251	255	-	-	C280	RB7	650
500	253	254	-	-	C280	b0	425
501	252	263	-	-	C280	RB8	900
502	253	257	-	-	C280	RB3	450
503	254	258	-	-	C280	b1a	450
504	255	260	-	-	C280	RB6	250
505	256	257	-	-	C280	RG11	150
506	257	258	-	-	C280	RG11	425
507	258	259	-	-	C280	RG11	205
508	259	260	-	-	C280	RG11	300
509	260	261	-	-	C280	RG4	870
510	261	262	-	-	C280	RG4	870
511	262	263	-	-	C280	RG5	870
512	256	264	-	-	C280	RB4	420
513	260	266	-	-	C280	RB5	420
514	261	267	-	-	C280	RB4	420
515	264	265	-	-	C280	RG9	780
516	265	266	-	-	C280	RG9	300
517	266	267	-	-	C280	RG10	870
518	242	268	-	-	SN400	RSC1	120
519	243	269	-	-	SN400	RSC1	120
520	244	270	-	-	SN400	RSC1	120
521	245	271	-	-	SN400	RSC1	120
522	260	272	-	-	C280	4C4	120
523	261	273	-	-	SN400	RSC1	120
524	262	274	-	-	SN400	RSC1	120
525	263	275	-	-	SN400	RSC1	120
526	300	251	-	-	SN400	RSC2	170
527	303	252	-	-	SN400	RSC2	170
528	268	269	-	-	SS400	RSG1	870
529	269	270	-	-	SS400	RSG1	870
530	270	271	-	-	SS400	RSG1	870
531	272	273	-	-	SS400	RSG1	870
532	273	274	-	-	SS400	RSG1	870
533	274	275	-	-	SS400	RSG1	870
534	276	268	-	-	SN400	RSB2	225.5
535	277	269	-	-	SN400	RSB1	225.5
536	278	270	-	-	SN400	RSB1	225.5
537	279	271	-	-	SN400	RSB2	225.5
538	272	280	-	-	SN400	RSB2	225.5

539	273	281	-	-	SN400	RSB1	225.5
540	274	282	-	-	SN400	RSB1	225.5
541	275	283	-	-	SN400	RSB2	225.5
542	284	276	-	-	SN400	RSB2	225.5
543	285	277	-	-	SN400	RSB1	225.5
544	286	278	-	-	SN400	RSB1	225.5
545	287	279	-	-	SN400	RSB2	225.5
546	280	288	-	-	SN400	RSB2	225.5
547	281	289	-	-	SN400	RSB1	225.5
548	282	290	-	-	SN400	RSB1	225.5
549	283	291	-	-	SN400	RSB2	225.5
550	284	285	000011	000011	SS400	RSG1	870
551	285	286	000011	000011	SS400	RSG1	870
552	286	287	000011	000011	SS400	RSG1	870
553	288	289	000011	000011	SS400	RSG1	870
554	289	290	000011	000011	SS400	RSG1	870
555	290	291	000011	000011	SS400	RSG1	870
556	292	284	-	-	SN400	RSB2	225.5
557	293	285	-	-	SN400	RSB1	225.5
558	294	286	-	-	SN400	RSB1	225.5
559	295	287	-	-	SN400	RSB2	225.5
560	288	296	-	-	SN400	RSB2	225.5
561	289	297	-	-	SN400	RSB1	225.5
562	290	298	-	-	SN400	RSB1	225.5
563	291	299	-	-	SN400	RSB2	225.5
564	300	292	-	-	SN400	RSB2	225.5
565	301	293	-	-	SN400	RSB1	225.5
566	302	294	-	-	SN400	RSB1	225.5
567	303	295	-	-	SN400	RSB2	225.5
568	296	300	-	-	SN400	RSB2	225.5
569	297	301	-	-	SN400	RSB1	225.5
570	298	302	-	-	SN400	RSB1	225.5
571	299	303	-	-	SN400	RSB2	225.5
572	301	300	000011	000011	SS400	RSG1	870
573	302	301	000011	000011	SS400	RSG1	870
574	303	302	000011	000011	SS400	RSG1	870
575	236	304	-	-	C280	PC1	380
576	237	305	-	-	C280	PC0	380
577	240	306	-	-	C280	PC3	380
578	241	307	-	-	C280	PC0	380
579	256	308	-	-	C280	PC10	380
580	259	309	-	-	C280	PC0	380
581	264	312	-	-	C280	PC1	380
582	265	313	-	-	C280	PC0	380
583	266	314	-	-	C280	PC2	380
584	310	272	-	-	C280	PC4	260
585	304	305	-	-	C280	PG1	425
586	304	306	-	-	C280	PB1	440
587	305	307	-	-	C280	PB1	440
588	306	307	-	-	C280	PG1	425
589	308	309	-	-	C280	PG2	780
590	309	310	-	-	C280	PG3	300
591	310	311	-	-	C280	PCG3	290
592	308	312	-	-	C280	PB1	420
593	313	309	-	-	C280	PB2	420
594	310	314	-	-	C280	PB1	420
595	315	311	-	-	C280	Pb1	420
596	312	313	-	-	C280	PG2	780
597	313	314	-	-	C280	PG3	300

598 314 315 - - C280 PCG3 290

*** TOTAL WEIGHT / VOLUME / SURFACE AREA SUMMARY

SECTION NO	SECION NAME	SURFACE AREA	VOLUMN	WEIGHT	FRAME NUMBER	TRUSS NUMBER
101	1C1	5.22e+005	9.744e+006	2.339e+004	4	0
102	1C2	2.61e+005	4.872e+006	1.169e+004	2	0
103	1C3	1.74e+005	4.35e+006	1.044e+004	1	0
104	1C4	3.48e+005	8.7e+006	2.088e+004	2	0
105	1C5	6.96e+005	1.74e+007	4.176e+004	4	0
106	1C6	3.48e+005	8.7e+006	2.088e+004	2	0
107	1C7	1.74e+005	4.35e+006	1.044e+004	1	0
108	1C8	5.22e+005	1.305e+007	3.132e+004	3	0
109	1C9	1.74e+005	4.35e+006	1.044e+004	1	0
110	1C10	2.088e+005	5.873e+006	1.409e+004	1	0
111	2C1	4.8e+005	8.96e+006	2.15e+004	4	0
112	2C2	2.4e+005	4.48e+006	1.075e+004	2	0
113	2C3	1.6e+005	4e+006	9600	1	0
114	2C4	3.2e+005	8e+006	1.92e+004	2	0
115	2C5	6.4e+005	1.6e+007	3.84e+004	4	0
116	2C6	3.2e+005	8e+006	1.92e+004	2	0
117	2C7	1.6e+005	4e+006	9600	1	0
118	2C8	4.8e+005	1.2e+007	2.88e+004	3	0
119	2C9	1.6e+005	4e+006	9600	1	0
120	2C10	1.92e+005	5.4e+006	1.296e+004	1	0
121	3C1	4.8e+005	8.96e+006	2.15e+004	4	0
122	3C2	2.4e+005	4.48e+006	1.075e+004	2	0
123	3C3	1.6e+005	4e+006	9600	1	0
124	3C4	3.2e+005	8e+006	1.92e+004	2	0
125	3C5	6.4e+005	1.6e+007	3.84e+004	4	0
126	3C6	3.2e+005	8e+006	1.92e+004	2	0
127	3C7	1.6e+005	4e+006	9600	1	0
128	3C8	4.8e+005	1.2e+007	2.88e+004	3	0
129	3C9	1.6e+005	4e+006	9600	1	0
130	3C10	1.92e+005	5.4e+006	1.296e+004	1	0
131	4C1	4.8e+005	8.96e+006	2.15e+004	4	0
132	4C2	2.4e+005	4.48e+006	1.075e+004	2	0
133	4C3	1.6e+005	4e+006	9600	1	0
134	4C4	3.68e+005	9.2e+006	2.208e+004	3	0
135	4C5	6.4e+005	1.6e+007	3.84e+004	4	0
136	4C6	3.2e+005	8e+006	1.92e+004	2	0
137	4C7	1.6e+005	4e+006	9600	1	0
138	4C8	0	0	0	0	0
139	4C9	1.6e+005	4e+006	9600	1	0
140	4C10	1.92e+005	5.4e+006	1.296e+004	1	0
141	BC1	6.48e+005	1.21e+007	2.903e+004	8	0
142	BC2	3.24e+005	6.048e+006	1.452e+004	4	0
143	BC3	2.16e+005	5.4e+006	1.296e+004	2	0
144	BC4	4.32e+005	1.08e+007	2.592e+004	4	0
145	BC5	8.64e+005	2.16e+007	5.184e+004	8	0
146	BC6	4.32e+005	1.08e+007	2.592e+004	4	0
147	BC7	2.16e+005	5.4e+006	1.296e+004	2	0
148	BC8	6.48e+005	1.62e+007	3.888e+004	6	0
149	BC9	2.16e+005	5.4e+006	1.296e+004	2	0
150	BC10	2.592e+005	7.29e+006	1.75e+004	2	0
151	BC11	3.24e+005	6.048e+006	1.452e+004	4	0
152	BC12	1.296e+005	1.944e+006	4666	2	0

153	BC13	1.728e+005	3.456e+006	8294	2	0
154	BC14	5.184e+005	7.776e+006	1.866e+004	8	0
155	BC15	1.404e+005	2.268e+006	5443	2	0
156	BC16	3.456e+005	6.912e+006	1.659e+004	4	0
161	PC1	2.28e+005	4.256e+006	1.021e+004	2	0
162	PC2	1.14e+005	2.128e+006	5107	1	0
163	PC3	1.52e+005	3.8e+006	9120	1	0
164	PC4	1.04e+005	2.6e+006	6240	1	0
165	PC10	1.824e+005	5.13e+006	1.231e+004	1	0
166	PC0	2.736e+005	3.04e+006	7296	4	0
171	4C11	2.458e+005	1.968e+005	1545	2	0
201	FG1	4.278e+005	1.042e+007	2.5e+004	1	0
202	FG2	4.002e+005	9.744e+006	2.339e+004	1	0
203	FG3	9.648e+005	2.573e+007	6.175e+004	3	0
204	FG4	1.67e+006	4.454e+007	1.069e+005	4	0
205	FG5	8.352e+005	2.227e+007	5.345e+004	2	0
206	FG6	4.836e+005	1.488e+007	3.571e+004	1	0
207	FG7	9.048e+005	2.784e+007	6.682e+004	2	0
208	FG8	4.524e+005	1.392e+007	3.341e+004	1	0
209	FG9	4.968e+005	1.21e+007	2.903e+004	1	0
210	FG10	4.002e+005	9.744e+006	2.339e+004	1	0
211	FG11	8.004e+005	1.949e+007	4.677e+004	2	0
212	FG12	1.214e+006	2.65e+007	6.359e+004	4	0
213	FG13	2.684e+005	5.856e+006	1.405e+004	1	0
214	FG14	6.624e+005	1.766e+007	4.239e+004	2	0
215	FG15	2.684e+005	5.856e+006	1.405e+004	1	0
216	FG16	2.684e+005	5.856e+006	1.405e+004	1	0
251	FB1	2.112e+005	5.632e+006	1.352e+004	1	0
252	FB2	4.32e+005	1.152e+007	2.765e+004	2	0
253	FB3	4.32e+005	1.152e+007	2.765e+004	2	0
254	FB4	4.032e+005	1.075e+007	2.58e+004	2	0
255	FB5	6.24e+005	1.664e+007	3.994e+004	3	0
256	FB6	1.872e+006	5.76e+007	1.382e+005	8	0
257	FB7	9.36e+005	2.88e+007	6.912e+004	4	0
258	FB8	9.36e+005	2.88e+007	6.912e+004	4	0
259	FB9	4.224e+005	1.126e+007	2.703e+004	2	0
260	FB10	1.936e+005	4.224e+006	1.014e+004	1	0
261	FB11	3.96e+005	8.64e+006	2.074e+004	3	0
262	FB12	7.105e+005	1.55e+007	3.72e+004	3	0
263	FB13	7.04e+005	1.536e+007	3.686e+004	4	0
264	FB14	6.432e+005	1.715e+007	4.116e+004	3	0
265	Fb1	3.115e+006	6.797e+007	1.631e+005	8	0
266	Fb2	1.144e+006	2.496e+007	5.99e+004	4	0
301	1G1	2.418e+005	3.72e+006	8928	1	0
302	1G2	2.262e+005	3.48e+006	8352	1	0
303	1G3	5.226e+005	8.04e+006	1.93e+004	3	0
304	1G4	9.048e+005	1.392e+007	3.341e+004	4	0
305	1G5	4.524e+005	6.96e+006	1.67e+004	2	0
306	1G6	2.418e+005	3.72e+006	8928	1	0
307	1G7	4.524e+005	6.96e+006	1.67e+004	2	0
308	1G8	2.262e+005	3.48e+006	8352	1	0
309	1G9	2.808e+005	4.32e+006	1.037e+004	1	0
310	1G10	2.262e+005	3.48e+006	8352	1	0
311	1G11	4.524e+005	6.96e+006	1.67e+004	2	0
312	1G12	7.176e+005	1.104e+007	2.65e+004	8	0
313	1G13	1.281e+005	1.647e+006	3953	1	0
314	1G14	4.14e+005	7.452e+006	1.788e+004	4	0
315	1G15	1.281e+005	1.647e+006	3953	1	0
316	1G16	1.281e+005	1.647e+006	3953	1	0

351	1B1	1.144e+005	1.76e+006	4224	1	0
352	1B2	2.34e+005	3.6e+006	8640	2	0
353	1B3	2.34e+005	3.6e+006	8640	2	0
354	1B4	1.764e+005	2.268e+006	5443	2	0
355	1B5	3.38e+005	5.2e+006	1.248e+004	3	0
356	1B6	9.36e+005	1.44e+007	3.456e+004	8	0
357	1B7	4.68e+005	7.2e+006	1.728e+004	4	0
358	1B8	4.68e+005	7.2e+006	1.728e+004	4	0
359	1B9	1.848e+005	2.376e+006	5702	2	0
360	1B10	9.24e+004	1.188e+006	2851	1	0
361	1B11	2.16e+005	3.15e+006	7560	2	0
362	1B12	3.875e+005	5.651e+006	1.356e+004	3	0
363	1B13	3.84e+005	5.6e+006	1.344e+004	2	0
364	1B14	4.02e+005	7.236e+006	1.737e+004	3	0
401	2G1	2.418e+005	3.72e+006	8928	1	0
402	2G2	2.262e+005	3.48e+006	8352	1	0
403	2G3	5.226e+005	8.04e+006	1.93e+004	3	0
404	2G4	9.048e+005	1.392e+007	3.341e+004	4	0
405	2G5	4.524e+005	6.96e+006	1.67e+004	2	0
406	2G6	2.883e+005	5.441e+006	1.306e+004	1	0
407	2G7	5.394e+005	1.018e+007	2.443e+004	2	0
408	2G8	2.697e+005	5.09e+006	1.221e+004	1	0
409	2G9	2.808e+005	4.32e+006	1.037e+004	1	0
410	2G10	2.262e+005	3.48e+006	8352	1	0
451	2B1	2.112e+005	3.168e+006	7603	2	0
452	2B2	2.34e+005	3.6e+006	8640	2	0
453	2B3	2.34e+005	3.6e+006	8640	2	0
454	2B4	2.016e+005	3.024e+006	7258	2	0
455	2B5	2.064e+005	3.096e+006	7430	2	0
456	2B6	1.116e+006	2.106e+007	5.054e+004	8	0
457	2B7	5.58e+005	1.053e+007	2.527e+004	4	0
458	2B8	4.68e+005	7.2e+006	1.728e+004	4	0
501	3G1	2.418e+005	3.72e+006	8928	1	0
502	3G2	2.262e+005	3.48e+006	8352	1	0
503	3G3	2.418e+005	3.72e+006	8928	1	0
504	3G4	9.048e+005	1.392e+007	3.341e+004	4	0
505	3G5	4.524e+005	6.96e+006	1.67e+004	2	0
506	3G6	2.79e+005	5.022e+006	1.205e+004	1	0
507	3G7	5.22e+005	9.396e+006	2.255e+004	2	0
508	3G8	2.61e+005	4.698e+006	1.128e+004	1	0
509	3G9	2.808e+005	4.32e+006	1.037e+004	1	0
510	3G10	2.262e+005	3.48e+006	8352	1	0
511	3G11	2.808e+005	4.32e+006	1.037e+004	2	0
551	3B1	2.112e+005	3.08e+006	7392	2	0
552	3B2	2.34e+005	3.6e+006	8640	2	0
553	3B3	2.34e+005	3.6e+006	8640	2	0
554	3B4	1.848e+005	2.52e+006	6048	2	0
555	3B5	2.064e+005	3.01e+006	7224	2	0
556	3B6	1.116e+006	2.106e+007	5.054e+004	8	0
557	3B7	5.58e+005	1.053e+007	2.527e+004	4	0
558	3B8	4.68e+005	7.2e+006	1.728e+004	4	0
601	4G1	2.418e+005	3.72e+006	8928	1	0
602	4G2	2.262e+005	3.48e+006	8352	1	0
603	4G3	2.418e+005	3.72e+006	8928	1	0
604	4G4	9.048e+005	1.392e+007	3.341e+004	4	0
605	4G5	4.524e+005	6.96e+006	1.67e+004	2	0
606	4G6	2.79e+005	5.022e+006	1.205e+004	1	0
607	4G7	5.22e+005	9.396e+006	2.255e+004	2	0
608	4G8	2.61e+005	4.698e+006	1.128e+004	1	0

609	4G9	2.808e+005	4.32e+006	1.037e+004	1	0
610	4G10	2.262e+005	3.48e+006	8352	1	0
611	4G11	2.808e+005	4.32e+006	1.037e+004	2	0
651	4B1	2.112e+005	3.08e+006	7392	2	0
652	4B2	2.34e+005	3.6e+006	8640	2	0
653	4B3	2.34e+005	3.6e+006	8640	2	0
654	4B4	1.764e+005	2.268e+006	5443	2	0
655	4B5	2.064e+005	3.01e+006	7224	2	0
656	4B6	5.4e+005	9.72e+006	2.333e+004	6	0
657	4B7	1.08e+006	1.944e+007	4.666e+004	8	0
658	4B8	4.68e+005	7.2e+006	1.728e+004	4	0
701	RG1	2.232e+005	3.255e+006	7812	2	0
702	RG2	2.088e+005	3.045e+006	7308	1	0
703	RG3	2.418e+005	3.72e+006	8928	2	0
704	RG4	8.352e+005	1.218e+007	2.923e+004	4	0
705	RG5	4.176e+005	6.09e+006	1.462e+004	2	0
706	RG6	2.418e+005	3.72e+006	8928	2	0
707	RG7	0	0	0	0	0
708	RG8	0	0	0	0	0
709	RG9	2.592e+005	3.78e+006	9072	2	0
710	RG10	2.088e+005	3.045e+006	7308	1	0
711	RG11	2.808e+005	4.32e+006	1.037e+004	4	0
751	RB1	2.112e+005	3.08e+006	7392	2	0
752	RB2	2.16e+005	3.15e+006	7560	2	0
753	RB3	2.16e+005	3.15e+006	7560	2	0
754	RB4	1.764e+005	2.268e+006	5443	2	0
755	RB5	2.064e+005	3.01e+006	7224	2	0
756	RB6	1.35e+005	2.125e+006	5100	2	0
757	RB7	3.51e+005	5.525e+006	1.326e+004	2	0
758	RB8	4.32e+005	6.3e+006	1.512e+004	2	0
801	PG1	1.445e+005	1.488e+006	3570	2	0
802	PG2	2.964e+005	3.276e+006	7862	2	0
803	PG3	1.14e+005	1.26e+006	3024	2	0
804	PCG3	9.86e+004	1.015e+006	2436	2	0
811	PB1	2.924e+005	3.01e+006	7224	4	0
812	PB2	7.14e+004	7.35e+005	1764	1	0
813	Pb1	6.72e+004	6.3e+005	1512	1	0
1001	b1a	3.24e+005	3.465e+006	8316	4	0
1002	b0	1.36e+005	1.275e+006	3060	2	0
1003	b3	2.622e+005	2.898e+006	6955	4	0
1004	b4	4.556e+005	4.69e+006	1.126e+004	6	0
1005	b1	1.877e+006	2.324e+007	5.579e+004	10	0
1006	4b2	1.148e+006	1.527e+007	3.664e+004	6	0
1007	2b2	1.628e+006	2.23e+007	5.352e+004	8	0
1008	1b1	4.092e+005	5.441e+006	1.306e+004	2	0
1009	1b2	1.148e+006	1.527e+007	3.664e+004	6	0
3001	RSB1	9.285e+005	8.369e+005	6570	16	0
3002	RSB2	8.484e+005	6.746e+005	5295	16	0
3101	RSG1	2.558e+006	1.736e+006	1.362e+004	15	0
3151	RSC1	2.451e+005	2.705e+005	2123	7	0
3152	RSC2	8.99e+004	6.601e+004	518.2	2	0

*** LOAD DATA

; Self Weight, Nodal Load, Specified Displacement, Beam Load, Floor Load, Finishing Material Load,
System Temperature, Nodal Temperature, Element Temperature, Beam Section Temperature,
Wind Load, Static Seismic Load, Time History Analysis Data

** FLOOR LOAD TYPE DATA

NAME	LOADCASE NAME	LOAD	SUB-BEAM WEIGHT
PH	DL	-0.048	Consider
	LL	-0.02	Do not consider
RF-1	DL	-0.048	Consider
	LL	-0.03	Do not consider
RF-2	DL	-0.03	Consider
	LL	-0.01	Do not consider
RF-3	DL	-0.0861	Consider
	LL	-0.055	Do not consider
4FL-1	DL	-0.0921	Consider
	LL	-0.03	Do not consider
4FL-2	DL	-0.1	Consider
	LL	-0.04	Do not consider
3FL	DL	-0.0865	Consider
	LL	-0.025	Do not consider
2FL-1	DL	-0.0883	Consider
	LL	-0.04	Do not consider
2FL-2	DL	-0.0883	Consider
	LL	-0.09	Do not consider
1FL-1	DL	-0.0771	Consider
	LL	-0.03	Do not consider
1FL-2	DL	-0.075	Consider
	LL	-0.06	Do not consider
BFL	DL	-0.0868	Consider
	LL	-0.05	Do not consider
FT-1	DL	1	Consider
FT-2	DL	0.35	Consider

** FLOOR LOAD DATA

LOAD TYPE	DISTRIBUTION	DIR.	PROJ	SUB-BEAM NUMBER	SUB-BEAM ANGLE	SUB-BEAM UNIT-W	NODE LIST
PH	Two Way	GZ	NO	0	0	0	312 308 311 315
PH	Two Way	GZ	NO	0	0	0	306 304 305 307
RF-1	Two Way	GZ	NO	0	0	0	267 264 256 261
RF-1	Two Way	GZ	NO	0	0	0	257 249 250 241 240 236 239 243 242 260
RF-3	Two Way	GZ	NO	0	0	0	249 240 241 250
4FL-1	Two Way	GZ	NO	0	0	0	233 235 230 229 206 207 204 202 228 227
4FL-2	Two Way	GZ	NO	0	0	0	229 206 209 232
3FL	Two Way	GZ	NO	0	0	0	199 193 194 170 172 175 177 198 196 201
2FL-2	Two Way	GZ	NO	0	0	0	168 163 164 169
2FL-1	Two Way	GZ	NO	0	0	0	167 161 162 138 140 143 145 166 163 168
1FL-1	Two Way	GZ	NO	0	0	0	135 129 130 85 87 97 99 134 132 137
1FL-2	Two Way	GZ	NO	0	0	0	97 87 93 83 84 105 123 119 99
BFL	Two Way	GZ	NO	0	0	0	80 74 75 37 43 33 34 54 68 66 79 77 82
FT-1	Two Way	GZ	NO	0	0	0	80 74 75 37 39 47 49 79 77 82
FT-2	Two Way	GZ	NO	0	0	0	47 39 43 33 34 54 68 66

[LOAD CASE : SW]

** SELF WEIGHT DATA

 ; X=0, Y=0, Z=-1

[LOAD CASE : DL]

** NODAL LOAD DATA

NODE	FX	FY	FZ	MX	MY	MZ
242	0	0	-2025	0	0	0
243	0	0	-4050	0	0	0
244	0	0	-4050	0	0	0
245	0	0	-2025	0	0	0
260	0	0	-2025	0	0	0
261	0	0	-4050	0	0	0
262	0	0	-4050	0	0	0
263	0	0	-2025	0	0	0
268	0	0	-2025	0	0	0
269	0	0	-4050	0	0	0
270	0	0	-4050	0	0	0
271	0	0	-2025	0	0	0
272	0	0	-2025	0	0	0
273	0	0	-4050	0	0	0
274	0	0	-4050	0	0	0
275	0	0	-2025	0	0	0
284	0	0	-4050	0	0	0
285	0	0	-8100	0	0	0
286	0	0	-8100	0	0	0
287	0	0	-4050	0	0	0
288	0	0	-4050	0	0	0
289	0	0	-8100	0	0	0
290	0	0	-8100	0	0	0
291	0	0	-4050	0	0	0
300	0	0	-4050	0	0	0
301	0	0	-8100	0	0	0
302	0	0	-8100	0	0	0
303	0	0	-4050	0	0	0

[LOAD CASE : LL]

** NODAL LOAD DATA

NODE	FX	FY	FZ	MX	MY	MZ
242	0	0	-1012	0	0	0
243	0	0	-2025	0	0	0
244	0	0	-2025	0	0	0
245	0	0	-1012	0	0	0
260	0	0	-1012	0	0	0
261	0	0	-2025	0	0	0
262	0	0	-2025	0	0	0
263	0	0	-1012	0	0	0
268	0	0	-1012	0	0	0
269	0	0	-2025	0	0	0

270	0	0	-2025	0	0	0
271	0	0	-1012	0	0	0
272	0	0	-1012	0	0	0
273	0	0	-2025	0	0	0
274	0	0	-2025	0	0	0
275	0	0	-1012	0	0	0
284	0	0	-2025	0	0	0
285	0	0	-4050	0	0	0
286	0	0	-4050	0	0	0
287	0	0	-2025	0	0	0
288	0	0	-2025	0	0	0
289	0	0	-4050	0	0	0
290	0	0	-4050	0	0	0
291	0	0	-2025	0	0	0
300	0	0	-2025	0	0	0
301	0	0	-4050	0	0	0
302	0	0	-4050	0	0	0
303	0	0	-2025	0	0	0

[LOAD CASE : EX]

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ECCENTRICITY RELATED DATA
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	X-DIRECTIONAL LOAD				Y-DIRECTIONAL LOAD			
STORY NAME	ACCIDENTAL ECCENT.	INHERENT ECCENT.	ACCIDENTAL AMP.FACTOR	INHERENT AMP.FACTOR	ACCIDENTAL ECCENT.	INHERENT ECCENT.	ACCIDENTAL AMP.FACTOR	INHERENT AMP.FACTOR
PH	0.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0
RF-2	-67.25	0.0	1.0	0.0	162.5	0.0	1.0	0.0
RF-1	0.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0
4F	-67.25	0.0	1.0	0.0	162.5	0.0	1.13	0.0
3F	-67.25	0.0	1.0	0.0	162.5	0.0	1.11	0.0
2F	-210.105	0.0	1.0	0.0	215.905	0.0	1.05	0.0
G.L	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

** Story Force = Seismic Force x Scale Factor + Added Force

SEISMIC LOAD GENERATION DATA X-DIRECTION

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
PH	92944.4	2015.0	55696.66	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RF-2	140720.5	1755.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RF-1	511192.1	1635.0	248560.9	482500.0	482500.0	0.0	0.0	0.0	0.0	0.0
4F	1.3e+006	1235.0	489987.0	399200.0	399200.0	482500.0	1.9e+008	2.7e+007	0.0	2.7e+007
3F	1.2e+006	835.0	309546.1	261800.0	261800.0	881700.0	5.5e+008	1.8e+007	0.0	1.8e+007
2F	1.3e+006	435.0	170513.7	140500.0	140500.0	1.1e+006	1.0e+009	3.0e+007	0.0	3.0e+007
G.L.	--	0.0	--	--	--	1.3e+006	1.6e+009	---	---	---

[LOAD CASE : EY]

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ECCENTRICITY RELATED DATA

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X-DIRECTIONAL LOAD Y-DIRECTIONAL LOAD

STORY NAME	ACCIDENTAL ECCENT.	INHERENT ECCENT.	ACCIDENTAL AMP.FACTOR	INHERENT AMP.FACTOR	ACCIDENTAL ECCENT.	INHERENT ECCENT.	ACCIDENTAL AMP.FACTOR	INHERENT AMP.FACTOR
PH	0.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0
RF-2	-67.25	0.0	1.0	0.0	162.5	0.0	1.0	0.0
RF-1	0.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0
4F	-67.25	0.0	1.0	0.0	162.5	0.0	1.13	0.0
3F	-67.25	0.0	1.0	0.0	162.5	0.0	1.11	0.0
2F	-210.105	0.0	1.0	0.0	215.905	0.0	1.05	0.0
G.L	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

** Story Force = Seismic Force x Scale Factor + Added Force

SEISMIC LOAD GENERATION DATA Y-DIRECTION

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
PH	92944.4	2015.0	55696.66	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RF-2	140720.5	1755.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
RF-1	511192.1	1635.0	248560.9	482500.0	482500.0	0.0	0.0	0.0	0.0	0.0
4F	1.3e+006	1235.0	489987.0	399200.0	399200.0	482500.0	1.9e+008	7.3e+007	0.0	7.3e+007
3F	1.2e+006	835.0	309546.1	261800.0	261800.0	881700.0	5.5e+008	4.7e+007	0.0	4.7e+007
2F	1.3e+006	435.0	170513.7	140500.0	140500.0	1.1e+006	1.0e+009	3.2e+007	0.0	3.2e+007
G.L.	--	0.0	--	--	--	1.3e+006	1.6e+009	---	---	---

[LOAD CASE : WX]

** Story Force = Wind Force x Scale Factor + Added Force

** Story Torsion = Wind Torsion x Scale Factor + Added Torsion

WIND LOAD GENERATION DATA X-DIRECTION

STORY NAME	PRESSURE	ELEV.	LOADED HEIGHT	LOADED BREADTH	WIND FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN`G MOMENT
PH	0.025	2015.0	130.0	0.0	4371.25	0.0	0.0	0.0	0.0
RF-2	0.025	1755.0	190.0	1345.0	0.0	0.0	0.0	0.0	0.0
RF-1	0.025	1635.0	260.0	0.0	6725.0	4574.2	4574.2	0.0	0.0
4F	0.025	1235.0	400.0	1345.0	12105.0	8302.6	8302.6	4574.2	1829680.0
3F	0.02	835.0	400.0	1345.0	9415.0	7304.4	7304.4	12876.8	6980400.0
2F	0.015	435.0	417.5	1345.0	17744.351	6282.9	6282.9	20181.2	1.51e+007
G.L.	0.015	0.0	217.5	4202.1	0.0	0.0	--	26464.1	2.66e+007

[LOAD CASE : WY]

** Story Force = Wind Force x Scale Factor + Added Force

** Story Torsion = Wind Torsion x Scale Factor + Added Torsion

W I N D L O A D G E N E R A T I O N D A T A Y - D I R E C T I O N

STORY NAME	PRESSURE	ELEV.	LOADED HEIGHT	LOADED BREADTH	WIND FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN`G MOMENT
PH	0.025	2015.0	130.0	0.0	10562.5	0.0	0.0	0.0	0.0
RF-2	0.025	1755.0	190.0	3250.0	0.0	0.0	0.0	0.0	0.0
RF-1	0.025	1635.0	260.0	0.0	16250.0	7853.9	7853.9	0.0	0.0
4F	0.025	1235.0	400.0	3250.0	29250.0	14493.2	14493.2	7853.9	3141560.0
3F	0.02	835.0	400.0	3250.0	22750.0	13059.8	13059.8	22347.1	1.21e+007
2F	0.015	435.0	417.5	3250.0	23837.801	11705.6	11705.6	35406.9	2.62e+007
G.L.	0.015	0.0	217.5	4318.1	0.0	0.0	--	47112.5	4.67e+007

[LOAD CASE : EZ]

** SELF WEIGHT DATA

; X=0, Y=0, Z=-0.104

** NODAL LOAD DATA

NODE	FX	FY	FZ	MX	MY	MZ
242	0	0	-421.2	0	0	0
243	0	0	-842.4	0	0	0
244	0	0	-842.4	0	0	0
245	0	0	-421.2	0	0	0
260	0	0	-421.2	0	0	0
261	0	0	-842.4	0	0	0
262	0	0	-842.4	0	0	0
263	0	0	-421.2	0	0	0
268	0	0	-421.2	0	0	0
269	0	0	-842.4	0	0	0
270	0	0	-842.4	0	0	0
271	0	0	-421.2	0	0	0
272	0	0	-421.2	0	0	0
273	0	0	-842.4	0	0	0
274	0	0	-842.4	0	0	0
275	0	0	-421.2	0	0	0
284	0	0	-842.4	0	0	0
285	0	0	-1685	0	0	0
286	0	0	-1685	0	0	0
287	0	0	-842.4	0	0	0
288	0	0	-842.4	0	0	0
289	0	0	-1685	0	0	0
290	0	0	-1685	0	0	0
291	0	0	-842.4	0	0	0
300	0	0	-842.4	0	0	0
301	0	0	-1685	0	0	0
302	0	0	-1685	0	0	0
303	0	0	-842.4	0	0	0

** BEAM LOAD DATA

MEMBER	TYPE	DIR.	PROJ.	D1	P1	D2	P2	D3	P3	D4	P4

33	Uniform Load	GZ	NO	0	0	0.5	-4.44	1	0	0	0
34	Uniform Load	GZ	NO	0	-4.44	0.24	-4.44	1	0	0	0
35	Uniform Load	GZ	NO	0	-4.44	0.24	-4.44	1	0	0	0
37	Uniform Load	GZ	NO	0	0	0.76	-4.44	1	-4.44	0	0
38	Uniform Load	GZ	NO	0	0	0.76	-4.44	1	-4.44	0	0
39	Uniform Load	GZ	NO	0	0	0.22	-3.2	0.78	-3.2	1	0
40	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
41	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
42	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
43	Uniform Load	GZ	NO	0	0	0.45	-3.2	1	-3.2	0	0
44	Uniform Load	GZ	NO	0	-3.2	0.55	-3.2	1	0	0	0
45	Uniform Load	GZ	NO	0	0	0.36	-3.2	0.64	-3.2	1	0
45	Uniform Load	GZ	NO	0	0	0.5	-4.44	1	0	0	0
46	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
47	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
47	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
48	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
48	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
49	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
49	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
50	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
50	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
52	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
52	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
53	Uniform Load	GZ	NO	0	0	0.5	-3.2	1	0	0	0
54	Uniform Load	GZ	NO	0	0	0.22	-3.2	0.78	-3.2	1	0
54	Uniform Load	GZ	NO	0	0	0.46	-6.55	0.54	-6.55	1	0
55	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
55	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
56	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
56	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
57	Uniform Load	GZ	NO	0	0	0.24	-3.2	0.76	-3.2	1	0
57	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
58	Uniform Load	GZ	NO	0	0	0.93	-6.55	1	-6.55	0	0
58	Uniform Load	GZ	NO	0	0	0.45	-3.2	1	-3.2	0	0
59	Uniform Load	GZ	NO	0	-6.55	0.072	-6.55	1	0	0	0
59	Uniform Load	GZ	NO	0	-3.2	0.55	-3.2	1	0	0	0
60	Uniform Load	GZ	NO	0	0	0.36	-3.2	0.64	-3.2	1	0
60	Uniform Load	GZ	NO	0	0	0.46	-4.07	0.59	-3.61	1	0
61	Uniform Load	GZ	NO	0	0	0.78	-4.07	1	-3.54	0	0
61	Uniform Load	GZ	NO	0	0	1	-5.24	0	0	0	0
62	Uniform Load	GZ	NO	0	0	0.31	-3.61	1	0	0	0
63	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
64	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
64	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
65	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
65	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
66	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
66	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
67	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
67	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
69	Uniform Load	GZ	NO	0	0	0.36	-3.2	0.64	-3.2	1	0
69	Uniform Load	GZ	NO	0	0	0.46	-4.07	0.59	-3.61	1	0
70	Uniform Load	GZ	NO	0	-5.24	1	-6.55	0	0	0	0
70	Uniform Load	GZ	NO	0	-3.54	1	-2.95	0	0	0	0
77	Uniform Load	GZ	NO	0	0	0.76	-4.07	0.86	-3.61	1	0
78	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
79	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
79	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0

80	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
80	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
81	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
81	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
82	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
82	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
84	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
84	Uniform Load	GZ	NO	0	-2.95	1	0	0	0	0	0
85	Uniform Load	GZ	NO	0	0	0.46	-6.55	0.54	-6.55	1	0
85	Uniform Load	GZ	NO	0	0	0.46	-6.55	0.54	-6.55	1	0
86	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
86	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
87	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
87	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
88	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
88	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
89	Uniform Load	GZ	NO	0	0	0.93	-6.55	1	-6.55	0	0
90	Uniform Load	GZ	NO	0	-6.55	0.072	-6.55	1	0	0	0
91	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
92	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
92	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
93	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
93	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
94	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
94	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
95	Uniform Load	GZ	NO	0	0	1	-6.55	0	0	0	0
100	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
101	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
101	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
102	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
102	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
103	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
103	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
104	Uniform Load	GZ	NO	0	-6.55	1	0	0	0	0	0
105	Uniform Load	GZ	NO	0	0	1	-2.18	0	0	0	0
106	Uniform Load	GZ	NO	0	0	0.46	-6.55	0.54	-6.55	1	0
106	Uniform Load	GZ	NO	0	-2.18	0.061	-3.06	0.79	-3.06	1	0
107	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
107	Uniform Load	GZ	NO	0	0	0.23	-3.06	0.77	-3.06	1	0
108	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
109	Uniform Load	GZ	NO	0	0	0.5	-6.55	1	0	0	0
110	Uniform Load	GZ	NO	0	0	0.5	-3.06	1	0	0	0
111	Uniform Load	GZ	NO	0	0	0.5	-3.06	1	0	0	0
111	Uniform Load	GZ	NO	0	0	0.5	-3.06	1	0	0	0
112	Uniform Load	GZ	NO	0	0	0.5	-3.06	1	0	0	0
113	Uniform Load	GZ	NO	0	0	0.19	-3.06	0.81	-3.06	1	0
114	Uniform Load	GZ	NO	0	0	0.23	-3.06	0.77	-3.06	1	0
147	Uniform Load	GZ	NO	0	0	0.5	-4.38	1	0	0	0
148	Uniform Load	GZ	NO	0	0	0.38	-4.38	0.62	-4.38	1	0
149	Uniform Load	GZ	NO	0	0	0.38	-4.38	0.62	-4.38	1	0
150	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
151	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
152	Uniform Load	GZ	NO	0	0	0.24	-3.16	0.76	-3.16	1	0
153	Uniform Load	GZ	NO	0	0	0.24	-3.16	0.76	-3.16	1	0
154	Uniform Load	GZ	NO	0	0	0.91	-3.16	1	-3.16	0	0
155	Uniform Load	GZ	NO	0	-3.16	1	-3.16	0	0	0	0
156	Uniform Load	GZ	NO	0	-3.16	1	-3.16	0	0	0	0
157	Uniform Load	GZ	NO	0	-3.16	0.093	-3.16	1	0	0	0
158	Uniform Load	GZ	NO	0	0	0.5	-4.38	1	0	0	0

158	Uniform Load	GZ	NO	0	0	0.36	-3.16	0.64	-3.16	1	0
159	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
160	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
160	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
161	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
161	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
162	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
162	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
163	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
163	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
167	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
167	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
168	Uniform Load	GZ	NO	0	0	0.5	-3.16	1	0	0	0
169	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
169	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
170	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
170	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
171	Uniform Load	GZ	NO	0	0	0.24	-3.16	0.76	-3.16	1	0
171	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
172	Uniform Load	GZ	NO	0	0	0.24	-3.16	0.76	-3.16	1	0
172	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
173	Uniform Load	GZ	NO	0	0	1	-3.48	0	0	0	0
173	Uniform Load	GZ	NO	0	0	0.91	-3.16	1	-3.16	0	0
174	Uniform Load	GZ	NO	0	-3.16	1	-3.16	0	0	0	0
174	Uniform Load	GZ	NO	0	-3.48	0.86	-6.46	1	-6.46	0	0
175	Uniform Load	GZ	NO	0	-6.46	0.14	-6.46	1	-3.48	0	0
175	Uniform Load	GZ	NO	0	-3.16	1	-3.16	0	0	0	0
176	Uniform Load	GZ	NO	0	-3.16	0.093	-3.16	1	0	0	0
176	Uniform Load	GZ	NO	0	-3.48	1	0	0	0	0	0
177	Uniform Load	GZ	NO	0	0	0.36	-3.16	0.64	-3.16	1	0
177	Uniform Load	GZ	NO	0	0	0.46	-4.01	0.59	-3.56	1	0
178	Uniform Load	GZ	NO	0	0	0.31	-3.56	1	0	0	0
179	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
180	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
180	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
181	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
181	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
182	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
182	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
183	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
183	Uniform Load	GZ	NO	0	0	1	-6.46	0	0	0	0
187	Uniform Load	GZ	NO	0	0	0.62	-4.01	1	-2.91	0	0
187	Uniform Load	GZ	NO	0	0	1	-6.46	0	0	0	0
196	Uniform Load	GZ	NO	0	0	0.76	-4.01	0.86	-3.56	1	0
197	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
198	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
198	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
199	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
199	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
200	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
200	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
201	Uniform Load	GZ	NO	2.5e-016	-6.46	1	0	0	0	0	0
201	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
205	Uniform Load	GZ	NO	2.5e-016	-6.46	1	0	0	0	0	0
205	Uniform Load	GZ	NO	0	-2.91	1	0	0	0	0	0
206	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
206	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
207	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
207	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0

208	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
208	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
209	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
209	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
210	Uniform Load	GZ	NO	0	0	1	-3.48	0	0	0	0
211	Uniform Load	GZ	NO	0	-3.48	0.86	-6.46	1	-6.46	0	0
212	Uniform Load	GZ	NO	0	-6.46	0.14	-6.46	1	-3.48	0	0
213	Uniform Load	GZ	NO	0	-3.48	1	0	0	0	0	0
214	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
215	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
215	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
216	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
216	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
217	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
217	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
218	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
223	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
224	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
224	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
225	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
225	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
226	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
226	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
227	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
228	Uniform Load	GZ	NO	0	0	1	-2.03	0	0	0	0
229	Uniform Load	GZ	NO	0	-2.03	0.061	-2.84	0.79	-2.84	1	0
229	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
230	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
230	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
231	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
232	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
233	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
234	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
234	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
235	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
236	Uniform Load	GZ	NO	0	0	0.19	-2.84	0.81	-2.84	1	0
237	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
259	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
260	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
261	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
262	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
262	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
263	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
264	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
264	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
265	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
265	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
266	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
267	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
268	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
269	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
269	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
270	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
270	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
271	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
271	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
272	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
277	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
278	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0

278	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
279	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
279	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
280	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
280	Uniform Load	GZ	NO	5.1e-016	-6.08	1	0	0	0	0	0
281	Uniform Load	GZ	NO	5.1e-016	-6.08	1	0	0	0	0	0
282	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
282	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
283	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
283	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
284	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
284	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
285	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
285	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
286	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
287	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
287	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
288	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
288	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
289	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
289	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
290	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
295	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
296	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
296	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
297	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
297	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
298	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
298	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
299	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
300	Uniform Load	GZ	NO	0	0	1	-2.03	0	0	0	0
301	Uniform Load	GZ	NO	0	-2.03	0.061	-2.84	0.79	-2.84	1	0
301	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
302	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
302	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
303	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
304	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
305	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
306	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
306	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
307	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
308	Uniform Load	GZ	NO	0	0	0.19	-2.84	0.81	-2.84	1	0
309	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
331	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
332	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
333	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
334	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
334	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
335	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
336	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
336	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
337	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
337	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
338	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
339	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
340	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
341	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
341	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
342	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0

342	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
343	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
343	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
344	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
349	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
350	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
350	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
351	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
351	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
352	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
352	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
353	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
354	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
354	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
355	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
355	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
356	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
356	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
357	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
357	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
358	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
359	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
359	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
360	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
360	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
361	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
361	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
362	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
367	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
368	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
368	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
369	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
369	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
370	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
370	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
371	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
372	Uniform Load	GZ	NO	0	0	1	-2.03	0	0	0	0
373	Uniform Load	GZ	NO	0	-2.03	0.061	-2.84	0.79	-2.84	1	0
373	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
374	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
374	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
375	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
376	Uniform Load	GZ	NO	0	0	0.5	-6.08	1	0	0	0
377	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
378	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
378	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
379	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
380	Uniform Load	GZ	NO	0	0	0.19	-2.84	0.81	-2.84	1	0
381	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
403	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
404	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0
405	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
406	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
406	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
407	Uniform Load	GZ	NO	0	0	0.5	-2.97	1	0	0	0
408	Uniform Load	GZ	NO	0	0	0.22	-2.97	0.78	-2.97	1	0
408	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
409	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
409	Uniform Load	GZ	NO	0	0	0.24	-2.97	0.76	-2.97	1	0

410	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
411	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
412	Uniform Load	GZ	NO	0	0	1	-5.2	0	0	0	0
412	Uniform Load	GZ	NO	0	0	1	-3.38	0	0	0	0
413	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
414	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
414	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
415	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
415	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
416	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
417	Uniform Load	GZ	NO	0	-5.2	1	-9.36	0	0	0	0
417	Uniform Load	GZ	NO	0	-3.38	1	-6.08	0	0	0	0
422	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
423	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
423	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
424	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
424	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
425	Uniform Load	GZ	NO	5.1e-016	-9.36	1	0	0	0	0	0
425	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
426	Uniform Load	GZ	NO	5.1e-016	-9.36	1	0	0	0	0	0
427	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
427	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
428	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
428	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
429	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
429	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
430	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
430	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
431	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
432	Uniform Load	GZ	NO	0	0	1	-6.08	0	0	0	0
432	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
433	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
433	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
434	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
434	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
435	Uniform Load	GZ	NO	0	0	1	-9.36	0	0	0	0
440	Uniform Load	GZ	NO	0	-6.08	1	-3.38	0	0	0	0
440	Uniform Load	GZ	NO	0	-9.36	1	-5.2	0	0	0	0
441	Uniform Load	GZ	NO	0	-6.08	1	0	0	0	0	0
442	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
442	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
443	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
443	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
444	Uniform Load	GZ	NO	0	-9.36	1	0	0	0	0	0
445	Uniform Load	GZ	NO	0	-5.2	1	0	0	0	0	0
445	Uniform Load	GZ	NO	0	-3.38	1	0	0	0	0	0
446	Uniform Load	GZ	NO	0	0	1	-2.03	0	0	0	0
447	Uniform Load	GZ	NO	0	0	0.46	-6.08	0.54	-6.08	1	0
447	Uniform Load	GZ	NO	0	-2.03	0.061	-2.84	0.79	-2.84	1	0
448	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
448	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0
449	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
450	Uniform Load	GZ	NO	0	0	0.5	-9.36	1	0	0	0
451	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
452	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
452	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
453	Uniform Load	GZ	NO	0	0	0.5	-2.84	1	0	0	0
454	Uniform Load	GZ	NO	0	0	0.19	-2.84	0.81	-2.84	1	0
455	Uniform Load	GZ	NO	0	0	0.23	-2.84	0.77	-2.84	1	0

476	Uniform Load	GZ	NO	0	0	0.52	-2.29	1	-2.29	0	0
477	Uniform Load	GZ	NO	0	-2.29	0.6	-2.29	1	0	0	0
478	Uniform Load	GZ	NO	0	0	0.24	-2.29	0.76	-2.29	1	0
479	Uniform Load	GZ	NO	0	0	0.5	-2.29	1	0	0	0
480	Uniform Load	GZ	NO	0	0	0.5	-2.29	1	0	0	0
480	Uniform Load	GZ	NO	0	0	0.5	-2.29	1	0	0	0
481	Uniform Load	GZ	NO	0	0	0.5	-2.29	1	0	0	0
482	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
482	Uniform Load	GZ	NO	0	0	0.52	-2.29	1	-2.29	0	0
483	Uniform Load	GZ	NO	0	-2.29	0.6	-2.29	1	0	0	0
483	Uniform Load	GZ	NO	0	0	0.5	-2.89	1	0	0	0
484	Uniform Load	GZ	NO	0	0	0.24	-2.29	0.76	-2.29	1	0
487	Uniform Load	GZ	NO	0	0	1	-2.6	0	0	0	0
488	Uniform Load	GZ	NO	0	0	0.47	-2.21	1	-2.21	0	0
489	Uniform Load	GZ	NO	0	0	0.62	-2.89	1	-2.89	0	0
489	Uniform Load	GZ	NO	0	0	0.47	-2.21	1	-2.21	0	0
490	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
490	Uniform Load	GZ	NO	0	0	0.52	-2.29	1	-2.29	0	0
492	Uniform Load	GZ	NO	0	-2.6	0.042	-2.89	0.57	-2.89	1	0
493	Uniform Load	GZ	NO	0	-2.21	0.53	-2.21	1	0	0	0
494	Uniform Load	GZ	NO	0	-2.89	0.38	-2.89	1	0	0	0
494	Uniform Load	GZ	NO	0	-2.21	0.53	-2.21	1	0	0	0
495	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
495	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
496	Uniform Load	GZ	NO	0	0	0.5	-2.89	1	0	0	0
496	Uniform Load	GZ	NO	0	0	0.5	-2.89	1	0	0	0
497	Uniform Load	GZ	NO	0	0	0.47	-2.21	1	-2.21	0	0
498	Uniform Load	GZ	NO	0	0	0.47	-2.21	1	-2.21	0	0
498	Uniform Load	GZ	NO	0	0	0.62	-2.89	1	-2.89	0	0
499	Uniform Load	GZ	NO	0	0	0.43	-2.89	0.96	-2.89	1	-2.6
500	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
500	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
502	Uniform Load	GZ	NO	0	-2.21	0.53	-2.21	1	0	0	0
503	Uniform Load	GZ	NO	0	-2.89	0.38	-2.89	1	0	0	0
503	Uniform Load	GZ	NO	0	-2.21	0.53	-2.21	1	0	0	0
504	Uniform Load	GZ	NO	0	-2.6	1	0	0	0	0	0
505	Uniform Load	GZ	NO	0	0	1	-1.56	0	0	0	0
506	Uniform Load	GZ	NO	0	-1.56	0.14	-2.18	1	-2.18	0	0
506	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
507	Uniform Load	GZ	NO	0	0	1	-2.13	0	0	0	0
507	Uniform Load	GZ	NO	0	-2.18	1	-2.18	0	0	0	0
508	Uniform Load	GZ	NO	0	-2.18	0.4	-2.18	1	0	0	0
508	Uniform Load	GZ	NO	0	-2.13	0.21	-2.89	1	0	0	0
509	Uniform Load	GZ	NO	0	0	0.23	-2.18	0.77	-2.18	1	0
512	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
513	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
513	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
514	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
515	Uniform Load	GZ	NO	0	0	0.27	-2.18	1	-2.18	0	0
516	Uniform Load	GZ	NO	0	-2.18	0.4	-2.18	1	0	0	0
517	Uniform Load	GZ	NO	0	0	0.23	-2.18	0.77	-2.18	1	0
585	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
586	Uniform Load	GZ	NO	0	0	0.48	-2.21	0.52	-2.21	1	0
587	Uniform Load	GZ	NO	0	0	0.48	-2.21	0.52	-2.21	1	0
588	Uniform Load	GZ	NO	0	0	0.5	-2.21	1	0	0	0
589	Uniform Load	GZ	NO	0	0	0.27	-2.18	0.73	-2.18	1	0
590	Uniform Load	GZ	NO	0	0	0.5	-1.82	1	0	0	0
592	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
593	Uniform Load	GZ	NO	0	0	0.42	-1.82	0.58	-1.82	1	0

593	Uniform Load	GZ	NO	0	0	0.5	-2.18	1	0	0	0
594	Uniform Load	GZ	NO	0	0	0.42	-1.82	0.58	-1.82	1	0
596	Uniform Load	GZ	NO	0	0	0.27	-2.18	0.73	-2.18	1	0
597	Uniform Load	GZ	NO	0	0	0.5	-1.82	1	0	0	0

*** LOAD COMBINATION DATA

** CONCRETE DESIGN

NO	NAME	TYPE	ACTIVE	DESCRIPTION
1	cLCB42	Add	STRENGTH	1.4D
2	cLCB1	Add	STRENGTH	1.2D + 1.6L
3	cLCB2	Add	STRENGTH	1.2D +1.0L + 1.6WX
4	cLCB3	Add	STRENGTH	1.2D + 1.0L + 1.6WY
5	cLCB4	Add	STRENGTH	1.2D +1.0L - 1.6WX
6	cLCB5	Add	STRENGTH	1.2D +1.0L - 1.6WY
7	cLCB6	Add	STRENGTH	0.9D + 1.6WX
8	cLCB7	Add	STRENGTH	0.9D + 1.6WY
9	cLCB8	Add	STRENGTH	0.9D - 1.6WX
10	cLCB9	Add	STRENGTH	0.9D - 1.6WY
11	cLCB10	Add	STRENGTH	1.2D + 1.0L + 1.0(EX+0.3EZ)
12	cLCB11	Add	STRENGTH	1.2D + 1.0L + 1.0(EX-0.3EZ)
13	cLCB12	Add	STRENGTH	1.2D + 1.0L + 1.0(EY+0.3EZ)
14	cLCB13	Add	STRENGTH	1.2D + 1.0L + 1.0(EY-0.3EZ)
15	cLCB14	Add	STRENGTH	1.2D + 1.0L + 1.0(EZ+0.3EX)
16	cLCB15	Add	STRENGTH	1.2D + 1.0L + 1.0(EZ-0.3EX)
17	cLCB16	Add	STRENGTH	1.2D + 1.0L + 1.0(EZ+0.3EY)
18	cLCB17	Add	STRENGTH	1.2D + 1.0L + 1.0(EZ-0.3EY)
19	cLCB18	Add	STRENGTH	1.2D + 1.0L - 1.0(EX+0.3EZ)
20	cLCB19	Add	STRENGTH	1.2D + 1.0L - 1.0(EX-0.3EZ)
21	cLCB20	Add	STRENGTH	1.2D + 1.0L - 10(EY+0.3EZ)
22	cLCB21	Add	STRENGTH	1.2D + 1.0L - 1.0(EY-0.3EZ)
23	cLCB22	Add	STRENGTH	1.2D + 1.0L - 1.0(EZ+0.3EX)
24	cLCB23	Add	STRENGTH	1.2D + 1.0L - 1.0(EZ-0.3EX)
25	cLCB24	Add	STRENGTH	1.2D + 1.0L - 1.0(EZ+0.3EY)
26	cLCB25	Add	STRENGTH	1.2D + 1.0L - 1.0(EZ-0.3EY)
27	cLCB26	Add	STRENGTH	0.9D + 1.0(EX+0.3EZ)
28	cLCB27	Add	STRENGTH	0.9D + 1.0(EX-0.3EZ)
29	cLCB28	Add	STRENGTH	0.9D + 1.0(EY+0.3EZ)
30	cLCB29	Add	STRENGTH	0.9D + 1.0(EY-0.3EZ)
31	cLCB30	Add	STRENGTH	0.9D + 1.0(EZ+0.3EX)
32	cLCB31	Add	STRENGTH	0.9D + 1.0(EZ-0.3EX)
33	cLCB32	Add	STRENGTH	0.9D + 1.0(EZ+0.3EY)
34	cLCB33	Add	STRENGTH	0.9D + 1.0(EZ-0.3EY)
35	cLCB34	Add	STRENGTH	0.9D - 1.0(EX+0.3EZ)
36	cLCB35	Add	STRENGTH	0.9D - 1.0(EX-0.3EZ)
37	cLCB36	Add	STRENGTH	0.9D - 1.0(EY+0.3EZ)
38	cLCB37	Add	STRENGTH	0.9D - 1.0(EY-0.3EZ)
39	cLCB38	Add	STRENGTH	0.9D - 1.0(EZ+0.3EX)
40	cLCB39	Add	STRENGTH	0.9D - 1.0(EZ-0.3EX)
41	cLCB40	Add	STRENGTH	0.9D - 1.0(EZ+0.3EY)
42	cLCB41	Add	STRENGTH	0.9D - 1.0(EZ-0.3EY)

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 301 (1G1, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.0E+07(20)	42.693	6-D32	6988483(28)	27.369	4-D32	32047.4(20)	4.1667	2-D13 @180
M	OK	3837721(35)	14.543	2-D32	4847527(11)	18.557	3-D32	27073.2(11)	4.1667	2-D13 @300
J	OK	1.1E+07(11)	43.299	6-D32	6194156(35)	24.050	3-D32	33237.5(11)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 302 (1G2, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.0E+07(20)	41.144	6-D32	6447848(28)	25.103	4-D32	31998.4(20)	4.1667	2-D13 @180
M	OK	3797245(35)	14.383	2-D32	4203065(11)	15.985	2-D32	26591.8(11)	4.1667	2-D13 @300
J	OK	1.0E+07(11)	41.061	6-D32	6428586(35)	25.023	4-D32	32159.7(11)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 303 (1G3, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(20)	71.825	9-D32	7846116(20)	31.549	4-D32	61070.2(20)	13.716	2-D13 @170
M	OK	4255496(28)	16.193	2-D32	5566252(11)	21.468	3-D32	36527.7(11)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	63.750	8-D32	7193091(11)	28.764	4-D32	46128.7(11)	7.6538	2-D13 @170

*.MEMB = 0, SECT = 304 (1G4, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	61.792	8-D32	8404367(28)	33.445	5-D32	47680.8(20)	8.2666	2-D13 @170
M	OK	4860526(35)	18.610	3-D32	5486763(20)	21.144	3-D32	38597.6(11)	4.3795	2-D13 @300
J	OK	1.4E+07(11)	61.493	8-D32	8682617(35)	34.665	5-D32	47818.8(11)	8.3211	2-D13 @170

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 305 (1G5, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(20)	58.183	8-D32	8481488(28)	33.783	5-D32	45827.6(20)	7.5350	2-D13 @170
M	OK	4804915(35)	18.386	3-D32	5554426(20)	21.420	3-D32	38734.8(11)	4.4326	2-D13 @300
J	OK	1.4E+07(11)	62.174	8-D32	9039154(35)	36.240	5-D32	47956.0(11)	8.3753	2-D13 @170

*.MEMB = 0, SECT = 306 (1G6, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	61.233	8-D32	7571665(28)	29.846	4-D32	46099.1(20)	7.6422	2-D13 @170
M	OK	4458915(28)	17.002	3-D32	5745307(11)	22.201	3-D32	37633.7(11)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	63.424	8-D32	7163255(11)	28.764	4-D32	47228.9(11)	8.0882	2-D13 @170

*.MEMB = 0, SECT = 307 (1G7, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	63.307	8-D32	8015568(28)	31.756	4-D32	47400.0(20)	8.1557	2-D13 @170
M	OK	4923958(35)	18.865	3-D32	5592845(20)	21.577	3-D32	38834.8(20)	4.4713	2-D13 @300
J	OK	1.4E+07(11)	62.354	8-D32	8154212(35)	32.356	4-D32	47093.2(11)	8.0346	2-D13 @170

*.MEMB = 0, SECT = 308 (1G8, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	60.853	8-D32	7619913(28)	30.053	4-D32	46337.9(20)	7.7364	2-D13 @170
M	OK	4740285(35)	18.127	3-D32	5457069(11)	21.023	3-D32	37772.7(20)	4.1667	2-D13 @300
J	OK	1.3E+07(11)	58.852	8-D32	7711348(35)	30.445	4-D32	45917.3(11)	7.5704	2-D13 @170

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 309 (1G9, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(20)	43.963	6-D32	5707592(28)	22.777	3-D32	31293.0(20)	4.1667	2-D13 @180
M	OK	3423790(35)	12.921	2-D32	4584198(11)	17.502	3-D32	24281.9(11)	4.1667	2-D13 @300
J	OK	1.1E+07(11)	43.737	6-D32	5345123(11)	22.777	3-D32	31863.0(11)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 310 (1G10, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(20)	46.344	6-D32	7058427(28)	27.665	4-D32	35099.3(20)	4.1667	2-D13 @180
M	OK	4261526(28)	16.217	2-D32	5105053(20)	19.595	3-D32	29535.2(20)	4.1667	2-D13 @300
J	OK	1.1E+07(11)	44.552	6-D32	8060602(35)	31.951	4-D32	33357.7(11)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 311 (1G11, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	8670350(20)	34.611	5-D32	4921420(28)	19.232	3-D32	28036.4(20)	4.1667	2-D13 @180
M	OK	3350231(35)	12.634	2-D32	3947781(11)	14.976	2-D32	22460.5(11)	4.1667	2-D13 @300
J	OK	7187099(11)	28.209	4-D32	3593549(11)	15.579	2-D32	28684.5(11)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 312 (1G12, RECT), Span = 690.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	47.956	6-D32	8257576(28)	32.805	5-D32	38306.2(20)	4.2667	2-D13 @180
M	OK	5392413(35)	20.760	3-D32	5505116(11)	21.219	3-D32	34887.4(11)	4.1667	2-D13 @300
J	OK	9032369(11)	36.210	5-D32	5379438(35)	20.707	3-D32	39469.5(11)	4.7169	2-D13 @180

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 313 (1G13, RECT), Span = 610.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3163665(20)	16.608	3-D32	1581832(20)	11.613	2-D32	21097.7(20)	3.7500	2-D13 @130
M	OK	790916(20)	7.5706	2-D32	1602100(2)	8.1423	2-D32	16292.4(20)	3.7500	2-D13 @260
J	OK	2369085(11)	12.229	2-D32	1263118(20)	8.0475	2-D32	17832.1(11)	3.7500	2-D13 @130

*.MEMB = 0, SECT = 314 (1G14, RECT), Span = 1380.00
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.3E+07(20)	91.388	12-D32	1.1E+07(20)	42.070	6-D32	70759.8(20)	12.107	2-D13 @190
M	OK	5748014(20)	20.183	3-D32	6974841(11)	23.374	3-D32	55969.3(20)	6.3615	2-D13 @300
J	OK	1.6E+07(11)	56.522	7-D32	7912378(11)	26.882	4-D32	58308.9(11)	7.1570	2-D13 @200

*.MEMB = 0, SECT = 315 (1G15, RECT), Span = 610.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	5657913(20)	31.534	4-D32	2828957(20)	15.185	2-D32	26766.4(2)	4.4729	2-D13 @130
M	OK	1864030(20)	9.5230	2-D32	1436658(11)	8.0475	2-D32	21877.2(20)	3.7500	2-D13 @260
J	OK	1414478(20)	8.0475	2-D32	1414478(20)	8.0475	2-D32	12995.3(11)	3.7500	2-D13 @130

*.MEMB = 0, SECT = 316 (1G16, RECT), Span = 610.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1123597(2)	7.5433	2-D32	561798(2)	8.0475	2-D32	11192.2(2)	3.7500	2-D13 @130
M	OK	280899(2)	5.1773	2-D32	1048787(2)	7.0310	2-D32	7843.51(2)	0.0000	2-D13 @260
J	OK	1024862(11)	6.8675	2-D32	512431(11)	8.0475	2-D32	10853.8(2)	3.7500	2-D13 @130

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 351 (1B1, RECT), Span = 440.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(22)	75.283	10-D32	1.6E+07(30)	73.004	9-D32	79731.4(22)	21.306	2-D13 @110
M	OK	9314366(13)	37.467	5-D32	8866964(37)	35.478	5-D32	81248.0(13)	20.885	2-D13 @120
J	OK	1.8E+07(13)	85.192	11-D32	1.7E+07(37)	77.766	10-D32	83313.9(13)	22.886	2-D13 @110

*.MEMB = 0, SECT = 352 (1B2, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(22)	70.947	9-D32	8958977(30)	35.885	5-D32	48601.7(22)	8.7600	2-D13 @170
M	OK	5791871(30)	22.392	3-D32	6067996(13)	23.528	3-D32	43995.5(13)	6.4685	2-D13 @300
J	OK	1.6E+07(13)	76.174	10-D32	8878418(37)	36.544	5-D32	49858.9(13)	9.3675	2-D13 @170

*.MEMB = 0, SECT = 353 (1B3, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(22)	64.141	8-D32	7228898(22)	28.764	4-D32	43455.6(22)	6.5985	2-D13 @170
M	OK	5219905(21)	20.060	3-D32	5904086(13)	22.853	3-D32	37592.2(22)	4.1667	2-D13 @300
J	OK	6752114(13)	26.375	4-D32	4107005(37)	15.605	2-D32	33762.2(13)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 354 (1B4, RECT), Span = 420.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	9353400(22)	58.522	8-D32	8595389(30)	53.619	7-D32	46766.1(22)	15.932	2-D13 @120
M	OK	4519696(37)	24.476	4-D32	4486318(13)	24.276	3-D32	45230.6(22)	14.113	2-D13 @170
J	OK	8938261(13)	56.090	7-D32	8549442(37)	53.352	7-D32	45812.5(13)	15.169	2-D13 @130

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 355 (1B5, RECT), Span = 440.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(22)	83.698	11-D32	1.5E+07(30)	70.617	9-D32	86372.2(22)	24.114	2-D13 @100
M	OK	9143808(21)	36.706	5-D32	8244128(29)	32.747	5-D32	83735.6(22)	21.848	2-D13 @110
J	OK	1.8E+07(13)	82.932	11-D32	1.5E+07(37)	68.644	9-D32	82402.5(13)	22.520	2-D13 @110

*.MEMB = 0, SECT = 356 (1B6, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.9E+07(22)	87.935	11-D32	9510528(22)	39.437	5-D32	65582.7(22)	15.768	2-D13 @160
M	OK	5273853(30)	20.279	3-D32	7141054(2)	28.014	4-D32	56662.0(13)	11.370	2-D13 @220
J	OK	1.9E+07(13)	88.285	11-D32	9551548(13)	39.437	5-D32	65729.3(13)	15.827	2-D13 @160

*.MEMB = 0, SECT = 357 (1B7, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(22)	78.945	10-D32	8522377(22)	36.544	5-D32	61233.9(22)	13.913	2-D13 @170
M	OK	4324445(30)	17.380	3-D32	6929762(2)	27.122	4-D32	52166.6(22)	9.6307	2-D13 @260
J	OK	1.7E+07(13)	79.079	10-D32	8538202(13)	36.544	5-D32	61007.6(13)	13.823	2-D13 @170

*.MEMB = 0, SECT = 358 (1B8, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(22)	63.915	8-D32	7208234(22)	28.764	4-D32	54432.6(22)	10.932	2-D13 @170
M	OK	3741418(13)	15.118	2-D32	6385425(2)	24.844	4-D32	45763.4(13)	7.1527	2-D13 @300
J	OK	1.5E+07(13)	67.634	9-D32	7482836(13)	31.549	4-D32	55325.9(13)	11.433	2-D13 @170

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 359 (1B9, RECT), Span = 440.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	5721748(22)	31.943	4-D32	4764024(30)	25.953	4-D32	33263.1(22)	7.8649	2-D13 @130
M	OK	3287201(30)	17.304	3-D32	3109155(22)	16.303	3-D32	32360.5(13)	7.3937	2-D13 @260
J	OK	7020746(13)	40.663	5-D32	5357982(37)	29.629	4-D32	35105.2(13)	8.8267	2-D13 @130

*.MEMB = 0, SECT = 360 (1B10, RECT), Span = 440.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2082799(2)	10.688	2-D32	1041400(2)	8.0475	2-D32	13475.5(2)	3.7500	2-D13 @130
M	OK	730063(2)	5.1773	2-D32	520700(2)	5.1773	2-D32	10369.5(2)	3.7500	2-D13 @260
J	OK	775035(2)	5.1773	2-D32	520700(2)	8.0475	2-D32	7531.10(2)	0.0000	2-D13 @130

*.MEMB = 0, SECT = 361 (1B11, RECT), Span = 900.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6274863(2)	28.648	4-D32	3137432(2)	15.465	2-D32	37474.0(2)	6.1039	2-D13 @150
M	OK	1568716(2)	10.077	2-D32	4962735(2)	22.223	3-D32	26046.1(2)	4.1667	2-D13 @300
J	OK	3779241(2)	16.646	3-D32	1904956(2)	11.762	2-D32	28653.2(2)	4.1667	2-D13 @150

*.MEMB = 0, SECT = 362 (1B12, RECT), Span = 814.678
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2474830(20)	10.714	2-D32	1538244(11)	10.500	2-D32	18384.3(2)	4.1667	2-D13 @150
M	OK	989278(11)	7.7080	2-D32	2081544(11)	10.500	2-D32	13292.9(11)	4.1667	2-D13 @300
J	OK	3957113(11)	17.472	3-D32	1978556(11)	11.762	2-D32	21300.8(11)	4.1667	2-D13 @150

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 363 (1B13, RECT), Span = 800.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3210314(2)	14.032	2-D32	1605157(2)	10.500	2-D32	19840.4(2)	4.1667	2-D13 @150
M	OK	802579(2)	5.2368	2-D32	2103190(2)	10.500	2-D32	14104.4(2)	4.1667	2-D13 @300
J	OK	1796134(2)	10.278	2-D32	1114235(2)	10.500	2-D32	16515.9(2)	4.1667	2-D13 @150

*.MEMB = 0, SECT = 364 (1B14, RECT), Span = 1340.00
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(2)	41.586	6-D32	5994174(2)	23.256	3-D32	61574.4(2)	8.2674	2-D13 @200
M	OK	2997087(2)	15.169	2-D32	1.0E+07(2)	34.710	5-D32	41056.9(2)	5.0000	2-D13 @300
J	OK	1.1E+07(2)	38.171	5-D32	5538122(2)	19.554	3-D32	49997.4(2)	5.0000	2-D13 @200

*.MEMB = 0, SECT = 401 (2G1, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	63.008	8-D32	9922635(28)	40.209	5-D32	39796.0(20)	15.712	2-D13 @160
M	OK	5608647(35)	21.642	3-D32	6432237(11)	25.038	4-D32	34665.0(11)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	60.835	8-D32	9099003(35)	36.506	5-D32	39869.0(12)	15.740	2-D13 @160

*.MEMB = 0, SECT = 402 (2G2, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	63.427	8-D32	1.0E+07(28)	41.682	6-D32	44090.1(20)	17.407	2-D13 @140
M	OK	6279569(28)	24.404	3-D32	7000806(20)	27.421	4-D32	37967.7(20)	4.1667	2-D13 @300
J	OK	1.5E+07(11)	67.941	9-D32	1.1E+07(35)	46.481	6-D32	43592.9(11)	17.327	2-D13 @140

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 403 (2G3, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.9E+07(20)	89.405	11-D32	9779202(28)	39.559	5-D32	45890.9(20)	18.340	2-D13 @130
M	OK	5874149(35)	22.730	3-D32	7038880(11)	27.582	4-D32	42606.2(20)	5.9308	2-D13 @300
J	OK	1.8E+07(11)	81.990	11-D32	8845542(11)	39.437	5-D32	45961.6(12)	18.368	2-D13 @130

*.MEMB = 0, SECT = 404 (2G4, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(20)	80.362	10-D32	1.1E+07(28)	47.128	6-D32	54345.7(20)	21.718	2-D13 @110
M	OK	6285443(28)	24.428	4-D32	7005672(11)	27.442	4-D32	48125.6(11)	8.0668	2-D13 @300
J	OK	1.8E+07(11)	81.589	11-D32	1.1E+07(35)	47.151	6-D32	54388.8(12)	21.833	2-D13 @110

*.MEMB = 0, SECT = 405 (2G5, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(20)	72.422	9-D32	1.2E+07(28)	49.593	7-D32	47082.1(20)	18.714	2-D13 @130
M	OK	6748988(28)	26.362	4-D32	7373854(20)	29.002	4-D32	40959.7(20)	5.2936	2-D13 @300
J	OK	1.6E+07(11)	75.683	10-D32	1.2E+07(35)	52.243	7-D32	47045.1(11)	18.801	2-D13 @130

*.MEMB = 0, SECT = 406 (2G6, RECT), Span = 930.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.6E+07(20)	103.65	13-D32	1.8E+07(28)	64.747	8-D32	73170.4(20)	25.562	2-D13 @90
M	OK	1.0E+07(35)	34.104	5-D32	1.2E+07(11)	39.895	5-D32	64279.9(11)	8.1315	2-D13 @300
J	OK	2.6E+07(11)	102.44	13-D32	1.7E+07(35)	59.175	8-D32	73379.1(12)	25.635	2-D13 @90

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 407 (2G7, RECT), Span = 870.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.5E+07(20)	101.07	13-D32	1.8E+07(28)	64.734	8-D32	76677.5(20)	26.788	2-D13 @90
M	OK	9997961(28)	34.013	5-D32	1.1E+07(11)	37.454	5-D32	66219.7(11)	8.7911	2-D13 @280
J	OK	2.6E+07(11)	102.12	13-D32	1.8E+07(35)	64.696	8-D32	77082.8(11)	26.929	2-D13 @90

*.MEMB = 0, SECT = 408 (2G8, RECT), Span = 870.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.6E+07(20)	105.77	13-D32	1.9E+07(28)	69.154	9-D32	80625.6(20)	28.167	2-D13 @90
M	OK	1.1E+07(28)	38.993	5-D32	1.3E+07(20)	43.363	6-D32	69762.6(20)	9.9958	2-D13 @250
J	OK	2.8E+07(11)	112.77	14-D32	2.0E+07(35)	76.385	10-D32	80591.4(11)	28.243	2-D13 @80

*.MEMB = 0, SECT = 409 (2G9, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(20)	56.992	8-D32	7433358(28)	29.256	4-D32	32263.7(20)	12.738	2-D13 @170
M	OK	4523494(35)	17.260	3-D32	5617474(11)	21.678	3-D32	29185.0(11)	4.1667	2-D13 @300
J	OK	1.3E+07(11)	55.987	7-D32	6747012(35)	26.354	4-D32	32310.2(12)	12.647	2-D13 @170

*.MEMB = 0, SECT = 410 (2G10, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	60.169	8-D32	9348954(28)	37.621	5-D32	42262.8(20)	16.685	2-D13 @150
M	OK	5598513(28)	21.600	3-D32	6236274(20)	24.224	3-D32	37146.7(11)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	63.738	8-D32	9879003(35)	40.011	5-D32	42251.4(12)	16.681	2-D13 @150

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 451 (2B1, RECT), Span = 440.000
*.Bc = 60.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(22)	79.035	10-D32	1.2E+07(30)	76.697	10-D32	62128.6(22)	33.145	2-D13 @70
M	OK	6590822(13)	36.065	5-D32	6404503(13)	34.924	5-D32	62962.6(13)	20.204	2-D13 @120
J	OK	1.4E+07(13)	84.565	11-D32	1.2E+07(37)	75.540	10-D32	65040.3(13)	34.975	2-D13 @70

*.MEMB = 0, SECT = 452 (2B2, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.0E+07(22)	90.798	12-D32	1.2E+07(30)	52.269	7-D32	57411.3(22)	23.132	2-D13 @100
M	OK	7358103(30)	28.935	4-D32	8013948(13)	31.749	4-D32	54507.1(13)	10.536	2-D13 @240
J	OK	2.0E+07(13)	93.877	12-D32	1.2E+07(37)	49.851	7-D32	57618.4(14)	23.216	2-D13 @100

*.MEMB = 0, SECT = 453 (2B3, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(22)	79.760	10-D32	8618705(22)	36.544	5-D32	50690.9(22)	9.7000	2-D13 @170
M	OK	6420093(21)	24.988	4-D32	7002821(13)	27.430	4-D32	44234.2(22)	6.5609	2-D13 @300
J	OK	8332178(13)	33.130	5-D32	5129807(37)	19.695	3-D32	40278.9(13)	5.0302	2-D13 @180

*.MEMB = 0, SECT = 454 (2B4, RECT), Span = 420.000
*.Bc = 60.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(22)	88.140	11-D32	1.4E+07(30)	83.847	11-D32	69092.4(22)	37.154	2-D13 @60
M	OK	7052787(37)	38.932	5-D32	7157309(13)	39.589	5-D32	67442.3(13)	22.543	2-D13 @110
J	OK	1.3E+07(13)	83.401	11-D32	1.3E+07(37)	78.716	10-D32	69386.2(13)	37.312	2-D13 @60

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 455 (2B5, RECT), Span = 440.000
*.Bc = 60.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(22)	76.307	10-D32	1.1E+07(30)	67.902	9-D32	64920.5(22)	34.634	2-D13 @70
M	OK	5732148(37)	30.879	4-D32	5848503(14)	31.571	4-D32	61874.0(22)	19.636	2-D13 @120
J	OK	1.2E+07(13)	76.364	10-D32	1.1E+07(37)	68.576	9-D32	63870.9(13)	34.074	2-D13 @70

*.MEMB = 0, SECT = 456 (2B6, RECT), Span = 900.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3.2E+07(22)	128.69	16-D32	1.8E+07(30)	64.756	8-D32	86083.1(22)	30.323	2-D13 @80
M	OK	1.1E+07(30)	37.607	5-D32	1.3E+07(13)	44.154	6-D32	83566.4(22)	14.689	2-D13 @170
J	OK	3.2E+07(13)	129.99	16-D32	1.8E+07(37)	63.855	8-D32	86184.4(14)	30.359	2-D13 @80

*.MEMB = 0, SECT = 457 (2B7, RECT), Span = 900.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.8E+07(22)	114.39	15-D32	1.4E+07(30)	53.731	7-D32	79248.2(22)	27.849	2-D13 @90
M	OK	9344413(37)	31.660	4-D32	1.1E+07(13)	37.090	5-D32	76731.5(22)	12.365	2-D13 @200
J	OK	2.8E+07(13)	112.73	14-D32	1.4E+07(37)	50.819	7-D32	79443.4(14)	27.841	2-D13 @90

*.MEMB = 0, SECT = 458 (2B8, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.5E+07(22)	68.600	9-D32	7676328(30)	31.549	4-D32	41196.6(22)	16.375	2-D13 @150
M	OK	5032230(37)	19.301	3-D32	5644131(13)	21.787	3-D32	39817.6(22)	4.8516	2-D13 @300
J	OK	1.5E+07(13)	67.442	9-D32	7465920(13)	31.549	4-D32	41217.5(14)	16.383	2-D13 @150

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 501 (3G1, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(20)	54.321	7-D32	8403086(28)	33.440	5-D32	36947.1(20)	14.462	2-D13 @170
M	OK	4885304(35)	18.709	3-D32	5544930(11)	21.381	3-D32	30613.6(20)	4.1667	2-D13 @300
J	OK	1.2E+07(11)	51.208	7-D32	7892785(35)	31.226	4-D32	36743.8(11)	14.382	2-D13 @170

*.MEMB = 0, SECT = 502 (3G2, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	52.935	7-D32	8570584(28)	34.173	5-D32	38461.5(20)	15.055	2-D13 @160
M	OK	5140100(28)	19.737	3-D32	5768167(20)	22.295	3-D32	32745.1(20)	4.1667	2-D13 @300
J	OK	1.3E+07(11)	54.542	7-D32	9180553(35)	36.869	5-D32	38085.6(11)	14.908	2-D13 @170

*.MEMB = 0, SECT = 503 (3G3, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(20)	75.170	10-D32	8593594(28)	36.544	5-D32	46011.4(20)	18.388	2-D13 @130
M	OK	5389249(35)	20.747	3-D32	6284971(11)	24.426	4-D32	41100.6(20)	5.3482	2-D13 @300
J	OK	1.6E+07(11)	71.057	9-D32	8494858(35)	33.841	5-D32	45946.5(12)	18.262	2-D13 @130

*.MEMB = 0, SECT = 504 (3G4, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(20)	72.530	9-D32	1.0E+07(28)	42.299	6-D32	48552.1(20)	19.298	2-D13 @130
M	OK	5751410(28)	22.226	3-D32	6480216(11)	25.238	4-D32	42750.9(11)	5.9868	2-D13 @300
J	OK	1.6E+07(11)	73.512	10-D32	1.0E+07(35)	42.216	6-D32	48673.5(12)	19.452	2-D13 @130

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 505 (3G5, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	62.074	8-D32	1.1E+07(28)	42.890	6-D32	41953.5(20)	16.563	2-D13 @150
M	OK	5972563(28)	23.135	3-D32	6314857(20)	24.550	4-D32	37315.5(11)	4.1667	2-D13 @300
J	OK	1.5E+07(11)	66.646	9-D32	1.1E+07(35)	43.065	6-D32	43032.0(11)	17.104	2-D13 @140

*.MEMB = 0, SECT = 506 (3G6, RECT), Span = 930.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.3E+07(20)	92.528	12-D32	1.6E+07(28)	56.378	7-D32	66395.4(20)	23.249	2-D13 @100
M	OK	8843184(35)	30.006	4-D32	1.0E+07(11)	34.935	5-D32	57500.9(11)	6.8823	2-D13 @300
J	OK	2.3E+07(11)	92.129	12-D32	1.5E+07(35)	53.277	7-D32	66528.8(11)	23.295	2-D13 @100

*.MEMB = 0, SECT = 507 (3G7, RECT), Span = 870.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.3E+07(20)	92.807	12-D32	1.7E+07(28)	60.163	8-D32	69756.2(20)	24.425	2-D13 @100
M	OK	9223303(28)	31.377	4-D32	1.0E+07(11)	34.596	5-D32	60672.7(11)	7.9608	2-D13 @300
J	OK	2.4E+07(11)	94.051	12-D32	1.6E+07(35)	59.328	8-D32	70434.8(11)	24.663	2-D13 @100

*.MEMB = 0, SECT = 508 (3G8, RECT), Span = 870.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.3E+07(20)	89.952	12-D32	1.7E+07(28)	60.503	8-D32	69417.8(20)	24.307	2-D13 @100
M	OK	9570220(28)	32.634	5-D32	1.0E+07(20)	34.788	5-D32	61772.5(11)	8.3347	2-D13 @300
J	OK	2.4E+07(11)	97.450	12-D32	1.7E+07(35)	60.754	8-D32	71534.6(11)	25.048	2-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 509 (3G9, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	52.460	7-D32	6818613(28)	26.654	4-D32	30194.4(20)	11.819	2-D13 @170
M	OK	4171693(35)	15.861	2-D32	5186172(11)	19.923	3-D32	26927.0(11)	4.1667	2-D13 @300
J	OK	1.2E+07(11)	51.079	7-D32	6220326(35)	25.838	4-D32	30252.3(12)	11.841	2-D13 @170

*.MEMB = 0, SECT = 510 (3G10, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	51.069	7-D32	8802487(28)	35.193	5-D32	37337.6(20)	14.615	2-D13 @170
M	OK	5253934(28)	20.198	3-D32	5537508(20)	21.351	3-D32	32941.6(11)	4.1667	2-D13 @300
J	OK	1.3E+07(11)	56.290	7-D32	8994147(35)	36.041	5-D32	38654.0(11)	15.130	2-D13 @160

*.MEMB = 0, SECT = 511 (3G11, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(20)	83.901	11-D32	9069980(20)	39.437	5-D32	44052.6(36)	17.684	2-D13 @140
M	OK	4889695(35)	18.727	3-D32	5859904(11)	22.671	3-D32	37675.7(11)	4.1667	2-D13 @300
J	OK	1.6E+07(11)	75.241	10-D32	8085086(11)	36.544	5-D32	42299.4(12)	16.904	2-D13 @150

*.MEMB = 0, SECT = 551 (3B1, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(22)	70.726	9-D32	1.3E+07(30)	68.818	9-D32	65181.2(22)	29.885	2-D13 @80
M	OK	7094796(13)	32.807	5-D32	6899957(37)	31.808	4-D32	65430.0(13)	18.534	2-D13 @130
J	OK	1.4E+07(13)	76.758	10-D32	1.3E+07(37)	69.983	9-D32	67371.4(13)	31.084	2-D13 @80

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 552 (3B2, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(22)	82.587	11-D32	1.1E+07(30)	44.921	6-D32	52376.5(22)	21.025	2-D13 @120
M	OK	6752832(30)	26.378	4-D32	7191620(13)	28.228	4-D32	49113.1(13)	8.4490	2-D13 @300
J	OK	1.9E+07(13)	85.844	11-D32	1.1E+07(37)	43.805	6-D32	52536.1(14)	21.089	2-D13 @120

*.MEMB = 0, SECT = 553 (3B3, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(22)	71.718	9-D32	7836984(22)	31.549	4-D32	45987.6(22)	7.7210	2-D13 @170
M	OK	5897225(37)	22.825	3-D32	6293375(13)	24.461	4-D32	39965.3(22)	4.9088	2-D13 @300
J	OK	7768742(13)	30.692	4-D32	4680920(37)	17.889	3-D32	37131.2(13)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 554 (3B4, RECT), Span = 420.000
*.Bc = 50.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(22)	66.959	9-D32	1.0E+07(30)	64.364	8-D32	53353.1(22)	28.883	2-D13 @80
M	OK	5290778(37)	28.821	4-D32	5449149(13)	29.789	4-D32	52783.3(13)	17.001	2-D13 @140
J	OK	1.1E+07(13)	66.087	9-D32	9835953(37)	61.021	8-D32	54446.4(13)	29.475	2-D13 @80

*.MEMB = 0, SECT = 555 (3B5, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(22)	66.544	9-D32	1.2E+07(30)	60.957	8-D32	63254.5(22)	29.002	2-D13 @80
M	OK	6078387(30)	27.669	4-D32	6256291(13)	28.556	4-D32	63791.3(13)	17.805	2-D13 @140
J	OK	1.3E+07(13)	68.331	9-D32	1.1E+07(37)	58.859	8-D32	66342.0(13)	30.418	2-D13 @80

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 556 (3B6, RECT), Span = 900.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3.0E+07(22)	120.30	15-D32	1.6E+07(30)	57.968	8-D32	84708.7(22)	29.768	2-D13 @80
M	OK	1.0E+07(30)	35.209	5-D32	1.1E+07(13)	39.341	5-D32	82125.4(13)	14.199	2-D13 @170
J	OK	3.0E+07(13)	122.20	16-D32	1.6E+07(37)	56.771	7-D32	85095.4(14)	29.975	2-D13 @80

*.MEMB = 0, SECT = 557 (3B7, RECT), Span = 900.000
*.Bc = 65.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.5E+07(22)	97.103	12-D32	1.2E+07(22)	42.878	6-D32	67291.5(22)	23.423	2-D13 @100
M	OK	8007876(30)	26.909	4-D32	9271488(13)	31.398	4-D32	64812.7(22)	8.3127	2-D13 @300
J	OK	2.5E+07(13)	97.661	12-D32	1.2E+07(13)	42.878	6-D32	67323.4(14)	23.434	2-D13 @100

*.MEMB = 0, SECT = 558 (3B8, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(22)	54.745	7-D32	6404892(22)	25.838	4-D32	36481.9(22)	14.280	2-D13 @170
M	OK	4038274(37)	15.333	2-D32	4545723(22)	17.349	3-D32	35121.8(22)	4.1667	2-D13 @300
J	OK	1.3E+07(13)	54.236	7-D32	6354560(13)	25.838	4-D32	36442.7(14)	14.264	2-D13 @170

*.MEMB = 0, SECT = 601 (4G1, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(20)	46.226	6-D32	6882717(28)	26.924	4-D32	31969.8(20)	12.372	2-D13 @180
M	OK	4063983(35)	15.435	2-D32	4839899(11)	18.527	3-D32	28221.9(11)	4.1667	2-D13 @300
J	OK	1.1E+07(11)	45.425	6-D32	6278674(35)	24.400	3-D32	32046.9(12)	12.402	2-D13 @180

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 602 (4G2, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(20)	44.200	6-D32	7185019(28)	28.200	4-D32	33997.7(20)	13.157	2-D13 @180
M	OK	4397926(28)	16.759	3-D32	4857270(20)	18.597	3-D32	30089.5(11)	4.1667	2-D13 @300
J	OK	1.2E+07(11)	48.123	6-D32	7406524(35)	29.141	4-D32	34077.5(12)	13.188	2-D13 @180

*.MEMB = 0, SECT = 603 (4G3, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.5E+07(20)	65.139	9-D32	7319692(20)	31.549	4-D32	40535.7(20)	16.112	2-D13 @150
M	OK	4493774(35)	17.141	3-D32	5526589(11)	21.307	3-D32	37089.9(20)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	64.215	8-D32	7235602(11)	28.764	4-D32	40569.7(12)	16.017	2-D13 @150

*.MEMB = 0, SECT = 604 (4G4, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	62.755	8-D32	8501448(28)	33.870	5-D32	42859.7(20)	16.921	2-D13 @150
M	OK	4799952(28)	18.366	3-D32	5833961(11)	22.565	3-D32	39743.1(12)	4.8228	2-D13 @300
J	OK	1.5E+07(11)	67.915	9-D32	8138594(35)	32.289	4-D32	43026.3(12)	17.102	2-D13 @140

*.MEMB = 0, SECT = 605 (4G5, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	51.795	7-D32	8394165(28)	33.401	5-D32	39221.0(20)	15.352	2-D13 @160
M	OK	4895176(28)	18.749	3-D32	5406271(11)	20.817	3-D32	34377.0(11)	4.1667	2-D13 @300
J	OK	1.3E+07(11)	56.496	7-D32	8229252(35)	32.682	5-D32	40053.7(11)	15.678	2-D13 @160

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 606 (4G6, RECT), Span = 930.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.0E+07(20)	75.466	10-D32	1.1E+07(28)	39.565	5-D32	53913.9(20)	18.722	2-D13 @130
M	OK	7128812(35)	23.914	3-D32	8229272(11)	27.807	4-D32	48740.5(20)	5.0000	2-D13 @300
J	OK	1.9E+07(11)	69.084	9-D32	1.0E+07(35)	34.462	5-D32	53949.8(12)	18.632	2-D13 @130

*.MEMB = 0, SECT = 607 (4G7, RECT), Span = 870.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(20)	67.489	9-D32	1.0E+07(28)	35.863	5-D32	54118.1(20)	18.690	2-D13 @130
M	OK	6260306(28)	20.882	3-D32	7293297(11)	24.492	4-D32	50583.1(11)	5.0000	2-D13 @300
J	OK	1.9E+07(11)	69.116	9-D32	1.0E+07(35)	35.211	5-D32	54201.2(12)	18.719	2-D13 @130

*.MEMB = 0, SECT = 608 (4G8, RECT), Span = 870.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.9E+07(20)	69.996	9-D32	1.1E+07(28)	38.323	5-D32	57757.0(20)	19.947	2-D13 @120
M	OK	7473464(28)	25.127	4-D32	8725921(20)	29.584	4-D32	53361.0(11)	5.4746	2-D13 @300
J	OK	2.0E+07(11)	77.189	10-D32	1.2E+07(35)	43.447	6-D32	57759.7(12)	20.058	2-D13 @120

*.MEMB = 0, SECT = 609 (4G9, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(20)	45.833	6-D32	5569245(20)	22.777	3-D32	25552.9(36)	9.8890	2-D13 @180
M	OK	3465231(35)	13.083	2-D32	4642668(11)	17.736	3-D32	23367.7(20)	4.1667	2-D13 @300
J	OK	1.1E+07(11)	45.814	6-D32	5567203(11)	22.777	3-D32	25656.8(27)	9.9292	2-D13 @180

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 610 (4G10, RECT), Span = 870.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.0E+07(20)	42.615	6-D32	7263028(28)	28.531	4-D32	34065.0(20)	13.183	2-D13 @180
M	OK	4475759(28)	17.069	3-D32	4695144(11)	17.946	3-D32	30206.7(11)	4.1667	2-D13 @300
J	OK	1.2E+07(11)	49.711	7-D32	7213812(35)	28.322	4-D32	36196.0(11)	14.168	2-D13 @170

*.MEMB = 0, SECT = 611 (4G11, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(20)	79.256	10-D32	8559129(20)	36.544	5-D32	40267.6(36)	16.092	2-D13 @150
M	OK	4279564(20)	17.380	3-D32	5201450(11)	19.985	3-D32	34338.9(12)	4.1667	2-D13 @300
J	OK	1.5E+07(11)	69.661	9-D32	7659992(11)	31.549	4-D32	38470.6(15)	15.291	2-D13 @160

*.MEMB = 0, SECT = 651 (4B1, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(22)	59.222	8-D32	1.1E+07(30)	55.814	7-D32	56332.8(22)	25.629	2-D13 @90
M	OK	6006009(13)	27.310	4-D32	5806059(37)	26.322	4-D32	56620.1(13)	14.617	2-D13 @170
J	OK	1.2E+07(13)	65.353	9-D32	1.1E+07(37)	56.023	7-D32	58632.4(13)	26.883	2-D13 @90

*.MEMB = 0, SECT = 652 (4B2, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(22)	76.894	10-D32	9288940(30)	37.353	5-D32	46990.6(22)	18.779	2-D13 @130
M	OK	5889371(30)	22.792	3-D32	6484720(13)	25.257	4-D32	45571.5(14)	7.0784	2-D13 @300
J	OK	1.7E+07(13)	79.603	10-D32	8882869(37)	36.544	5-D32	47167.9(14)	18.850	2-D13 @130

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 653 (4B3, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.5E+07(22)	66.899	9-D32	7417918(22)	31.549	4-D32	45733.1(22)	7.6198	2-D13 @170
M	OK	5144244(37)	19.754	3-D32	6310946(13)	24.534	4-D32	39414.2(22)	4.6955	2-D13 @300
J	OK	7238198(13)	28.426	4-D32	3984318(37)	15.579	2-D32	36355.5(13)	4.1667	2-D13 @180

*.MEMB = 0, SECT = 654 (4B4, RECT), Span = 420.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	8368427(22)	52.304	7-D32	7866650(30)	48.102	6-D32	42176.9(22)	22.713	2-D13 @110
M	OK	4055816(37)	21.724	3-D32	4192747(13)	22.530	3-D32	41349.8(13)	12.087	2-D13 @210
J	OK	8254186(13)	51.642	7-D32	7510486(37)	45.360	6-D32	42984.5(13)	23.148	2-D13 @100

*.MEMB = 0, SECT = 655 (4B5, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(22)	57.059	8-D32	9905235(30)	48.066	6-D32	57275.5(22)	26.057	2-D13 @90
M	OK	5486045(13)	24.753	4-D32	5223055(29)	23.477	3-D32	55312.2(13)	14.035	2-D13 @180
J	OK	1.2E+07(13)	60.754	8-D32	9741412(37)	47.128	6-D32	57437.7(13)	26.131	2-D13 @90

*.MEMB = 0, SECT = 656 (4B6, RECT), Span = 900.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.7E+07(22)	111.32	14-D32	1.4E+07(22)	50.093	7-D32	78172.0(22)	27.536	2-D13 @90
M	OK	7566825(30)	25.456	4-D32	1.3E+07(13)	44.331	6-D32	74227.2(22)	12.570	2-D13 @200
J	OK	2.8E+07(13)	112.41	14-D32	1.4E+07(13)	50.093	7-D32	78038.1(17)	27.489	2-D13 @90

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 657 (4B7, RECT), Span = 900.000
*.Bc = 60.000, Hc = 90.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.3E+07(22)	92.698	12-D32	1.2E+07(22)	42.070	6-D32	67196.5(22)	23.529	2-D13 @100
M	OK	6928970(30)	23.213	3-D32	9020051(13)	30.643	4-D32	64484.0(22)	9.2567	2-D13 @270
J	OK	2.4E+07(13)	95.075	12-D32	1.2E+07(13)	42.070	6-D32	67300.5(14)	23.566	2-D13 @100

*.MEMB = 0, SECT = 658 (4B8, RECT), Span = 900.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.1E+07(22)	45.807	6-D32	5566471(22)	22.777	3-D32	31171.6(22)	12.063	2-D13 @180
M	OK	2785129(37)	12.063	2-D32	4501882(2)	17.174	3-D32	29669.3(22)	4.1667	2-D13 @300
J	OK	1.1E+07(13)	45.230	6-D32	5505095(13)	22.777	3-D32	31147.4(17)	12.054	2-D13 @180

*.MEMB = 0, SECT = 701 (RG1, RECT), Span = 930.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6525972(20)	29.910	6-D25	3262986(20)	14.503	3-D25	19358.6(20)	8.6073	2-D13 @150
M	OK	2082798(11)	10.500	3-D25	4736664(11)	21.142	5-D25	18265.4(28)	4.1667	2-D13 @300
J	OK	8331191(11)	39.825	8-D25	4165595(11)	18.759	4-D25	19451.4(28)	8.7359	2-D13 @150

*.MEMB = 0, SECT = 702 (RG2, RECT), Span = 870.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6056077(20)	27.558	6-D25	3382275(28)	14.818	3-D25	18742.2(20)	8.3332	2-D13 @150
M	OK	2375188(28)	10.500	3-D25	2681536(20)	11.640	3-D25	17117.7(11)	4.1667	2-D13 @300
J	OK	6564662(11)	30.105	6-D25	3782361(35)	16.660	4-D25	18771.3(12)	8.3461	2-D13 @150

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 703 (RG3, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(20)	51.385	7-D32	6069322(20)	25.838	4-D32	38295.3(20)	14.990	2-D13 @160
M	OK	3536459(11)	13.841	2-D32	7692552(11)	30.364	4-D32	36143.4(20)	4.1667	2-D13 @300
J	OK	1.4E+07(11)	62.444	8-D32	7072918(11)	28.764	4-D32	37998.7(28)	15.002	2-D13 @160

*.MEMB = 0, SECT = 704 (RG4, RECT), Span = 870.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6896486(20)	31.791	7-D25	5223182(28)	23.477	5-D25	21927.1(20)	9.7493	2-D13 @150
M	OK	2779204(11)	12.079	3-D25	3022174(11)	13.177	3-D25	19622.9(11)	4.1667	2-D13 @300
J	OK	7556340(11)	35.202	7-D25	4889166(35)	21.870	5-D25	22096.3(12)	9.8245	2-D13 @150

*.MEMB = 0, SECT = 705 (RG5, RECT), Span = 870.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	5610244(36)	25.360	6-D25	5117461(12)	22.967	5-D25	16014.1(20)	7.1202	2-D13 @150
M	OK	2895882(11)	12.606	3-D25	2931787(11)	12.768	3-D25	15646.3(11)	4.1667	2-D13 @300
J	OK	6543584(11)	29.999	6-D25	4828214(35)	21.579	5-D25	17895.7(11)	7.9568	2-D13 @150

*.MEMB = 0, SECT = 706 (RG6, RECT), Span = 930.000
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(20)	82.167	11-D32	8866311(20)	39.437	5-D32	44108.4(23)	17.706	2-D13 @140
M	OK	4787559(20)	18.673	3-D32	6842864(11)	26.756	4-D32	45583.8(2)	7.0832	2-D13 @300
J	OK	4433156(20)	18.673	3-D32	4908001(20)	18.801	3-D32	28927.7(11)	11.195	2-D13 @180

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 709 (RG9, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	7787834(20)	36.882	8-D25	3893917(20)	18.759	4-D25	18071.2(36)	8.1160	2-D13 @150
M	OK	2985899(28)	13.013	3-D25	3939477(20)	17.390	4-D25	16687.5(28)	4.1667	2-D13 @300
J	OK	6971044(11)	32.172	7-D25	3485522(11)	16.769	4-D25	18793.5(28)	8.3560	2-D13 @150

*.MEMB = 0, SECT = 710 (RG10, RECT), Span = 870.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6321007(20)	28.879	6-D25	4104569(28)	18.160	4-D25	20266.7(20)	9.0110	2-D13 @150
M	OK	2499286(28)	10.823	3-D25	2779421(11)	12.080	3-D25	18407.6(11)	4.1667	2-D13 @300
J	OK	7006071(11)	32.352	7-D25	3968807(35)	17.526	4-D25	20400.0(12)	9.0703	2-D13 @150

*.MEMB = 0, SECT = 711 (RG11, RECT), Span = 1080.00
*.Bc = 50.000, Hc = 80.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.4E+07(20)	62.859	8-D32	7111238(20)	28.764	4-D32	33425.2(36)	13.196	2-D13 @170
M	OK	4229358(28)	16.090	2-D32	5250171(20)	20.183	3-D32	30488.8(20)	4.1667	2-D13 @300
J	OK	1.2E+07(11)	49.641	7-D32	5892232(11)	25.838	4-D32	33354.8(18)	13.056	2-D13 @170

*.MEMB = 0, SECT = 751 (RB1, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	7935715(22)	37.677	8-D25	7323451(30)	33.989	7-D25	39901.1(22)	17.741	2-D13 @140
M	OK	4059171(14)	17.948	4-D25	4678238(22)	20.864	5-D25	38214.6(22)	6.4332	2-D13 @300
J	OK	8327289(13)	39.804	8-D25	8166500(22)	38.927	8-D25	39566.3(13)	17.770	2-D13 @140

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 752 (RB2, RECT), Span = 900.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	9678021(22)	48.348	10-D25	4839010(22)	22.568	5-D25	27724.1(22)	12.630	2-D13 @150
M	OK	3157235(30)	13.791	3-D25	3647271(22)	16.036	4-D25	26427.1(22)	4.1667	2-D13 @300
J	OK	1.0E+07(13)	51.897	11-D25	5101351(13)	24.383	5-D25	27790.0(14)	12.726	2-D13 @150

*.MEMB = 0, SECT = 753 (RB3, RECT), Span = 900.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	8408732(22)	40.251	8-D25	4204366(22)	18.759	4-D25	26242.7(22)	4.1667	2-D13 @150
M	OK	2941082(37)	12.810	3-D25	3023414(13)	13.183	3-D25	21626.5(22)	4.1667	2-D13 @300
J	OK	3985092(13)	17.602	4-D25	2830786(37)	12.312	3-D25	19593.0(13)	4.1667	2-D13 @150

*.MEMB = 0, SECT = 754 (RB4, RECT), Span = 420.000
*.Bc = 45.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	5941087(22)	33.969	7-D25	5976163(13)	34.203	7-D25	30103.5(22)	15.931	2-D13 @130
M	OK	2841706(22)	14.815	3-D25	3351277(13)	17.666	4-D25	30427.1(13)	6.3843	2-D13 @260
J	OK	5821282(13)	33.172	7-D25	5266190(37)	29.053	6-D25	31764.9(13)	16.810	2-D13 @130

*.MEMB = 0, SECT = 755 (RB5, RECT), Span = 440.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	7497038(22)	34.892	7-D25	7258538(13)	33.652	7-D25	38809.4(22)	17.256	2-D13 @140
M	OK	3896703(14)	17.191	4-D25	3953258(13)	17.454	4-D25	40861.6(13)	7.6101	2-D13 @300
J	OK	8187002(13)	39.038	8-D25	6952091(37)	32.075	7-D25	42947.7(13)	19.288	2-D13 @130

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 756 (RB6, RECT), Span = 250.000
*.Bc = 50.000, Hc = 85.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(22)	52.993	7-D32	6731637(22)	26.032	4-D32	23228.9(22)	8.4985	2-D13 @190
M	OK	1.3E+07(13)	48.095	6-D32	3514760(13)	12.897	2-D32	24041.2(13)	4.1667	2-D13 @190
J	OK	1.4E+07(13)	55.740	7-D32	7029521(13)	26.032	4-D32	25003.3(13)	9.1477	2-D13 @190

*.MEMB = 0, SECT = 757 (RB7, RECT), Span = 1300.00
*.Bc = 50.000, Hc = 85.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	8284736(22)	30.437	4-D32	4142368(22)	15.628	2-D32	17247.7(38)	6.2436	2-D13 @190
M	OK	2109229(13)	10.226	2-D32	9365065(22)	34.776	5-D32	20825.0(2)	4.1667	2-D13 @300
J	OK	8436916(13)	31.042	4-D32	4218458(13)	15.628	2-D32	17238.6(34)	6.2403	2-D13 @190

*.MEMB = 0, SECT = 758 (RB8, RECT), Span = 900.000
*.Bc = 50.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3574096(22)	15.699	4-D25	2182161(30)	10.500	3-D25	10960.1(22)	4.8731	2-D13 @150
M	OK	1369856(22)	7.7975	2-D25	1467918(22)	8.3658	2-D25	8633.13(22)	0.0000	2-D13 @300
J	OK	3523605(13)	15.466	4-D25	2229877(37)	10.500	3-D25	10845.2(13)	4.8220	2-D13 @150

*.MEMB = 0, SECT = 801 (PG1, RECT), Span = 425.000
*.Bc = 35.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2429843(20)	16.597	6-D19	1214922(20)	7.9762	3-D19	12879.4(20)	8.4066	2-D13 @100
M	OK	1106732(20)	6.9898	3-D19	1009353(20)	6.3503	3-D19	11874.3(20)	2.9167	2-D13 @210
J	OK	1325879(28)	8.4480	3-D19	1802327(20)	11.716	5-D19	8041.74(20)	5.1606	2-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 802 (PG2, RECT), Span = 780.000
*.Bc = 35.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	4013689(20)	22.982	9-D19	2006845(20)	11.497	5-D19	12821.3(20)	6.9479	2-D13 @120
M	OK	1377839(20)	7.0262	3-D19	1501383(20)	7.6806	3-D19	11889.0(20)	2.9167	2-D13 @260
J	OK	1911823(11)	9.8866	4-D19	1744753(20)	8.9826	4-D19	10318.2(11)	5.3873	2-D13 @130

*.MEMB = 0, SECT = 803 (PG3, RECT), Span = 300.000
*.Bc = 35.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1048755(11)	6.2592	3-D19	2013131(11)	10.439	4-D19	19543.0(11)	10.204	2-D13 @130
M	OK	2527793(11)	13.295	5-D19	1048755(11)	6.2592	3-D19	21844.7(11)	4.0148	2-D13 @260
J	OK	4195021(11)	24.366	9-D19	2097511(11)	11.497	5-D19	22538.6(11)	12.214	2-D13 @120

*.MEMB = 0, SECT = 804 (PCG3, RECT), Span = 290.000
*.Bc = 35.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1351803(13)	8.6223	4-D19	675901(13)	5.5365	2-D19	6343.08(13)	2.9167	2-D13 @100
M	OK	910448(13)	5.7060	2-D19	337951(13)	3.6300	2-D19	5764.23(13)	2.9167	2-D13 @210
J	OK	337951(13)	3.6300	2-D19	337951(13)	3.7611	2-D19	3790.78(13)	0.0000	2-D13 @100

*.MEMB = 0, SECT = 811 (PB1, RECT), Span = 440.000
*.Bc = 35.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2118786(22)	13.968	5-D19	1589945(30)	10.242	4-D19	11747.5(22)	7.5387	2-D13 @100
M	OK	929375(22)	5.8289	3-D19	1011405(13)	6.3637	3-D19	11325.8(13)	2.9167	2-D13 @210
J	OK	2046129(13)	13.445	5-D19	1659288(37)	10.720	4-D19	12741.5(13)	8.1765	2-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 812 (PB2, RECT), Span = 420.000
*.Bc = 35.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1168193(13)	7.3960	3-D19	584097(13)	5.0925	2-D19	8034.48(14)	5.1559	2-D13 @100
M	OK	292048(13)	2.7603	2-D19	784194(13)	5.0925	2-D19	7520.43(14)	2.9167	2-D13 @210
J	OK	785388(37)	5.0925	2-D19	883956(13)	5.5343	2-D19	6850.39(22)	4.3961	2-D13 @100

*.MEMB = 0, SECT = 813 (Pb1, RECT), Span = 420.000
*.Bc = 30.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	190681(13)	1.8568	2-D19	206601(22)	3.7497	2-D19	3211.94(13)	0.0000	2-D13 @100
M	OK	47670.2(13)	1.8568	2-D19	283023(2)	2.3142	2-D19	2318.77(13)	0.0000	2-D13 @210
J	OK	155539(21)	1.8568	2-D19	261307(13)	3.7497	2-D19	2920.27(22)	0.0000	2-D13 @100

*.MEMB = 0, SECT = 1001 (b1a, RECT), Span = 900.000
*.Bc = 35.000, Hc = 55.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	4350006(22)	29.021	6-D25	2175003(22)	13.188	3-D25	23165.8(2)	6.4248	2-D13 @110
M	OK	1087502(22)	6.3527	2-D25	2565073(2)	15.136	3-D25	14805.5(22)	2.9167	2-D13 @240
J	OK	3547638(13)	22.341	5-D25	1773819(13)	11.424	3-D25	22367.8(2)	5.7607	2-D13 @110

*.MEMB = 0, SECT = 1002 (b0, RECT), Span = 425.000
*.Bc = 30.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2244073(20)	15.147	4-D22	1122036(20)	7.2908	2-D22	15146.9(2)	3.3854	2-D13 @100
M	OK	848350(20)	5.3333	2-D22	779919(11)	4.8878	2-D22	12528.1(2)	2.5000	2-D13 @210
J	OK	561018(20)	4.3650	2-D22	779919(11)	4.8878	2-D22	4244.28(11)	0.0000	2-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 1003 (b3, RECT), Span = 690.000
*.Bc = 35.000, Hc = 60.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2563772(20)	13.498	4-D22	1366639(11)	7.4369	2-D22	18657.5(2)	2.9167	2-D13 @130
M	OK	640943(20)	4.8682	2-D22	2068058(11)	10.740	3-D22	12079.6(2)	2.9167	2-D13 @260
J	OK	2003268(11)	10.385	3-D22	1221094(20)	6.2592	2-D22	17544.9(2)	2.9167	2-D13 @130

*.MEMB = 0, SECT = 1004 (b4, RECT), Span = 450.000
*.Bc = 35.000, Hc = 50.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2063248(13)	13.568	4-D22	1289973(2)	8.2072	3-D22	15936.3(2)	2.9167	2-D13 @100
M	OK	552742(13)	4.7984	2-D22	1289973(2)	8.2072	3-D22	13094.8(2)	2.9167	2-D13 @210
J	OK	1824883(22)	11.874	4-D22	1255741(2)	7.9783	3-D22	16015.8(2)	2.9167	2-D13 @100

*.MEMB = 0, SECT = 1005 (b1, RECT), Span = 930.000
*.Bc = 40.000, Hc = 65.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	0.00000(42)	7.8000	2-D25	5789352(2)	30.007	6-D25	28627.2(2)	5.4608	2-D13 @140
M	OK	0.00000(42)	7.8000	2-D25	7919763(2)	45.280	9-D25	18326.1(2)	3.3333	2-D13 @280
J	OK	0.00000(42)	7.8000	2-D25	5789352(2)	30.007	6-D25	28627.2(2)	5.4608	2-D13 @140

*.MEMB = 0, SECT = 1006 (4b2, RECT), Span = 870.000
*.Bc = 45.000, Hc = 65.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	0.00000(42)	8.7750	2-D25	5774599(2)	28.917	6-D25	30367.4(2)	5.0386	2-D13 @140
M	OK	0.00000(42)	8.7750	2-D25	7932306(2)	43.446	9-D25	19830.9(2)	3.7500	2-D13 @280
J	OK	0.00000(42)	8.7750	2-D25	5774599(2)	28.917	6-D25	30367.4(2)	5.0386	2-D13 @140

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 1007 (2b2, RECT), Span = 930.000
*.Bc = 45.000, Hc = 70.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	0.00000(42)	9.3856	2-D25	6291776(2)	29.046	6-D25	31178.4(2)	4.3605	2-D13 @150
M	OK	0.00000(42)	9.3856	2-D25	8604140(2)	43.045	9-D25	19891.3(2)	3.7500	2-D13 @300
J	OK	0.00000(42)	9.3856	2-D25	6291776(2)	29.046	6-D25	31178.4(2)	4.3605	2-D13 @150

*.MEMB = 0, SECT = 1008 (1b1, RECT), Span = 930.000
*.Bc = 45.000, Hc = 65.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	0.00000(42)	8.7750	2-D25	5276807(2)	26.156	6-D25	26207.1(2)	3.7500	2-D13 @140
M	OK	0.00000(42)	8.7750	2-D25	7213589(2)	38.387	8-D25	16660.5(2)	3.7500	2-D13 @280
J	OK	0.00000(42)	8.7750	2-D25	5276807(2)	26.156	6-D25	26207.1(2)	3.7500	2-D13 @140

*.MEMB = 0, SECT = 1009 (1b2, RECT), Span = 870.000
*.Bc = 45.000, Hc = 65.000
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	0.00000(42)	7.8885	2-D25	4551290(2)	22.240	5-D25	24057.4(2)	3.7500	2-D13 @140
M	OK	0.00000(42)	7.8885	2-D25	6246206(2)	32.094	7-D25	15577.7(2)	3.7500	2-D13 @280
J	OK	0.00000(42)	7.8885	2-D25	4551290(2)	22.240	5-D25	24057.4(2)	3.7500	2-D13 @140

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 201 (FG1, RECT), Span = 930.000
*.Bc = 70.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(19)	31.276	4-D32	1.5E+07(12)	35.555	5-D32	76164.9(12)	5.8333	2-D13 @250
M	OK	1.3E+07(19)	31.276	4-D32	5947612(19)	15.777	2-D32	78980.7(19)	5.8333	2-D13 @300
J	OK	5947612(19)	15.777	2-D32	2.4E+07(19)	42.820	6-D32	99984.7(19)	5.8333	2-D13 @250

*.MEMB = 0, SECT = 202 (FG2, RECT), Span = 870.000
*.Bc = 70.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	5064381(12)	13.218	2-D32	2.0E+07(12)	36.257	5-D32	83911.0(1)	5.8333	2-D13 @250
M	OK	7881273(1)	18.455	3-D32	5064381(12)	13.218	2-D32	65561.9(12)	5.8333	2-D13 @300
J	OK	6495012(12)	15.178	2-D32	1.4E+07(19)	31.999	4-D32	68530.6(19)	5.8333	2-D13 @250

*.MEMB = 0, SECT = 203 (FG3, RECT), Span = 1080.00
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.9E+07(19)	51.574	7-D32	6.8E+07(12)	133.78	17-D32	262248(12)	32.072	2-D13 @70
M	OK	2.9E+07(19)	51.574	7-D32	1.7E+07(12)	40.635	5-D32	176401(19)	15.534	2-D13 @160
J	OK	1.7E+07(12)	40.635	5-D32	6.3E+07(19)	120.97	15-D32	240348(19)	27.849	2-D13 @90

*.MEMB = 0, SECT = 204 (FG4, RECT), Span = 870.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(12)	30.561	4-D32	4.8E+07(12)	90.696	12-D32	190509(12)	18.348	2-D13 @130
M	OK	1.6E+07(1)	37.500	5-D32	1.2E+07(12)	30.561	4-D32	148660(12)	10.435	2-D13 @240
J	OK	1.5E+07(12)	35.417	5-D32	3.9E+07(19)	70.546	9-D32	166797(19)	13.769	2-D13 @180

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 205 (FG5, RECT), Span = 870.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(36)	28.280	4-D32	3.4E+07(12)	62.699	8-D32	125053(12)	6.6667	2-D13 @250
M	OK	2.0E+07(12)	40.635	5-D32	9722643(36)	22.788	3-D32	108019(12)	6.6667	2-D13 @300
J	OK	2.2E+07(27)	40.635	5-D32	3.1E+07(19)	55.191	7-D32	116761(19)	6.6667	2-D13 @250

*.MEMB = 0, SECT = 206 (FG6, RECT), Span = 930.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.8E+07(19)	50.793	7-D32	3.7E+07(12)	67.322	9-D32	174632(12)	10.986	2-D13 @230
M	OK	2.8E+07(19)	50.793	7-D32	1.3E+07(19)	31.139	4-D32	170916(19)	10.303	2-D13 @240
J	OK	1.3E+07(12)	31.581	4-D32	5.2E+07(19)	95.685	12-D32	217206(19)	18.812	2-D13 @130

*.MEMB = 0, SECT = 207 (FG7, RECT), Span = 870.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(19)	30.100	4-D32	4.5E+07(12)	81.943	11-D32	179874(12)	11.950	2-D13 @210
M	OK	1.6E+07(1)	36.769	5-D32	1.1E+07(12)	28.656	4-D32	139947(12)	8.3333	2-D13 @300
J	OK	1.2E+07(12)	28.656	4-D32	4.3E+07(19)	78.966	10-D32	178821(19)	11.756	2-D13 @210

*.MEMB = 0, SECT = 208 (FG8, RECT), Span = 870.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(19)	27.975	4-D32	4.4E+07(12)	80.375	10-D32	188966(12)	13.621	2-D13 @180
M	OK	2.0E+07(1)	46.872	6-D32	1.1E+07(12)	26.151	4-D32	149039(12)	8.3333	2-D13 @300
J	OK	1.9E+07(12)	44.172	6-D32	3.3E+07(19)	59.094	8-D32	161310(19)	8.5370	2-D13 @250

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 209 (FG9, RECT), Span = 1080.00
*.Bc = 70.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.8E+07(19)	35.555	5-D32	2.3E+07(12)	41.562	6-D32	93015.3(12)	5.8333	2-D13 @250
M	OK	1.8E+07(19)	35.555	5-D32	7709091(19)	18.306	3-D32	87814.3(19)	5.8333	2-D13 @300
J	OK	7709091(19)	18.306	3-D32	3.1E+07(19)	56.143	7-D32	115726(19)	6.4922	2-D13 @250

*.MEMB = 0, SECT = 210 (FG10, RECT), Span = 870.000
*.Bc = 70.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6975618(12)	18.306	3-D32	2.8E+07(12)	50.556	7-D32	104371(12)	5.8333	2-D13 @250
M	OK	1.3E+07(12)	31.702	4-D32	6975618(12)	18.306	3-D32	86389.4(12)	5.8333	2-D13 @300
J	OK	1.3E+07(12)	31.702	4-D32	2.0E+07(19)	35.555	5-D32	76631.6(19)	5.8333	2-D13 @250

*.MEMB = 0, SECT = 211 (FG11, RECT), Span = 870.000
*.Bc = 70.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6735784(20)	15.746	4-D25	1.1E+07(12)	26.954	6-D25	27298.3(12)	0.0000	2-D13 @200
M	OK	4323008(19)	10.070	2-D25	5362830(11)	12.511	3-D25	27298.3(12)	0.0000	2-D13 @300
J	OK	7232367(11)	16.919	4-D25	8531471(35)	19.997	4-D25	23891.3(35)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 212 (FG12, RECT), Span = 690.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	8911062(20)	20.945	5-D25	2.1E+07(12)	38.304	8-D25	54254.3(12)	5.0000	2-D13 @200
M	OK	5680845(19)	13.278	3-D25	1.2E+07(12)	27.947	6-D25	54254.3(12)	5.0000	2-D13 @200
J	OK	1.3E+07(11)	29.780	6-D25	1.2E+07(35)	27.229	6-D25	45190.6(11)	5.0000	2-D13 @200

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 213 (FG13, RECT), Span = 610.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1679609(19)	5.0138	2-D25	4638207(12)	10.821	3-D25	28376.9(12)	0.0000	2-D13 @200
M	OK	3921191(12)	9.1373	2-D25	1159552(12)	5.0138	2-D25	26199.6(12)	0.0000	2-D13 @200
J	OK	3921191(12)	9.1373	2-D25	4230673(35)	9.8637	2-D25	22340.1(35)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 214 (FG14, RECT), Span = 1380.00
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	6253004(35)	14.827	3-D25	2.4E+07(12)	42.275	9-D25	58878.5(12)	6.6667	2-D13 @200
M	OK	7706844(12)	18.016	4-D25	5901233(12)	14.827	3-D25	43251.4(12)	0.0000	2-D13 @300
J	OK	7706844(12)	18.016	4-D25	1.7E+07(35)	39.231	8-D25	48361.0(1)	6.6667	2-D13 @200

*.MEMB = 0, SECT = 215 (FG15, RECT), Span = 610.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3329807(36)	11.521	3-D25	1.3E+07(12)	30.476	7-D25	37878.4(12)	5.0000	2-D13 @200
M	OK	3501768(35)	11.521	3-D25	7518930(12)	17.630	4-D25	35701.1(12)	5.0000	2-D13 @200
J	OK	3322273(12)	11.521	3-D25	3322273(12)	11.521	3-D25	17381.1(35)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 216 (FG16, RECT), Span = 610.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1110720(1)	3.3551	2-D25	383951(20)	6.7266	2-D25	5840.57(1)	0.0000	2-D13 @200
M	OK	1576348(1)	3.6587	2-D25	191976(20)	3.3551	2-D25	5967.25(1)	0.0000	2-D13 @200
J	OK	1091401(1)	3.3551	2-D25	397107(35)	6.7266	2-D25	5967.25(1)	0.0000	2-D13 @200

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 251 (FB1, RECT), Span = 440.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.0E+07(22)	40.635	5-D32	1.8E+07(30)	40.635	5-D32	110135(22)	6.6667	2-D13 @250
M	OK	9739669(29)	22.829	3-D32	1.9E+07(21)	40.635	5-D32	140320(21)	8.9018	2-D13 @250
J	OK	1.7E+07(29)	40.314	5-D32	3.5E+07(21)	63.813	8-D32	142724(21)	9.3437	2-D13 @250

*.MEMB = 0, SECT = 252 (FB2, RECT), Span = 900.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.7E+07(21)	40.272	5-D32	3.8E+07(14)	70.212	9-D32	143170(14)	9.4255	2-D13 @250
M	OK	2.1E+07(1)	40.635	5-D32	1.4E+07(22)	34.077	5-D32	153065(21)	11.245	2-D13 @220
J	OK	1.3E+07(21)	32.852	5-D32	5.2E+07(21)	97.730	13-D32	171570(21)	14.922	2-D13 @170

*.MEMB = 0, SECT = 253 (FB3, RECT), Span = 900.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.2E+07(14)	28.243	4-D32	4.7E+07(14)	87.246	11-D32	158855(14)	12.409	2-D13 @200
M	OK	2.5E+07(1)	44.171	6-D32	1.2E+07(13)	28.844	4-D32	140351(14)	8.9073	2-D13 @280
J	OK	1.4E+07(14)	32.487	4-D32	1.5E+07(21)	34.235	5-D32	103614(21)	6.6667	2-D13 @250

*.MEMB = 0, SECT = 254 (FB4, RECT), Span = 420.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3.2E+07(38)	57.604	8-D32	3.9E+07(14)	70.726	9-D32	147005(14)	10.130	2-D13 @250
M	OK	2.0E+07(37)	40.635	5-D32	2.3E+07(14)	41.566	6-D32	144985(14)	9.7593	2-D13 @250
J	OK	1.8E+07(29)	40.635	5-D32	1.9E+07(21)	40.635	5-D32	135399(21)	7.9971	2-D13 @250

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 255 (FB5, RECT), Span = 440.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.9E+07(22)	40.635	5-D32	4.3E+07(14)	79.891	10-D32	185707(14)	17.245	2-D13 @140
M	OK	1.1E+07(21)	25.899	4-D32	2.4E+07(14)	43.124	6-D32	177898(14)	15.810	2-D13 @160
J	OK	1.8E+07(13)	40.635	5-D32	4.4E+07(21)	80.742	10-D32	184700(21)	17.060	2-D13 @140

*.MEMB = 0, SECT = 256 (FB6, RECT), Span = 900.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.4E+07(21)	50.793	7-D32	8.1E+07(14)	157.56	20-D32	333010(1)	40.915	2-D13 @60
M	OK	5.7E+07(1)	105.42	13-D32	2.0E+07(21)	49.502	7-D32	284066(14)	31.103	2-D13 @80
J	OK	2.4E+07(14)	50.793	7-D32	8.1E+07(21)	158.00	20-D32	330549(1)	40.456	2-D13 @60

*.MEMB = 0, SECT = 257 (FB7, RECT), Span = 900.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.9E+07(14)	47.297	6-D32	7.8E+07(14)	150.05	19-D32	330089(1)	40.291	2-D13 @60
M	OK	5.6E+07(1)	102.53	13-D32	1.9E+07(14)	47.297	6-D32	279371(14)	30.239	2-D13 @80
J	OK	3.1E+07(14)	56.063	7-D32	7.0E+07(21)	132.64	17-D32	306616(1)	35.750	2-D13 @70

*.MEMB = 0, SECT = 258 (FB8, RECT), Span = 900.000
*.Bc = 100.00, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.3E+07(21)	30.813	4-D32	4.4E+07(14)	80.438	10-D32	158716(14)	8.3333	2-D13 @250
M	OK	3.2E+07(1)	56.857	7-D32	1.1E+07(14)	26.151	4-D32	146775(21)	8.3333	2-D13 @300
J	OK	2.0E+07(14)	46.145	6-D32	4.1E+07(21)	75.035	10-D32	175510(1)	11.147	2-D13 @220

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 259 (FB9, RECT), Span = 440.000
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1.6E+07(22)	37.857	8-D25	1.4E+07(30)	33.112	7-D25	112175(22)	6.6667	2-D13 @200
M	OK	8850074(21)	21.108	5-D25	2.2E+07(21)	40.635	9-D25	122572(21)	6.6667	2-D13 @200
J	OK	8850074(21)	21.108	5-D25	3.5E+07(21)	64.649	13-D25	122572(21)	6.6667	2-D13 @200

*.MEMB = 0, SECT = 260 (FB10, RECT), Span = 440.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	1527381(1)	5.0138	2-D25	6109522(1)	14.290	3-D25	20964.9(1)	0.0000	2-D13 @200
M	OK	1527381(1)	5.0138	2-D25	3771714(1)	8.7868	2-D25	20054.5(1)	0.0000	2-D13 @200
J	OK	1527381(1)	5.0138	2-D25	1609723(1)	6.7266	2-D25	1968.98(21)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 261 (FB11, RECT), Span = 900.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2603866(1)	6.6598	2-D25	7870438(1)	18.466	4-D25	49518.9(1)	5.0000	2-D13 @200
M	OK	8613973(1)	20.236	4-D25	1967610(1)	6.6598	2-D25	39241.5(1)	5.0000	2-D13 @300
J	OK	4643182(12)	10.833	3-D25	2821272(35)	10.064	2-D25	30604.8(1)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 262 (FB12, RECT), Span = 814.678
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	4811694(20)	11.230	3-D25	2405847(20)	10.064	2-D25	11522.5(20)	0.0000	2-D13 @200
M	OK	3017867(19)	7.0215	2-D25	1202924(20)	5.0138	2-D25	11098.6(35)	0.0000	2-D13 @300
J	OK	4464472(11)	10.413	3-D25	2942478(35)	10.064	2-D25	11845.0(35)	0.0000	2-D13 @200

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

*.MEMB = 0, SECT = 263 (FB13, RECT), Span = 800.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	849475(1)	3.3551	2-D25	2621525(1)	6.7266	2-D25	15130.5(1)	0.0000	2-D13 @200
M	OK	3242023(1)	7.5459	2-D25	655381(1)	3.3551	2-D25	14212.1(1)	0.0000	2-D13 @300
J	OK	1833281(1)	4.2568	2-D25	838794(35)	6.7266	2-D25	9649.58(1)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 264 (FB14, RECT), Span = 1340.00
*.Bc = 80.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	3197084(21)	7.4307	2-D25	6534610(30)	15.253	4-D25	27503.1(1)	0.0000	2-D13 @200
M	OK	1.0E+07(1)	24.219	5-D25	1657308(37)	6.6850	2-D25	24816.3(1)	0.0000	2-D13 @300
J	OK	7562774(1)	17.676	4-D25	6629232(37)	15.476	4-D25	42742.7(1)	0.0000	2-D13 @200

*.MEMB = 0, SECT = 265 (Fb1, RECT), Span = 930.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	2.4E+07(1)	43.185	6-D32	3.6E+07(1)	68.104	9-D32	191611(1)	22.719	2-D13 @110
M	OK	3.0E+07(1)	55.627	7-D32	9981501(1)	25.154	4-D32	161508(1)	17.020	2-D13 @140
J	OK	1.6E+07(12)	30.476	4-D32	4.0E+07(1)	75.699	10-D32	223047(1)	28.799	2-D13 @80

*.MEMB = 0, SECT = 266 (Fb2, RECT), Span = 690.000
*.Bc = 60.000, Hc = 160.00
*.fc = 280.000, fy = 4200.00, fys = 4200.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	4315139(1)	11.521	3-D25	1.7E+07(1)	30.887	7-D25	51552.9(1)	5.0000	2-D13 @200
M	OK	5259985(1)	12.285	3-D25	8518230(1)	20.008	4-D25	46152.5(1)	5.0000	2-D13 @200
J	OK	4315139(1)	11.521	3-D25	4315139(1)	11.521	3-D25	28659.4(1)	0.0000	2-D13 @200

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB	Section Name	fc	fy	LCB	Pu	Mc	Ast	Vu	As-H
SECT	Bc	Hc	Height	fys	Rat-P	Rat-M	V-Rebar	Rat-V	H-Rebar
0	1C1, RT	280.000	4200.00	30	-123316	1.1E+07	146.52	52829.0	20.445
101	70.000	80.000	435.000	4200.00	0.939	0.928	18- 6-D32	0.346	5-D13 @100
0	1C2, RT	280.000	4200.00	12	170733	1.5E+07	130.24	35988.7	16.160
102	70.000	80.000	435.000	4200.00	0.969	0.963	16- 5-D32	0.254	5-D13 @100
0	1C3, RT	280.000	4200.00	38	79808.2	2.7E+07	195.36	101835	30.878
103	100.00	100.00	435.000	4200.00	0.861	0.847	24- 7-D32	0.486	5-D13 @100
0	1C4, RT	280.000	4200.00	29	269461	2.7E+07	162.80	115368	8.3333
104	100.00	100.00	435.000	4200.00	0.966	0.943	20- 6-D32	0.383	5-D13 @100
0	1C5, RT	280.000	4200.00	37	180459	2.4E+07	146.52	56951.3	17.269
105	100.00	100.00	435.000	4200.00	0.888	0.874	18- 6-D32	0.272	5-D13 @100
0	1C6, RT	280.000	4200.00	19	92563.5	2.7E+07	195.36	83875.0	25.432
106	100.00	100.00	435.000	4200.00	0.826	0.839	24- 7-D32	0.401	5-D13 @100
0	1C7, RT	280.000	4200.00	37	187134	2.2E+07	113.96	50252.6	15.237
107	100.00	100.00	435.000	4200.00	0.979	0.957	14- 5-D32	0.240	5-D13 @100
0	1C8, RT	280.000	4200.00	28	276615	2.3E+07	113.96	100690	8.3333
108	100.00	100.00	435.000	4200.00	0.927	0.936	14- 4-D32	0.344	5-D13 @100
0	1C9, RT	280.000	4200.00	35	58816.6	2.4E+07	162.80	83428.1	25.297
109	100.00	100.00	435.000	4200.00	0.862	0.861	20- 6-D32	0.398	5-D13 @100
0	1C10, RT	280.000	4200.00	30	41844.6	2.0E+07	146.52	73028.7	24.832
110	90.000	150.00	435.000	4200.00	0.898	0.877	18- 6-D32	0.274	5-D13 @70
0	2C1, RT	280.000	4200.00	30	-91616	7471149	101.40	32097.7	14.413
111	70.000	80.000	400.000	4200.00	0.890	0.909	20- 6-D25	0.227	5-D13 @100
0	2C2, RT	280.000	4200.00	27	83910.9	1.1E+07	91.260	53585.7	20.738
112	70.000	80.000	400.000	4200.00	0.908	0.904	18- 5-D25	0.351	5-D13 @100
0	2C3, RT	280.000	4200.00	37	37693.5	1.8E+07	131.82	82492.3	25.013
113	100.00	100.00	400.000	4200.00	0.812	0.805	26- 8-D25	0.394	5-D13 @100
0	2C4, RT	280.000	4200.00	30	161858	2.4E+07	162.24	117289	9.9781
114	100.00	100.00	400.000	4200.00	0.819	0.836	32- 9-D25	0.399	5-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB	Section Name	fc	fy	LCB	Pu	Mc	Ast	Vu	As-H
SECT	Bc	Hc	Height	fys	Rat-P	Rat-M	V-Rebar	Rat-V	H-Rebar
0	2C5, RT	280.000	4200.00	37	110300	2.2E+07	130.24	98253.6	29.792
115	100.00	100.00	400.000	4200.00	0.917	0.901	16- 5-D32	0.469	5-D13 @100
0	2C6, RT	280.000	4200.00	35	15430.9	8477675	101.40	35496.2	10.763
116	100.00	100.00	400.000	4200.00	0.487	0.478	20- 6-D25	0.169	5-D13 @100
0	2C7, RT	280.000	4200.00	37	132840	1.6E+07	101.40	69192.1	20.980
117	100.00	100.00	400.000	4200.00	0.736	0.742	20- 6-D25	0.330	5-D13 @100
0	2C8, RT	280.000	4200.00	37	191774	2.3E+07	152.10	117208	11.518
118	100.00	100.00	400.000	4200.00	0.826	0.836	30- 9-D25	0.406	5-D13 @100
0	2C9, RT	280.000	4200.00	35	46733.8	1.2E+07	101.40	48431.9	14.685
119	100.00	100.00	400.000	4200.00	0.632	0.617	20- 6-D25	0.231	5-D13 @100
0	2C10, RT	280.000	4200.00	30	35264.9	1.3E+07	141.96	55365.4	18.826
120	90.000	150.00	400.000	4200.00	0.565	0.561	28- 8-D25	0.208	5-D13 @70
0	3C1, RT	280.000	4200.00	30	-51938	7001640	81.120	30340.4	13.624
121	70.000	80.000	400.000	4200.00	0.909	0.922	16- 5-D25	0.215	5-D13 @100
0	3C2, RT	280.000	4200.00	36	50147.8	9640961	81.120	44334.0	17.157
122	70.000	80.000	400.000	4200.00	0.871	0.864	16- 5-D25	0.290	5-D13 @100
0	3C3, RT	280.000	4200.00	37	28548.5	1.6E+07	101.40	71611.9	21.714
123	100.00	100.00	400.000	4200.00	0.893	0.890	20- 6-D25	0.342	5-D13 @100
0	3C4, RT	280.000	4200.00	13	212071	2.1E+07	121.68	84527.9	25.630
124	100.00	100.00	400.000	4200.00	0.888	0.881	24- 7-D25	0.404	5-D13 @100
0	3C5, RT	280.000	4200.00	22	130182	1.9E+07	113.96	82782.5	25.101
125	100.00	100.00	400.000	4200.00	0.867	0.848	14- 5-D32	0.395	5-D13 @100
0	3C6, RT	280.000	4200.00	13	106819	1.2E+07	101.40	38005.1	11.524
126	100.00	100.00	400.000	4200.00	0.558	0.549	20- 6-D25	0.181	5-D13 @100
0	3C7, RT	280.000	4200.00	37	87880.0	1.4E+07	101.40	57511.9	17.439
127	100.00	100.00	400.000	4200.00	0.707	0.711	20- 6-D25	0.275	5-D13 @100
0	3C8, RT	280.000	4200.00	29	109329	3.0E+07	211.64	121148	36.734
128	100.00	100.00	400.000	4200.00	0.854	0.859	26- 8-D32	0.578	5-D13 @100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB SECT	Section Name Bc	Hc	fc Height	fy fys	LCB	Pu Rat-P	Mc Rat-M	Ast V-Rebar	Vu Rat-V	As-H H-Rebar
0	3C9, RT		280.000	4200.00	12	148544	1.6E+07	101.40	58627.8	17.777
129	100.00	100.00	400.000	4200.00		0.774	0.760	20- 6-D25	0.280 5-D13 @100	
0	3C10, RT		280.000	4200.00	30	28521.8	9491951	141.96	35625.7	12.500
130	90.000	150.00	400.000	4200.00		0.420	0.421	28- 8-D25	0.134 5-D13 @70	
0	4C1, RT		280.000	4200.00	30	-20989	6992673	81.120	31690.9	14.231
131	70.000	80.000	400.000	4200.00		0.866	0.846	16- 5-D25	0.224 5-D13 @100	
0	4C2, RT		280.000	4200.00	11	39003.6	1.1E+07	101.40	46867.1	18.138
132	70.000	80.000	400.000	4200.00		0.825	0.828	20- 6-D25	0.307 5-D13 @100	
0	4C3, RT		280.000	4200.00	21	41270.9	2.0E+07	152.10	75526.8	22.901
133	100.00	100.00	400.000	4200.00		0.774	0.777	30- 9-D25	0.361 5-D13 @100	
0	4C4, RT		280.000	4200.00	21	56660.5	2.3E+07	172.38	99974.4	30.314
134	100.00	100.00	400.000	4200.00		0.812	0.797	34-10-D25	0.477 5-D13 @100	
0	4C5, RT		280.000	4200.00	22	39794.3	1.3E+07	113.96	57878.7	17.550
135	100.00	100.00	400.000	4200.00		0.645	0.639	14- 5-D32	0.276 5-D13 @100	
0	4C6, RT		280.000	4200.00	11	38090.4	7083023	101.40	24235.3	8.3333
136	100.00	100.00	400.000	4200.00		0.370	0.362	20- 6-D25	0.116 5-D13 @100	
0	4C7, RT		280.000	4200.00	20	95243.3	2.0E+07	131.82	71600.2	21.710
137	100.00	100.00	400.000	4200.00		0.830	0.839	26- 7-D25	0.342 5-D13 @100	
0	4C9, RT		280.000	4200.00	37	14605.9	5775662	101.40	14928.5	8.3333
139	100.00	100.00	400.000	4200.00		0.322	0.318	20- 6-D25	0.071 5-D13 @100	
0	4C10, RT		280.000	4200.00	30	10614.3	1.5E+07	141.96	50853.6	17.291
140	90.000	150.00	400.000	4200.00		0.729	0.711	28- 8-D25	0.191 5-D13 @70	
0	BC1, RT		280.000	4200.00	30	-180988	1.2E+07	162.80	47137.5	20.235
141	70.000	80.000	360.000	4200.00		0.985	0.995	20- 6-D32	0.329 5-D13 @100	
0	BC2, RT		280.000	4200.00	30	-120234	1.1E+07	146.52	35561.2	14.247
142	70.000	80.000	360.000	4200.00		0.929	0.936	18- 6-D32	0.245 5-D13 @100	
0	BC3, RT		280.000	4200.00	37	50267.3	3.3E+07	227.92	102213	9.2321
143	100.00	100.00	360.000	4200.00		0.932	0.921	28- 8-D32	0.364 5-D13 @100	

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB	Section Name	fc	fy	LCB	Pu	Mc	Ast	Vu	As-H
SECT	Bc Hc	Height	fys		Rat-P	Rat-M	V-Rebar	Rat-V	H-Rebar
0	BC4, RT	280.000	4200.00	29	289193	3.4E+07	244.20	115983	8.3333
144	100.00 100.00	360.000	4200.00		0.928	0.912	30- 9-D32	0.381 5-D13 @100	
0	BC5, RT	280.000	4200.00	29	274234	3.0E+07	195.36	104006	8.3333
145	100.00 100.00	360.000	4200.00		0.924	0.947	24- 7-D32	0.348 5-D13 @100	
0	BC6, RT	280.000	4200.00	35	93956.8	2.3E+07	162.80	79177.9	8.3333
146	100.00 100.00	360.000	4200.00		0.826	0.820	20- 6-D32	0.257 5-D13 @100	
0	BC7, RT	280.000	4200.00	38	256353	3.0E+07	195.36	84643.1	8.3333
147	100.00 100.00	360.000	4200.00		0.959	0.945	24- 7-D32	0.289 5-D13 @100	
0	BC8, RT	280.000	4200.00	30	380785	3.1E+07	179.08	100233	8.3333
148	100.00 100.00	360.000	4200.00		0.895	0.913	22- 7-D32	0.328 5-D13 @100	
0	BC9, RT	280.000	4200.00	35	121250	2.1E+07	113.96	86867.4	8.3333
149	100.00 100.00	360.000	4200.00		0.938	0.944	14- 4-D32	0.274 5-D13 @100	
0	BC10, RT	280.000	4200.00	30	50470.2	3.1E+07	260.48	70192.0	12.500
150	90.000 150.00	360.000	4200.00		0.831	0.825	32- 9-D32	0.195 5-D13 @70	
0	BC11, RT	280.000	4200.00	21	72581.7	1.0E+07	97.680	44249.3	6.6667
151	70.000 80.000	360.000	4200.00		0.920	0.903	12- 4-D32	0.243 5-D13 @100	
0	BC12, RT	280.000	4200.00	35	27082.4	3074519	40.560	16692.1	5.0000
152	60.000 60.000	360.000	4200.00		0.695	0.682	8- 3-D25	0.135 5-D13 @100	
0	BC13, RT	280.000	4200.00	11	126530	1.5E+07	131.82	81482.5	12.384
153	80.000 80.000	360.000	4200.00		0.925	0.919	26- 7-D25	0.382 5-D13 @100	
0	BC14, RT	280.000	4200.00	12	41726.8	4196971	40.560	21629.1	5.0000
154	60.000 60.000	360.000	4200.00		0.937	0.916	8- 3-D25	0.175 5-D13 @100	
0	BC15, RT	280.000	4200.00	11	57112.2	7806712	81.120	25819.8	5.8333
155	60.000 70.000	360.000	4200.00		0.979	0.959	16- 5-D25	0.181 5-D13 @100	
0	BC16, RT	280.000	4200.00	11	103358	1.7E+07	162.80	71384.8	10.176
156	80.000 80.000	360.000	4200.00		0.947	0.943	20- 6-D32	0.341 5-D13 @100	
0	PC1, RT	280.000	4200.00	20	15608.6	3675080	60.840	8563.67	6.6667
161	70.000 80.000	380.000	4200.00		0.412	0.422	12- 4-D25	0.061 5-D13 @100	

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB	Section Name	fc	fy	LCB	Pu	Mc	Ast	Vu	As-H
SECT	Bc Hc	Height	fys		Rat-P	Rat-M	V-Rebar	Rat-V	H-Rebar
0	PC2, RT	280.000	4200.00	13	32245.7	2637210	60.840	10295.0	6.6667
162	70.000 80.000	380.000	4200.00		0.307	0.309	12- 4-D25	0.073 5-D13	@100
0	PC3, RT	280.000	4200.00	11	12563.7	5691694	101.40	12502.2	8.3333
163	100.00 100.00	380.000	4200.00		0.315	0.312	20- 6-D25	0.060 5-D13	@100
0	PC4, RT	280.000	4200.00	22	54327.6	7823633	101.40	59463.1	19.062
164	95.000 100.00	120.000	4200.00		0.404	0.412	20- 6-D25	0.300 5-D13	@100
0	PC10, RT	280.000	4200.00	11	22590.5	1.7E+07	141.96	12233.0	12.500
165	90.000 150.00	380.000	4200.00		0.429	0.431	28- 8-D25	0.046 5-D13	@70
0	PC0, RT	280.000	4200.00	11	-3693.7	4306772	57.400	20820.7	13.361
166	40.000 50.000	380.000	4200.00		1.14*	1.16*	20- 7-D19	0.395 5-D13	@100

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-LSD90] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz
	464	171	4C11, BB 400x400x16			400.000	400.000	1.00	1.00	1.00	1.00	-33459	0.00000	0.00000
OK	0.03	0.00	STKR400	2500.00	8	400.000	400.000		1.00	1.00	1.00	505879	6806250	6806250
	467	171	4C11, BB 400x400x16			400.000	400.000	1.00	1.00	1.00	1.00	-30302	0.00000	0.00000
OK	0.03	0.00	STKR400	2500.00	2	400.000	400.000		1.00	1.00	1.00	505879	6806250	6806250
	518	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-17034	4549815	3654.76
OK	0.33	0.39	SN400	2400.00	11	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	519	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-31256	8169802	-60465
OK	0.59	0.60	SN400	2400.00	11	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	520	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-27864	6256124	-1.3E+6
OK	0.64	0.33	SN400	2400.00	7	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	521	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-9361.8	728263	-808608
OK	0.18	0.06	SN400	2400.00	7	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	523	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-28696	-5.8E+6	-1.4E+6
OK	0.63	0.60	SN400	2400.00	7	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	524	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-27934	-6.3E+6	-1.4E+6
OK	0.67	0.33	SN400	2400.00	7	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	525	3151	RSC1			120.000	120.000	1.00	1.00	1.00	1.00	-9552.5	-745373	-880461
OK	0.19	0.06	SN400	2400.00	7	120.000	120.000		1.00	1.00	1.00	653715	+1.5E+7	6751991
	526	3152	RSC2			170.000	170.000	1.00	1.00	1.00	1.00	-2580.9	-46383	-2.7E+6
OK	0.82	0.50	SN400	2400.00	10	170.000	170.000		1.00	1.00	1.00	391357	7242538	3320561
	527	3152	RSC2			170.000	170.000	1.00	1.00	1.00	1.00	-14279	6024.10	689277
OK	0.23	0.06	SN400	2400.00	7	170.000	170.000		1.00	1.00	1.00	391357	7242538	3320561
	528	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	2021.36	-700072	-156905
OK	0.29	0.04	SS400	2500.00	10	870.000	870.000		1.00	1.00	1.00	299250	3782370	1620000
	529	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-4442.3	-696745	-113776
OK	0.27	0.04	SS400	2500.00	7	870.000	870.000		1.00	1.02	1.00	139214	3782370	1620000
	530	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	119.283	-828958	-156681
OK	0.32	0.04	SS400	2500.00	7	870.000	870.000		1.00	1.00	1.00	299250	3782370	1620000

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-LSD90] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz
	531	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.01	1.00	-11993	-888984	81384.4
OK	0.33	0.05	SS400	2500.00	7	870.000	870.000		1.00	1.06	1.00	139214	3782370	1620000
	532	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.01	1.00	-8722.7	-776148	104936
OK	0.30	0.04	SS400	2500.00	7	870.000	870.000		1.00	1.05	1.00	139214	3782370	1620000
	533	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-1305.2	-899487	144625
OK	0.33	0.05	SS400	2500.00	7	870.000	870.000		1.00	1.01	1.00	139214	3782370	1620000
	538	3002	RSB2, RH 588x300x12x20			1803.64	901.822	1.00	1.00	1.00	1.00	-8802.2	-5.8E+6	-29889
OK	0.68	0.15	SN400	2400.00	8	450.911	450.911		1.00	1.01	1.00	265319	8963494	1947240
	539	3001	RSB1, RH 700x300x13x24			1803.64	1803.64	1.00	1.00	1.05	1.00	-65052	-8.1E+6	-19130
OK	0.81	0.19	SN400	2400.00	2	450.911	450.911		1.00	1.06	1.00	256856	+1.3E+7	2336040
	540	3001	RSB1, RH 700x300x13x24			1803.64	1803.64	1.00	1.00	1.03	1.00	-36694	-8.5E+6	-9107.6
OK	0.73	0.21	SN400	2400.00	2	450.911	450.911		1.00	1.03	1.00	256856	+1.3E+7	2336040
	541	3002	RSB2, RH 588x300x12x20			901.822	901.822	1.00	1.00	1.00	1.00	-4855.0	-779010	240024
OK	0.22	0.06	SN400	2400.00	7	450.911	450.911		1.00	1.01	1.00	265319	8963494	1947240
	550	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-14.888	138294	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	551	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-52.331	138299	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	552	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-47.766	138299	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	553	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-50.139	138299	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	554	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-78.290	138303	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	555	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-55.886	138300	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000
	567	3002	RSB2, RH 588x300x12x20			901.822	901.822	1.00	1.00	1.00	1.00	-4878.0	-789001	240056
OK	0.22	0.06	SN400	2400.00	7	450.911	450.911		1.00	1.01	1.00	265319	8963494	1947240

*.PROJECT : 長億高中圖書館
*.UNIT SYSTEM : kgf, cm

[TWN-LSD90] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

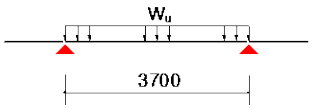
CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz
	572	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.01	1.00	-11985	119918	0.00000
OK	0.07	0.01	SS400	2500.00	10	870.000	870.000		1.00	1.06	1.00	139214	3782370	1080000
	573	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.01	1.00	-6481.0	119279	0.00000
OK	0.05	0.01	SS400	2500.00	10	870.000	870.000		1.00	1.03	1.00	139214	3782370	1080000
	574	3101	RSG1, RH 390x300x10x16			870.000	870.000	1.00	1.00	1.00	1.00	-841.41	138404	0.00000
OK	0.04	0.01	SS400	2500.00	1	870.000	870.000		1.00	1.00	1.00	139214	3782370	1080000

MIDAS/Set **Slab Design [FSO]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 3.70 m (Both End Fixed)
Slab Depth : 500 mm (c_c = 75 mm)



2. Applied Loads

Dead Load : W_d = 9.90 tf/m²
Live Load : W_l = 0.00 tf/m²
W_u = 1.4*W_d+1.7*W_l=13.86 tf/m²

3. Check Mnimum Slab Thk.

h_{min}= L/28 = 132 mm
h = h_{min}*(0.4+f_y/7000) = 106 mm
Thk = 500 > Req'd Thk = 106 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	17.25 (W_uL^2/11)	11.86 (W_uL^2/16)	0.00	
ρ (%)	0.402	0.274	0.000	0.200
A_{st} (cm²/m)	16.78	11.45	0.00	10.00
#4	@ 70	@ 110	@ 450	@ 120
#4+#5	@ 90	@ 140	@ 450	@ 160
#5	@ 110	@ 170	@ 450	@ 190
#5+#6	@ 140	@ 210	@ 450	@ 240

5. Check Shear Stresses

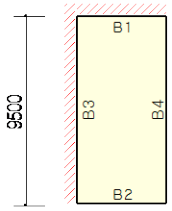
Strength Reduction Factor Φ = 0.850
V_{ux} = 25.64 < ΦV_c = 31.50 tf/m ..... O.K.

MIDAS/Set **Slab Design [FS1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

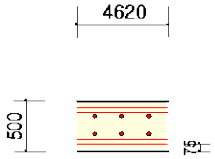
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4620 * 9500 * 500 mm (c_c = 75 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm



2. Applied Loads

Dead Load : W_d = 9.90 tf/m²
Live Load : W_l = 0.00 tf/m²
W_u = 1.4*W_d+1.7*W_l=13.86 tf/m²



3. Check Minimum Slab Thk.

α_m = (4.80+7.47+9.87+14.20)/4 = 9.0857
β = L_{ny}/L_{nx} = 2.2775
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 154 mm
Thk = 500 > Req'd Thk = 154 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.090	0.045	0.068	0.049	0.025	0.037	
M_u (tf-m/m)	18.20	9.10	13.75	9.91	5.06	7.48	
ρ (%)	0.423	0.209	0.317	0.242	0.123	0.182	0.200
A_{st} (cm²/m)	17.69	8.73	13.28	9.83	4.98	7.39	10.00
#4	@ 70	@140	@ 90	@130	@250	@170	@120
#4+#5	@ 90	@140	@120	@160	@320	@220	@160
#5	@110	@180	@140	@200	@390	@260	@190
#5+#6	@130	@220	@180	@240	@450	@320	@240

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 24.88 < ΦV_c = 31.50 tf/m ..... O.K.

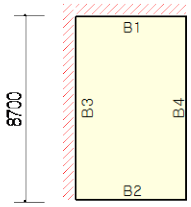
Long Direction Shear
V_{uy} = 3.18 < ΦV_c = 30.42 tf/m ..... O.K.

MIDAS/Set **Slab Design [FS2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

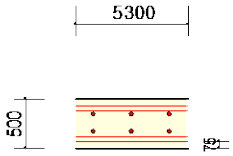
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 5300 * 8700 * 500 mm (c_c = 75 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm



2. Applied Loads

Dead Load : W_d = 9.90 tf/m²
Live Load : W_l = 0.00 tf/m²
W_u = 1.4*W_d+1.7*W_l=13.86 tf/m²



3. Check Minimum Slab Thk

α_m = (5.24+8.10+8.60+12.62)/4 = 8.6410
β = L_{ny}/L_{nx} = 1.7556
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 152 mm
Thk = 500 > Req'd Thk = 152 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.082	0.041	0.062	0.049	0.025	0.037	
M_u (tf-m/m)	22.92	11.46	17.33	13.75	7.02	10.38	
ρ (%)	0.536	0.263	0.402	0.338	0.171	0.254	0.200
A_{st} (cm²/m)	22.43	11.03	16.82	13.72	6.93	10.31	10.00
#4	@ 50	@110	@ 70	@ 90	@180	@120	@120
#4+#5	@ 70	@110	@ 90	@110	@230	@150	@160
#5	@ 80	@140	@110	@140	@280	@190	@190
#5+#6	@100	@180	@140	@170	@340	@230	@240

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear
V_{ux} = 28.33 < ΦV_c = 31.50 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 5.02 < ΦV_c = 30.42 tf/m ..... O.K.

MIDAS/Set **Slab Design [FS3]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

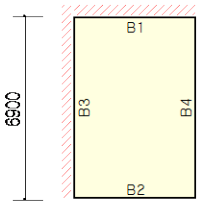
f_y = 2800 kgf/cm²

Slab Dim. : 4650 * 6900 * 500 mm (c_c = 75 mm)

Edge Beam Size :

B1 = 80 X160, B2 = 80 X160 cm

B3 = 80 X160, B4 = 80 X160 cm

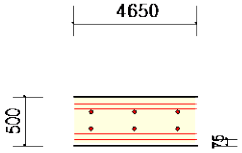


2. Applied Loads

Dead Load : W_d = 9.90 tf/m²

Live Load : W_l = 0.00 tf/m²

W_u = 1.4*W_d+1.7*W_l=13.86 tf/m²



3. Check Mnimum Slab Thk

α_m = (6.61+10.00+9.81+14.12)/4 = 10.1332

β = L_{ny}/L_{nx} = 1.5844

h_{min} = 90 mm

h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 121 mm

Thk = 500 > Req'd Thk = 121 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.076	0.038	0.057	0.049	0.025	0.037	
M_u (tf-m/m)	15.58	7.82	11.80	10.07	5.14	7.60	
ρ (%)	0.360	0.179	0.272	0.246	0.125	0.185	0.200
A_{st} (cm²/m)	15.08	7.49	11.37	9.98	5.06	7.51	10.00
#4	@ 80	@170	@110	@120	@250	@170	@120
#4+#5	@100	@170	@140	@160	@320	@210	@160
#5	@130	@210	@170	@190	@390	@260	@190
#5+#6	@150	@260	@210	@230	@450	@310	@240

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear

V_{ux} = 23.08 < ΦV_c = 31.50 tf/m ..... O.K.

Long Direction Shear

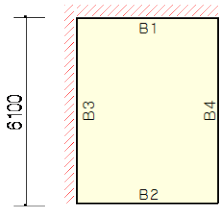
V_{uy} = 5.70 < ΦV_c = 30.42 tf/m ..... O.K.

MIDAS/Set **Slab Design [FS4]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

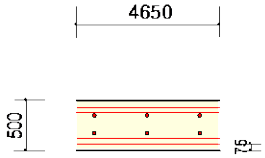
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : $f_c' = 280$ kgf/cm²
$f_y = 2800$ kgf/cm²
Slab Dim. : 4650 * 6100 * 500 mm (c_c = 75 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm



2. Applied Loads

Dead Load : $W_d = 9.90$ tf/m²
Live Load : $W_l = 0.00$ tf/m²
$W_u = 1.4*W_d+1.7*W_l = 13.86$ tf/m²



3. Check Minimum Slab Thk.

$\alpha_m = (7.47+11.16+9.81+14.12)/4 = 10.6396$
$\beta = L_{ny}/L_{nx} = 1.3766$
$h_{min} = 90$ mm
$h = l_n(800+f_y/14)/(36000+9000\beta) = 109$ mm
Thk = 500 > Req'd Thk = 109 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor $\Phi = 0.900$

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.069	0.035	0.052	0.049	0.025	0.037	
M_u (tf-m/m)	14.21	7.18	10.77	10.07	5.14	7.60	
ρ (%)	0.328	0.164	0.247	0.246	0.125	0.185	0.200
A_{st} (cm²/m)	13.73	6.87	10.36	9.98	5.06	7.51	10.00
#4	@ 90	@180	@120	@120	@250	@170	@120
#4+#5	@110	@180	@150	@160	@320	@210	@160
#5	@140	@230	@190	@190	@390	@260	@190
#5+#6	@170	@280	@230	@230	@450	@310	@240

5. Check Shear Stresses

Strength Reduction Factor $\Phi = 0.850$
Short Direction Shear
$V_{ux} = 20.92 < \Phi V_c = 31.50$ tf/m ..... O.K.

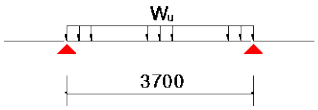
Long Direction Shear
$V_{uy} = 7.93 < \Phi V_c = 30.42$ tf/m ..... O.K.

MIDAS/Set **Slab Design [BS0]**

Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 3.70 m (Both End Fixed)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.50 tf/m²
W_u = 1.4*W_d+1.7*W_l= 2.07 tf/m²

3. Check Minimum Slab Thk

h_{min} = L/28 = 132 mm
h = h_{min}*(0.4+f_y/7000) = 106 mm
Thk = 200 > Req'd Thk = 106 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	2.57 (W_uL^2/11)	1.77 (W_uL^2/16)	0.00	
ρ (%)	0.386	0.263	0.000	0.200
A_{st} (cm²/m)	6.35	4.33	0.00	4.00
#3	@ 110	@ 160	@ 450	@ 170
#3+#4	@ 150	@ 230	@ 450	@ 240
#4	@ 200	@ 290	@ 450	@ 320
#4+#5	@ 250	@ 370	@ 450	@ 410

5. Check Shear Stresses

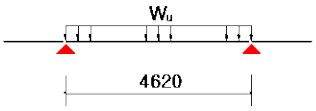
Strength Reduction Factor Φ = 0.850
V_{ux} = 3.82 < ΦV_c = 12.40 tf/m ..... O.K.

MIDAS/Set **Slab Design [BS1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 4.62 m (Both End Fixed)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.50 tf/m²
W_u = 1.4*W_d+1.7*W_l= 2.07 tf/m²

3. Check Minimum Slab Thk

h_{min} = L/28 = 165 mm
h = h_{min}*(0.4+f_y/7000) = 132 mm
Thk = 200 > Req'd Thk = 132 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	4.01 (W_uL^2/11)	2.76 (W_uL^2/16)	0.00	
ρ (%)	0.622	0.423	0.000	0.200
A_{st} (cm²/m)	10.14	6.88	0.00	4.00
#4	@ 120	@ 180	@ 450	@ 320
#4+#5	@ 160	@ 230	@ 450	@ 410
#5	@ 190	@ 280	@ 450	@ 450
#5+#6	@ 230	@ 340	@ 450	@ 450

5. Check Shear Stresses

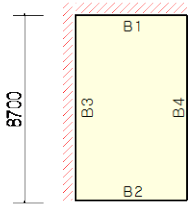
Strength Reduction Factor Φ = 0.850
V_{ux} = 4.77 < ΦV_c = 12.28 tf/m ..... O.K.

MIDAS/Set **Slab Design [BS2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

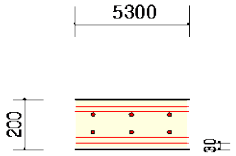
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 5300 * 8700 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.50 tf/m²
W_u = 1.4*W_d+1.7*W_l = 2.07 tf/m²



3. Check Minimum Slab Thk.

α_m = (68.89+108.41+113.09+168.83)/4 = 114.8041
β = L_{ny}/L_{nx} = 1.7556
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 152 mm
Thk = 200 > Req'd Thk = 152 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.082	0.041	0.062	0.049	0.025	0.037	
M_u (tf-m/m)	3.41	1.71	2.58	2.05	1.05	1.55	
ρ (%)	0.512	0.252	0.384	0.342	0.173	0.257	0.200
A_{st} (cm²/m)	8.45	4.16	6.34	5.33	2.69	4.00	4.00
#3	@ 80	@170	@110	@130	@260	@170	@170
#3+#4	@110	@170	@150	@180	@360	@240	@240
#4	@150	@230	@200	@230	@450	@310	@320
#4+#5	@190	@300	@250	@290	@450	@390	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 4.22 < ΦV_c = 12.40 tf/m ..... O.K.

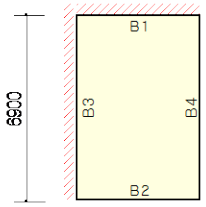
Long Direction Shear
V_{uy} = 0.75 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [BS3]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

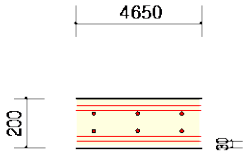
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4650 * 6900 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.50 tf/m²
W_u = 1.4*W_d+1.7*W_l= 2.07 tf/m²



3. Check Minimum Slab Thk.

α_m = (86.86+133.75+128.89+188.97)/4 = 134.6184
β = L_{ny}/L_{nx} = 1.5844
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 121 mm
Thk = 200 > Req'd Thk = 121 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.076	0.038	0.057	0.049	0.025	0.037	
M_u (tf-m/m)	2.32	1.17	1.76	1.50	0.77	1.13	
ρ (%)	0.344	0.171	0.260	0.249	0.126	0.187	0.200
A_{st} (cm²/m)	5.69	2.83	4.29	3.88	1.96	2.92	4.00
#3	@120	@250	@160	@180	@360	@240	@170
#3+#4	@170	@250	@230	@250	@450	@330	@240
#4	@220	@350	@290	@320	@450	@420	@320
#4+#5	@280	@450	@370	@400	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 3.44 < ΦV_c = 12.40 tf/m ..... O.K.

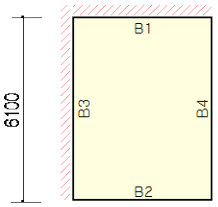
Long Direction Shear
V_{uy} = 0.85 < ΦV_c = 11.56 tf/m ..... O.K.

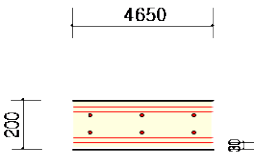
MIDAS/Set **Slab Design [BS4]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4650 * 6100 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 80 X160, B2 = 80 X160 cm
B3 = 80 X160, B4 = 80 X160 cm





2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.50 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 2.07 tf/m²

3. Check Mnimum Slab Thk.

α_m = (98.26+149.26+128.89+188.97)/4 = 141.3432
β = L_{ny}/L_{nx} = 1.3766
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 109 mm
Thk = 200 > Req'd Thk = 109 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.069	0.035	0.052	0.049	0.025	0.037	
M_u (tf-m/m)	2.12	1.07	1.60	1.50	0.77	1.13	
ρ (%)	0.313	0.157	0.236	0.249	0.126	0.187	0.200
A_{st} (cm²/m)	5.18	2.59	3.91	3.88	1.96	2.92	4.00
#3	@130	@270	@180	@180	@360	@240	@170
#3+#4	@190	@270	@250	@250	@450	@330	@240
#4	@240	@380	@320	@320	@450	@420	@320
#4+#5	@310	@450	@410	@400	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 3.12 < ΦV_c = 12.40 tf/m ..... O.K.

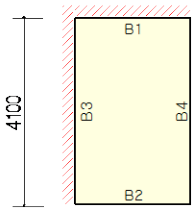
Long Direction Shear
V_{uy} = 1.18 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [1S0]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

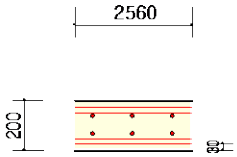
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 2560 * 4100 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 50 X 80 cm
B3 = 30 X 50, B4 = 50 X 80 cm



2. Applied Loads

Dead Load : W_d = 0.77 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l = 1.59 tf/m²



3. Check Minimum Slab Thk.

α_m = (13.04+19.59+2.94+29.45)/4 = 16.2562
β = L_{ny}/L_{nx} = 1.6667
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 71 mm
Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.078	0.039	0.059	0.049	0.025	0.037	
M_u (tf-m/m)	0.58	0.29	0.44	0.36	0.19	0.27	
ρ (%)	0.084	0.042	0.064	0.060	0.030	0.045	0.200
A_{st} (cm²/m)	1.40	0.70	1.05	0.93	0.47	0.70	4.00
#3	@450	@450	@450	@450	@450	@450	@170
#3+#4	@450	@450	@450	@450	@450	@450	@240
#4	@450	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 1.53 < ΦV_c = 12.40 tf/m ..... O.K.

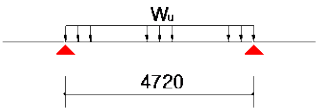
Long Direction Shear
V_{uy} = 0.31 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [1S1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 4.72 m (Both End Fixed)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.77 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.59 tf/m²

3. Check Minimum Slab Thk.

h_{min} = L/28 = 169 mm
h = h_{min}*(0.4+f_y/7000) = 135 mm
Thk = 200 > Req'd Thk = 135 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	3.22 (W_uL^2/11)	2.21 (W_uL^2/16)	0.00	
ρ (%)	0.486	0.331	0.000	0.200
A_{st} (cm²/m)	8.00	5.45	0.00	4.00
#3	@ 80	@ 130	@ 450	@ 170
#3+#4	@ 120	@ 180	@ 450	@ 240
#4	@ 160	@ 230	@ 450	@ 320
#4+#5	@ 200	@ 290	@ 450	@ 410

5. Check Shear Stresses

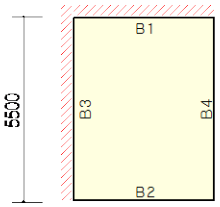
Strength Reduction Factor Φ = 0.850
V_{ux} = 3.75 < ΦV_c = 12.40 tf/m ..... O.K.

MIDAS/Set **Slab Design [1S2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

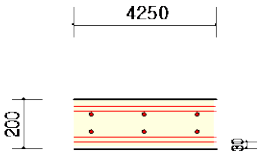
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4250 * 5500 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 30 X 50, B2 = 45 X 60 cm
B3 = 50 X 80, B4 = 50 X 80 cm



2. Applied Loads

Dead Load : W_d = 0.77 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.59 tf/m²



3. Check Minimum Slab Thk

α_m = (1.37+5.46+12.58+18.97)/4 = 9.5958
β = L_{ny}/L_{nx} = 1.3667
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 106 mm
Thk = 200 > Req'd Thk = 106 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.069	0.035	0.052	0.049	0.025	0.037	
M_u (tf-m/m)	1.54	0.78	1.16	1.10	0.56	0.83	
ρ (%)	0.226	0.114	0.171	0.181	0.092	0.136	0.200
A_{st} (cm²/m)	3.74	1.88	2.82	2.82	1.43	2.12	4.00
#3	@180	@370	@250	@250	@450	@330	@170
#3+#4	@260	@370	@350	@340	@450	@450	@240
#4	@340	@450	@450	@440	@450	@450	@320
#4+#5	@430	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 2.32 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 0.90 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [1S3]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

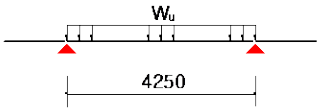
Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

f_y = 2800 kgf/cm²

Slab Span L : 4.25 m (Both End Fixed)

Slab Depth : 250 mm (c_c = 50 mm)



2. Applied Loads

Dead Load : W_d = 0.75 tf/m²

Live Load : W_l = 0.60 tf/m²

W_u = 1.4*W_d+1.7*W_l= 2.07 tf/m²

3. Check Minimum Slab Thk.

h_{min}= L/28 = 152 mm

h = h_{min}*(0.4+f_y/7000) = 121 mm

Thk = 250 > Req'd Thk = 121 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	3.40 (W_uL^2/11)	2.34 (W_uL^2/16)	0.00	
ρ (%)	0.365	0.249	0.000	0.200
A_{st} (cm²/m)	7.09	4.84	0.00	5.00
#3	@ 100	@ 140	@ 450	@ 140
#3+#4	@ 140	@ 200	@ 450	@ 190
#4	@ 180	@ 260	@ 450	@ 250
#4+#5	@ 230	@ 330	@ 450	@ 320

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

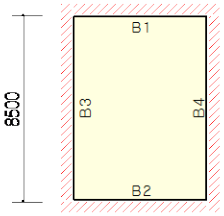
V_{ux} = 4.40 < ΦV_c = 14.66 tf/m ..... O.K.

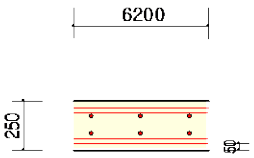
MIDAS/Set **Slab Design [1S4]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 6200 * 8500 * 250 mm (c_c = 50 mm)
Edge Beam Size :
B1 = 45 X 60, B2 = 45 X 60 cm
B3 = 50 X 70, B4 = 50 X 70 cm





2. Applied Loads

Dead Load : W_d = 0.75 tf/m²
Live Load : W_l = 0.60 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 2.07 tf/m²

3. Check Minimum Slab Thk

α_m = (1.11+1.11+2.77+2.77)/4 = 1.9412
β = L_{ny}/L_{nx} = 1.4123
h_{min} = 120 mm
h = $l_n(800+f_y/14)/(36000+5000\beta(\alpha_m-0.2))$ = 167 mm
Thk = 250 > Req'd Thk = 167 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span		Long Span		Minimum
	Cont.	Cent.	Cont.	Cent.	Ratio
Coefficient	0.054	0.041	0.033	0.025	
M_u (tf-m/m)	3.66	2.73	2.22	1.68	
ρ (%)	0.390	0.289	0.259	0.196	0.200
A_{st} (cm²/m)	7.62	5.65	4.82	3.63	5.00
#3	@ 90	@120	@140	@190	@140
#3+#4	@130	@170	@200	@270	@190
#4	@160	@220	@260	@340	@250
#4+#5	@210	@280	@320	@430	@320

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 4.73 < ΦV_c = 14.66 tf/m ..... O.K.

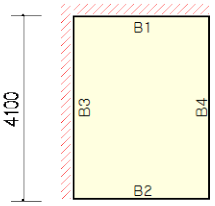
Long Direction Shear
V_{uy} = 1.65 < ΦV_c = 13.82 tf/m ..... O.K.

MIDAS/Set **Slab Design [2S0]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

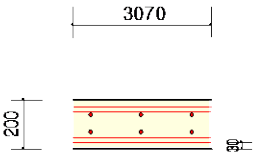
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 3070 * 4100 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 50 X 80 cm
B3 = 30 X 50, B4 = 60 X 60 cm



2. Applied Loads

Dead Load : W_d = 0.88 tf/m²
Live Load : W_l = 0.30 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 1.75 tf/m²



3. Check Minimum Slab Thk.

α_m = (13.04+19.59+2.46+11.19)/4 = 11.5676
β = L_{ny}/L_{nx} = 1.3740
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 74 mm
Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.069	0.035	0.052	0.049	0.025	0.037	
M_u (tf-m/m)	0.83	0.42	0.63	0.59	0.30	0.44	
ρ (%)	0.121	0.061	0.092	0.097	0.049	0.073	0.200
A_{st} (cm²/m)	2.00	1.01	1.51	1.51	0.77	1.14	4.00
#3	@350	@450	@450	@450	@450	@450	@170
#3+#4	@450	@450	@450	@450	@450	@450	@240
#4	@450	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 1.79 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 0.68 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [2S1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

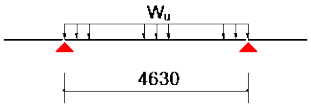
Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

f_y = 2800 kgf/cm²

Slab Span L : 4.63 m (Both End Fixed)

Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.88 tf/m²

Live Load : W_l = 0.90 tf/m²

W_u = 1.4*W_d+1.7*W_l = 2.77 tf/m²

3. Check Minimum Slab Thk

h_{min} = L/28 = 165 mm

h = h_{min}*(0.4+f_y/7000) = 132 mm

Thk = 200 > Req'd Thk = 132 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	5.39 (W_uL^2/11)	3.71 (W_uL^2/16)	0.00	
ρ (%)	0.849	0.574	0.000	0.200
A_{st} (cm²/m)	13.83	9.35	0.00	4.00
#4	@ 90	@ 130	@ 450	@ 320
#4+#5	@ 110	@ 170	@ 450	@ 410
#5	@ 140	@ 210	@ 450	@ 450
#5+#6	@ 170	@ 250	@ 450	@ 450

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

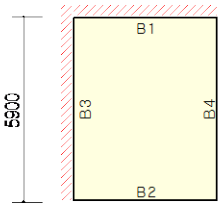
V_{ux} = 6.40 < ΦV_c = 12.28 tf/m ..... O.K.

MIDAS/Set **Slab Design [2S2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

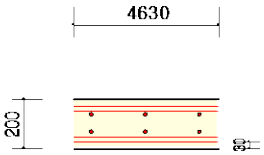
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4630 * 5900 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 30 X 50, B2 = 60 X 60 cm
B3 = 50 X 80, B4 = 60 X 60 cm



2. Applied Loads

Dead Load : W_d = 0.88 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l = 1.75 tf/m²



3. Check Minimum Slab Thk

α_m = (1.28+6.32+11.54+7.85)/4 = 6.7469
β = L_{ny}/L_{nx} = 1.3358
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 113 mm
Thk = 200 > Req'd Thk = 113 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.068	0.034	0.051	0.049	0.025	0.037	
M_u (tf-m/m)	1.96	0.99	1.48	1.42	0.73	1.08	
ρ (%)	0.291	0.145	0.219	0.236	0.120	0.178	0.200
A_{st} (cm²/m)	4.80	2.40	3.61	3.68	1.87	2.77	4.00
#3	@140	@290	@190	@190	@380	@250	@170
#3+#4	@200	@290	@270	@260	@450	@350	@240
#4	@260	@410	@350	@330	@450	@450	@320
#4+#5	@330	@450	@440	@420	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear
V_{ux} = 2.71 < ΦV_c = 12.40 tf/m ..... O.K.

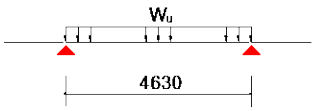
Long Direction Shear
V_{uy} = 1.13 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [2S3]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 4.63 m (Both End Fixed)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.88 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.75 tf/m²

3. Check Minimum Slab Thk

h_{min} = L/28 = 165 mm
h = h_{min}*(0.4+f_y/7000) = 132 mm
Thk = 200 > Req'd Thk = 132 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	3.40 (W_uL^2/11)	2.34 (W_uL^2/16)	0.00	
ρ (%)	0.515	0.351	0.000	0.200
A_{st} (cm²/m)	8.47	5.76	0.00	4.00
#3	@ 80	@ 120	@ 450	@ 170
#3+#4	@ 110	@ 170	@ 450	@ 240
#4	@ 150	@ 220	@ 450	@ 320
#4+#5	@ 190	@ 280	@ 450	@ 410

5. Check Shear Stresses

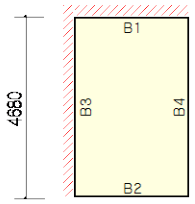
Strength Reduction Factor Φ = 0.850
V_{ux} = 4.04 < ΦV_c = 12.40 tf/m ..... O.K.

MIDAS/Set **Slab Design [3S0]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

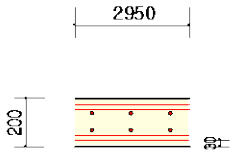
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 2950 * 4680 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 40 X 65 cm
B3 = 30 X 50, B4 = 50 X 80 cm



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.25 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 1.64 tf/m²



3. Check Minimum Slab Thk

α_m = (11.42+7.58+2.56+26.12)/4 = 11.9202
β = L_{ny}/L_{nx} = 1.6588
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 83 mm
Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.078	0.039	0.059	0.049	0.025	0.037	
M_u (tf-m/m)	0.83	0.41	0.63	0.52	0.27	0.39	
ρ (%)	0.121	0.060	0.091	0.086	0.044	0.065	0.200
A_{st} (cm²/m)	2.00	1.00	1.51	1.34	0.68	1.01	4.00
#3	@350	@450	@450	@450	@450	@450	@170
#3+#4	@450	@450	@450	@450	@450	@450	@240
#4	@450	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 1.85 < ΦV_c = 12.40 tf/m ..... O.K.

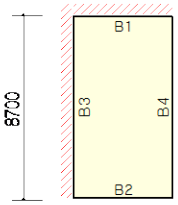
Long Direction Shear
V_{uy} = 0.39 < ΦV_c = 11.56 tf/m ..... O.K.

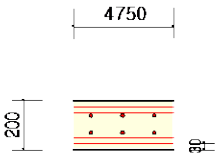
MIDAS/Set **Slab Design [3S1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4750 * 8700 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 65 X 90, B2 = 65 X 90 cm
B3 = 40 X 65, B4 = 50 X 80 cm





2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.25 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 1.64 tf/m²

3. Check Minimum Slab Thk.

α_m = (10.97+17.26+4.78+17.17)/4 = 12.5438
β = L_{ny}/L_{nx} = 1.8721
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 152 mm
Thk = 200 > Req'd Thk = 152 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.086	0.043	0.065	0.049	0.025	0.037	
M_u (tf-m/m)	2.60	1.30	1.96	1.48	0.76	1.12	
ρ (%)	0.386	0.191	0.290	0.246	0.125	0.185	0.200
A_{st} (cm²/m)	6.39	3.16	4.80	3.83	1.94	2.88	4.00
#3	@110	@220	@140	@180	@360	@240	@170
#3+#4	@150	@220	@200	@250	@450	@340	@240
#4	@200	@310	@260	@320	@450	@430	@320
#4+#5	@250	@400	@330	@400	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 3.26 < ΦV_c = 12.40 tf/m ..... O.K.

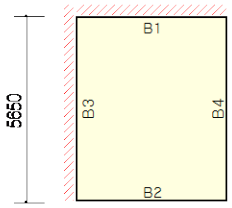
Long Direction Shear
V_{uy} = 0.48 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [3S2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

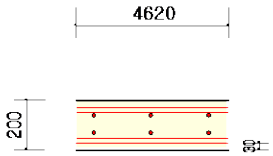
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4620 * 5650 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 30 X 50 cm
B3 = 40 X 65, B4 = 50 X 80 cm



2. Applied Loads

Dead Load : W_d = 0.87 tf/m²
Live Load : W_l = 0.25 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.64 tf/m²



3. Check Mnimum Slab Thk.

α_m = (9.46+2.16+4.92+17.60)/4 = 8.5351
β = L_{ny}/L_{nx} = 1.2590
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 111 mm
Thk = 200 > Req'd Thk = 111 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.064	0.032	0.048	0.049	0.025	0.037	
M_u (tf-m/m)	1.83	0.92	1.38	1.39	0.71	1.05	
ρ (%)	0.271	0.134	0.202	0.231	0.117	0.174	0.200
A_{st} (cm²/m)	4.47	2.22	3.34	3.60	1.83	2.71	4.00
#3	@150	@310	@210	@190	@380	@260	@170
#3+#4	@220	@310	@290	@270	@450	@360	@240
#4	@280	@440	@380	@340	@450	@450	@320
#4+#5	@360	@450	@450	@430	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 2.44 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 1.22 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [4S0]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : $f_c' = 280$ kgf/cm^2

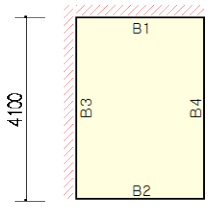
$f_y = 2800$ kgf/cm^2

Slab Dim. : 2930 * 4100 * 200 mm (c_c = 30 mm)

Edge Beam Size :

B1 = 50 X 80, B2 = 50 X 80 cm

B3 = 30 X 50, B4 = 50 X 70 cm

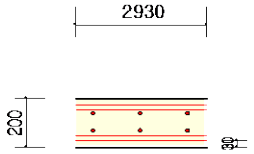


2. Applied Loads

Dead Load : $W_d = 0.92$ tf/m^2

Live Load : $W_l = 0.30$ tf/m^2

$W_u = 1.4*W_d+1.7*W_l = 1.80$ tf/m^2



3. Check Mnimum Slab Thk

$\alpha_m = (13.04+19.59+2.57+17.04)/4 = 13.0611$

$\beta = L_{ny}/L_{nx} = 1.4229$

$h_{min} = 90$ mm

$h = l_n(800+f_y/14)/(36000+9000\beta) = 74$ mm

Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor $\Phi = 0.900$

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.071	0.036	0.054	0.049	0.025	0.037	
M_u (tf-m/m)	0.82	0.41	0.62	0.56	0.29	0.43	
ρ (%)	0.119	0.060	0.091	0.093	0.047	0.070	0.200
A_{st} (cm^2/m)	1.97	1.00	1.50	1.45	0.74	1.09	4.00
#3	@350	@450	@450	@450	@450	@450	@170
#3+#4	@450	@450	@450	@450	@450	@450	@240
#4	@450	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor $\Phi = 0.850$

Short Direction Shear

$V_{ux} = 1.84 < \Phi V_c = 12.40$ tf/m ..... O.K.

Long Direction Shear

$V_{uy} = 0.62 < \Phi V_c = 11.56$ tf/m ..... O.K.

MIDAS/Set **Slab Design [4S1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

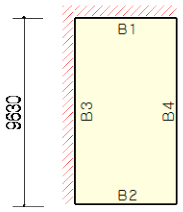
f_y = 2800 kgf/cm²

Slab Dim. : 5330 * 9630 * 200 mm (c_c = 30 mm)

Edge Beam Size :

B1 = 50 X 80, B2 = 60 X 90 cm

B3 = 40 X 65, B4 = 50 X 80 cm

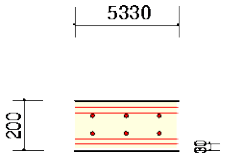


2. Applied Loads

Dead Load : W_d = 0.92 tf/m²

Live Load : W_l = 0.30 tf/m²

W_u = 1.4*W_d+1.7*W_l = 1.80 tf/m²



3. Check Minimum Slab Thk.

α_m = (5.55+14.82+4.26+15.46)/4 = 10.0223

β = L_{ny}/L_{nx} = 1.8607

h_{min} = 90 mm

h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 172 mm

Thk = 200 > Req'd Thk = 172 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.086	0.043	0.065	0.049	0.025	0.037	
M_u (tf-m/m)	3.66	1.83	2.77	2.10	1.07	1.59	
ρ (%)	0.550	0.271	0.413	0.351	0.177	0.264	0.200
A_{st} (cm²/m)	9.09	4.47	6.82	5.46	2.76	4.10	4.00
#3	@ 70	@150	@100	@120	@250	@170	@170
#3+#4	@100	@150	@140	@180	@350	@230	@240
#4	@140	@220	@180	@220	@450	@300	@320
#4+#5	@170	@280	@230	@280	@450	@380	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear

V_{ux} = 4.06 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear

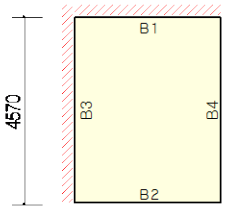
V_{uy} = 0.61 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [4S1a]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

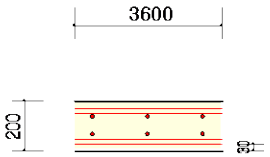
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 3600 * 4570 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 45 X 65 cm
B3 = 50 X 70, B4 = 50 X 70 cm



2. Applied Loads

Dead Load : W_d = 0.92 tf/m²
Live Load : W_l = 0.40 tf/m²
W_u = 1.4*W_d+1.7*W_l = 1.97 tf/m²



3. Check Minimum Slab Thk.

α_m = (11.70+8.42+9.55+14.26)/4 = 10.9792
β = L_{ny}/L_{nx} = 1.3210
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 85 mm
Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.067	0.034	0.051	0.049	0.025	0.037	
M_u (tf-m/m)	1.27	0.64	0.96	0.93	0.47	0.70	
ρ (%)	0.186	0.093	0.140	0.153	0.078	0.115	0.200
A_{st} (cm²/m)	3.08	1.54	2.32	2.38	1.21	1.80	4.00
#3	@230	@450	@300	@290	@450	@390	@170
#3+#4	@320	@450	@420	@410	@450	@450	@240
#4	@410	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 2.30 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 1.00 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [4S2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

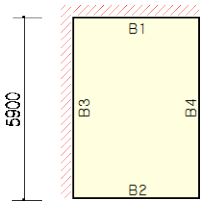
f_y = 2800 kgf/cm²

Slab Dim. : 4150 * 5900 * 200 mm (c_c = 30 mm)

Edge Beam Size :

B1 = 30 X 50, B2 = 45 X 60 cm

B3 = 50 X 80, B4 = 50 X 80 cm

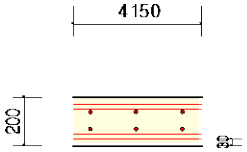


2. Applied Loads

Dead Load : W_d = 0.92 tf/m²

Live Load : W_l = 0.30 tf/m²

W_u = 1.4*W_d+1.7*W_l = 1.80 tf/m²



3. Check Minimum Slab Thk.

α_m = (1.28+5.12+12.88+19.38)/4 = 9.6643

β = L_{ny}/L_{nx} = 1.5137

h_{min} = 90 mm

h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 111 mm

Thk = 200 > Req'd Thk = 111 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.074	0.037	0.056	0.049	0.025	0.037	
M_u (tf-m/m)	1.77	0.89	1.34	1.17	0.60	0.89	
ρ (%)	0.261	0.131	0.197	0.194	0.099	0.146	0.200
A_{st} (cm²/m)	4.31	2.16	3.26	3.03	1.54	2.28	4.00
#3	@160	@320	@210	@230	@450	@310	@170
#3+#4	@230	@320	@300	@320	@450	@430	@240
#4	@290	@450	@390	@410	@450	@450	@320
#4+#5	@370	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear

V_{ux} = 2.76 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear

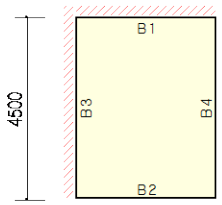
V_{uy} = 0.79 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [RSO]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

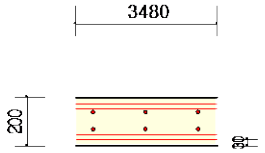
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 3480 * 4500 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 70, B2 = 50 X 80 cm
B3 = 30 X 50, B4 = 50 X 70 cm



2. Applied Loads

Dead Load : W_d = 0.48 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l = 1.18 tf/m²



3. Check Mnimum Slab Thk

α_m = (7.64+18.02+2.17+14.69)/4 = 10.6291
β = L_{ny}/L_{nx} = 1.2987
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 84 mm
Thk = 200 > Req'd Thk = 90 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.066	0.033	0.050	0.049	0.025	0.037	
M_u (tf-m/m)	0.74	0.37	0.56	0.55	0.28	0.41	
ρ (%)	0.108	0.054	0.082	0.090	0.046	0.068	0.200
A_{st} (cm²/m)	1.79	0.90	1.35	1.41	0.72	1.06	4.00
#3	@390	@450	@450	@450	@450	@450	@170
#3+#4	@450	@450	@450	@450	@450	@450	@240
#4	@450	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 1.35 < ΦV_c = 12.40 tf/m ..... O.K.

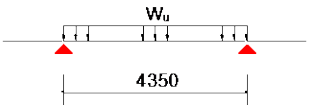
Long Direction Shear
V_{uy} = 0.61 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [RS1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 4.35 m (Both End Fixed)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.48 tf/m²
Live Load : W_l = 0.30 tf/m²
$W_u = 1.4*W_d+1.7*W_l$ = 1.18 tf/m²

3. Check Mnimum Slab Thk

h_{min} = L/28 = 155 mm
h = $h_{min}*(0.4+f_y/7000)$ = 124 mm
Thk = 200 > Req'd Thk = 124 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	2.03 ($W_uL^2/11$)	1.40 ($W_uL^2/16$)	0.00	
ρ (%)	0.304	0.208	0.000	0.200
A_{st} (cm²/m)	5.00	3.42	0.00	4.00
#3	@ 140	@ 200	@ 450	@ 170
#3+#4	@ 200	@ 290	@ 450	@ 240
#4	@ 250	@ 370	@ 450	@ 320
#4+#5	@ 320	@ 450	@ 450	@ 410

5. Check Shear Stresses

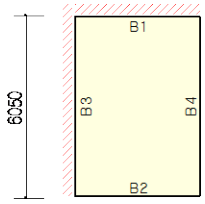
Strength Reduction Factor Φ = 0.850
V_{ux} = 2.57 < ΦV_c = 12.40 tf/m ..... O.K.

MIDAS/Set **Slab Design [RS2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

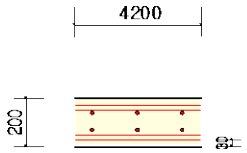
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4200 * 6050 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 30 X 50, B2 = 45 X 60 cm
B3 = 50 X 70, B4 = 50 X 70 cm



2. Applied Loads

Dead Load : W_d = 0.48 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l = 1.18 tf/m²



3. Check Mnimum Slab Thk

α_m = (1.25+5.00+8.18+12.44)/4 = 6.7170
β = L_{ny}/L_{nx} = 1.5338
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 114 mm
Thk = 200 > Req'd Thk = 114 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.074	0.037	0.056	0.049	0.025	0.037	
M_u (tf-m/m)	1.20	0.61	0.91	0.79	0.40	0.60	
ρ (%)	0.177	0.089	0.134	0.131	0.066	0.099	0.200
A_{st} (cm²/m)	2.92	1.46	2.21	2.04	1.03	1.53	4.00
#3	@240	@450	@320	@340	@450	@450	@170
#3+#4	@340	@450	@450	@450	@450	@450	@240
#4	@430	@450	@450	@450	@450	@450	@320
#4+#5	@450	@450	@450	@450	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear
V_{ux} = 1.86 < ΦV_c = 12.40 tf/m ..... O.K.

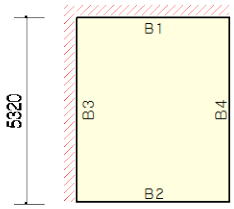
Long Direction Shear
V_{uy} = 0.51 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [RS3]**

Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
Designer	Microsoft	File Name	

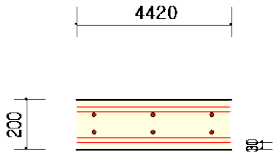
1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Dim. : 4420 * 5320 * 200 mm (c_c = 30 mm)
Edge Beam Size :
B1 = 50 X 80, B2 = 30 X 50 cm
B3 = 35 X 55, B4 = 50 X 85 cm



2. Applied Loads

Dead Load : W_d = 0.86 tf/m²
Live Load : W_l = 0.55 tf/m²
W_u = 1.4*W_d+1.7*W_l= 2.14 tf/m²



3. Check Minimum Slab Thk

α_m = (10.05+2.29+2.65+22.28)/4 = 9.3157
β = L_{my}/L_{nx} = 1.2315
h_{min} = 90 mm
h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 104 mm
Thk = 200 > Req'd Thk = 104 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.063	0.032	0.047	0.049	0.025	0.037	
M_u (tf-m/m)	2.16	1.08	1.62	1.67	0.85	1.26	
ρ (%)	0.320	0.158	0.239	0.279	0.141	0.209	0.200
A_{st} (cm²/m)	5.28	2.61	3.94	4.34	2.19	3.26	4.00
#3	@130	@270	@170	@160	@320	@210	@170
#3+#4	@180	@270	@250	@220	@440	@300	@240
#4	@240	@380	@320	@280	@450	@380	@320
#4+#5	@300	@450	@410	@360	@450	@450	@410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
Short Direction Shear
V_{ux} = 2.99 < ΦV_c = 12.40 tf/m ..... O.K.

Long Direction Shear
V_{uy} = 1.59 < ΦV_c = 11.56 tf/m ..... O.K.

MIDAS/Set **Slab Design [RS4]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

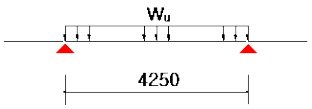
Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

f_y = 2800 kgf/cm²

Slab Span L : 4.25 m (Both End Fixed)

Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.48 tf/m²

Live Load : W_l = 0.30 tf/m²

W_u = 1.4*W_d+1.7*W_l = 1.18 tf/m²

3. Check Minimum Slab Thk.

h_{min} = L/28 = 152 mm

h = h_{min}*(0.4+f_y/7000) = 121 mm

Thk = 200 > Req'd Thk = 121 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	1.94 (W_uL^2/11)	1.33 (W_uL^2/16)	0.00	
ρ (%)	0.290	0.198	0.000	0.200
A_{st} (cm²/m)	4.76	3.26	0.00	4.00
#3	@ 140	@ 210	@ 450	@ 170
#3+#4	@ 200	@ 300	@ 450	@ 240
#4	@ 260	@ 390	@ 450	@ 320
#4+#5	@ 340	@ 450	@ 450	@ 410

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

V_{ux} = 2.51 < ΦV_c = 12.40 tf/m ..... O.K.

MIDAS/Set **Slab Design [PSO]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : $f_c' = 280$ kgf/cm^2

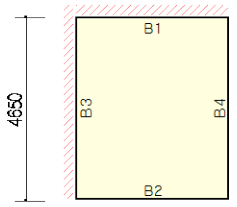
$f_y = 2800$ kgf/cm^2

Slab Dim. : 3930 * 4650 * 150 mm (c_c = 25 mm)

Edge Beam Size :

B1 = 35 X 60, B2 = 35 X 60 cm

B3 = 35 X 50, B4 = 35 X 50 cm

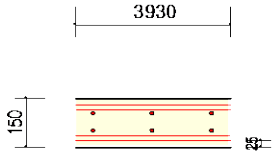


2. Applied Loads

Dead Load : $W_d = 0.48$ tf/m^2

Live Load : $W_l = 0.20$ tf/m^2

$W_u = 1.4*W_d+1.7*W_l = 1.01$ tf/m^2



3. Check Mnimum Slab Thk

$\alpha_m = (8.19+12.82+5.29+8.28)/4 = 8.6451$

$\beta = L_{ny}/L_{nx} = 1.2011$

$h_{min} = 90$ mm

$h = l_n(800+f_y/14)/(36000+9000\beta) = 92$ mm

Thk = 150 > Req'd Thk = 92 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor $\Phi = 0.900$

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.062	0.031	0.046	0.049	0.025	0.037	
M_u (tf-m/m)	0.80	0.40	0.60	0.64	0.32	0.48	
ρ (%)	0.223	0.110	0.167	0.208	0.106	0.157	0.200
A_{st} (cm^2/m)	2.68	1.32	2.00	2.31	1.17	1.74	3.00
#3	@260	@450	@350	@300	@450	@400	@230
#3+#4	@370	@450	@450	@420	@450	@450	@330
#4	@450	@450	@450	@450	@450	@450	@430
#4+#5	@450	@450	@450	@450	@450	@450	@450

5. Check Shear Stresses

Strength Reduction Factor $\Phi = 0.850$

Short Direction Shear

$V_{ux} = 1.23 < \Phi V_c = 9.00$ tf/m ..... O.K.

Long Direction Shear

$V_{uy} = 0.70 < \Phi V_c = 8.17$ tf/m ..... O.K.

MIDAS/Set **Slab Design [PS1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91

Material Data : f_c' = 280 kgf/cm²

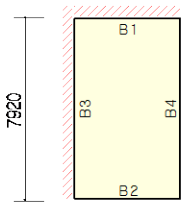
f_y = 2800 kgf/cm²

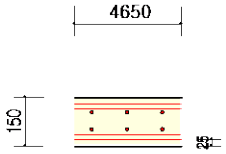
Slab Dim. : 4650 * 7920 * 150 mm (c_c = 25 mm)

Edge Beam Size :

B1 = 35 X 50, B2 = 35 X 50 cm

B3 = 35 X 60, B4 = 35 X 60 cm





2. Applied Loads

Dead Load : W_d = 0.48 tf/m²

Live Load : W_l = 0.20 tf/m²

$W_u = 1.4*W_d+1.7*W_l$ = 1.01 tf/m²

3. Check Minimum Slab Thk.

α_m = (2.63+4.28+8.19+12.82)/4 = 6.9800

β = L_{ny}/L_{nx} = 1.7605

h_{min} = 90 mm

h = $l_n(800+f_y/14)/(36000+9000\beta)$ = 146 mm

Thk = 150 > Req'd Thk = 146 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Long Span			Minimum
	Cont.	DisCon	Cent.	Cont.	DisCon	Cent.	Ratio
Coefficient	0.082	0.041	0.062	0.049	0.025	0.037	
M_u (tf-m/m)	1.53	0.77	1.16	0.92	0.47	0.69	
ρ (%)	0.431	0.213	0.324	0.302	0.153	0.227	0.200
A_{st} (cm²/m)	5.19	2.56	3.90	3.35	1.69	2.52	3.00
#3	@130	@270	@180	@210	@410	@280	@230
#3+#4	@190	@270	@250	@290	@450	@380	@330
#4	@240	@380	@320	@360	@450	@450	@430
#4+#5	@310	@450	@410	@450	@450	@450	@450

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850

Short Direction Shear

V_{ux} = 1.98 < ΦV_c = 9.00 tf/m ..... O.K.

Long Direction Shear

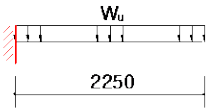
V_{uy} = 0.35 < ΦV_c = 8.17 tf/m ..... O.K.

MIDAS/Set **Slab Design [CS1]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 2.25 m (Cantilever)
Slab Depth : 200 mm (c_c = 30 mm)



2. Applied Loads

Dead Load : W_d = 0.92 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.80 tf/m²

3. Check Minimum Slab Thk

h_{min} = L_x/10 = 225 mm
h = h_{min}*(0.4+f_y/7000) = 180 mm
Thk = 200 > Req'd Thk = 180 mm ..... O.K.

4. Reinforcement

Strength Reduction Factor Φ = 0.900

	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	4.55 (W_uL^2/2)	0.00	0.00	
ρ (%)	0.711	0.000	0.000	0.200
A_{st} (cm²/m)	11.58	0.00	0.00	4.00
#4	@ 110	@ 450	@ 450	@ 320
#4+#5	@ 140	@ 450	@ 450	@ 410
#5	@ 170	@ 450	@ 450	@ 450
#5+#6	@ 200	@ 450	@ 450	@ 450

5. Check Shear Stresses

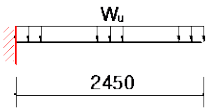
Strength Reduction Factor Φ = 0.850
V_{ux} = 4.05 < ΦV_c = 12.28 tf/m ..... O.K.

MIDAS/Set **Slab Design [CS2]**

	Company	Microsoft	Project Name	臺中市立長億高級中學圖書館資訊暨教
	Designer	Microsoft	File Name	

1. Geometry and Materials

Design Code : Taiwan-USD91
Material Data : f_c' = 280 kgf/cm²
f_y = 2800 kgf/cm²
Slab Span L : 2.45 m (Cantilever)
Slab Depth : 300 mm (c_c = 50 mm)



2. Applied Loads

Dead Load : W_d = 0.80 tf/m²
Live Load : W_l = 0.30 tf/m²
W_u = 1.4*W_d+1.7*W_l= 1.63 tf/m²

3. Check Minimum Slab Thk.

h_{min} = L_x/10 = 245 mm
h = h_{min}*(0.4+f_y/7000) = 196 mm
Thk = 300 > Req'd Thk = 196 mm ..... O.K.

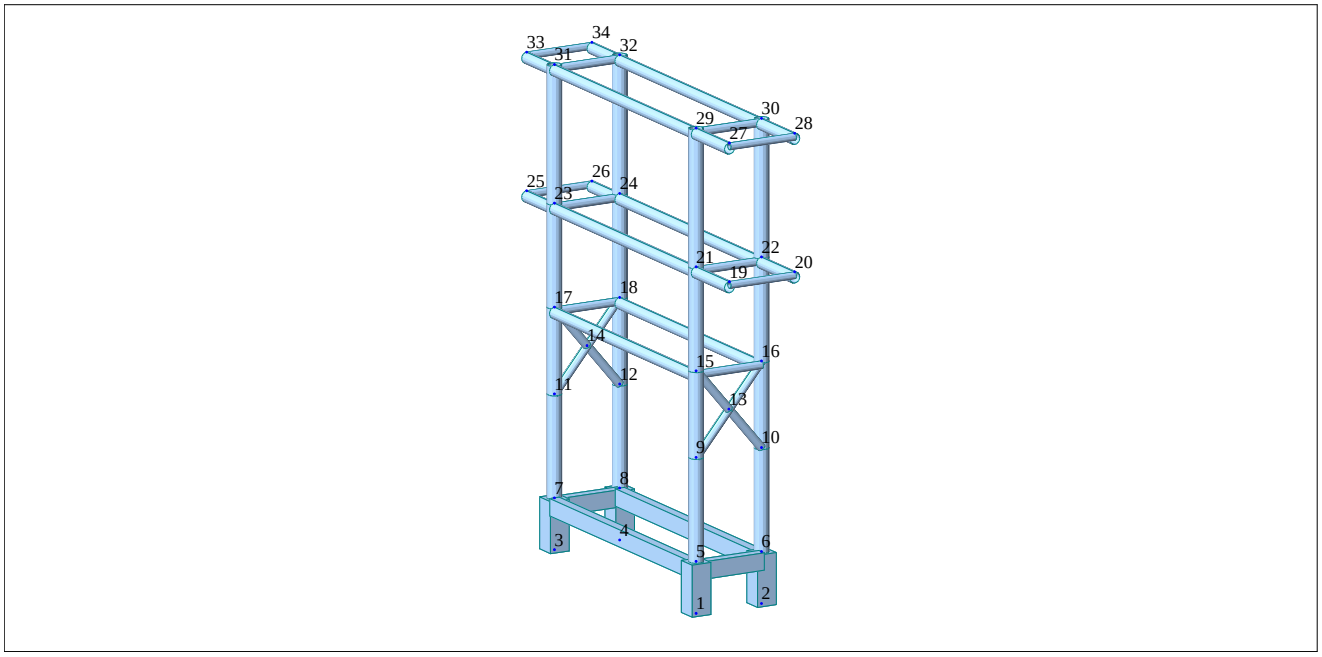
4. Reinforcement

Strength Reduction Factor Φ = 0.900

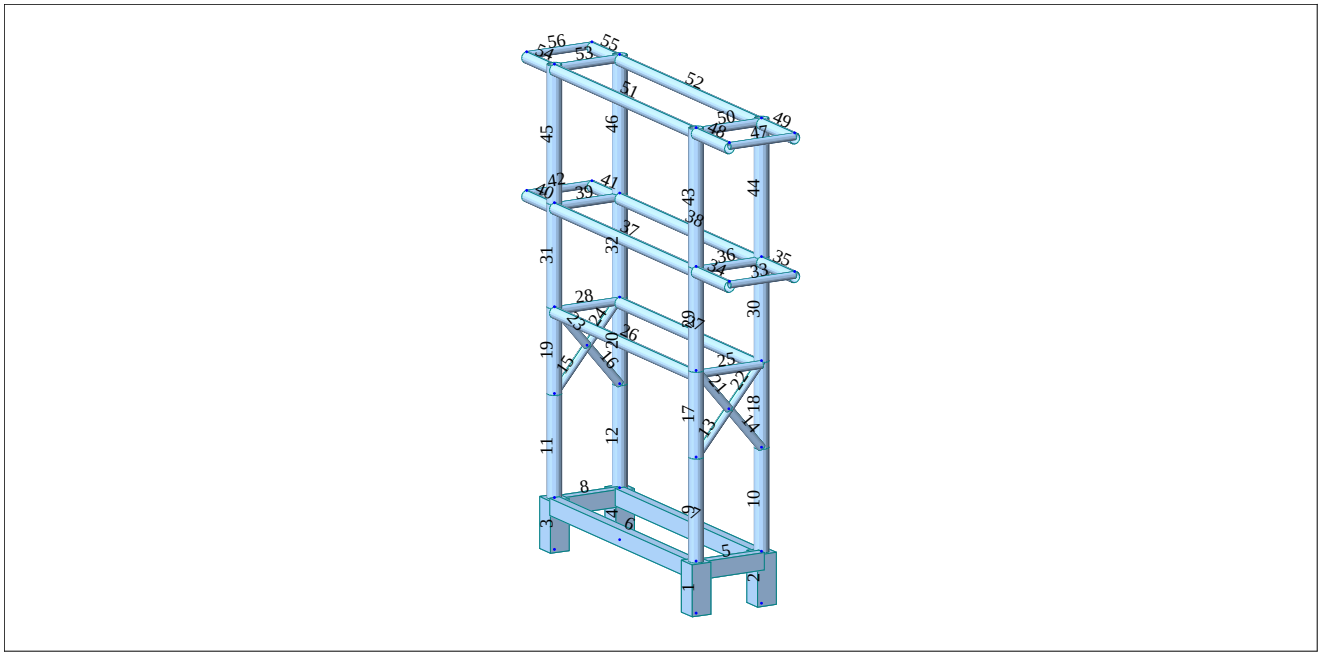
	Short Span			Minimum
	Cont.	Cent.	DisCon	Ratio (z)
M_u (tf-m/m)	4.89 (W_uL^2/2)	0.00	0.00	
ρ (%)	0.331	0.000	0.000	0.200
A_{st} (cm²/m)	8.10	0.00	0.00	6.00
#3	@ 80	@ 450	@ 450	@ 110
#3+#4	@ 120	@ 450	@ 450	@ 160
#4	@ 150	@ 450	@ 450	@ 210
#4+#5	@ 200	@ 450	@ 450	@ 270

5. Check Shear Stresses

Strength Reduction Factor Φ = 0.850
V_{ux} = 3.99 < ΦV_c = 18.43 tf/m ..... O.K.



分析模型圖（節點編號）



分析模型圖（桿件編號）

*** PROJECT INFORMATION

Project Name :
Date : 2021/6/30

*** CONTROL DATA

Panel Zone Effect : Do not Calculate
Unit System : KGF, CM
Definition of Frame
- X Direction of Frame : Unbraced I Sway
- Y Direction of Frame : Unbraced I Sway
- Design Type : 3-D
Design Code
- Steel : TWN-LSD96
- Concrete : TWN-USD92
- SRC : TWN-SRC100

*** LOAD CASE DATA

NO	NAME	TYPE	SELF WEIGHT FACTOR X	Y	Z	DESCRIPTION
1	SW	D	0.000	0.000	-1.000	INPUT ELEMENT
2	DL	D	0.000	0.000	0.000	SLAB DEAD LOAD
3	LL	L	0.000	0.000	0.000	SLAB LIVE LOAD
4	EX	E	0.000	0.000	0.000	X-DIR
5	EY	E	0.000	0.000	0.000	Y-DIR
6	EZ	EVT	0.000	0.000	-0.100	Z-DIR
7	WX	W	0.000	0.000	0.000	X-DIR
8	WY	W	0.000	0.000	0.000	Y-DIR

*** MATERIAL PROPERTY DATA

NO	NAME	TYPE	MODULUS OF ELASTICITY	SHEAR MODULUS	THERMAL COEFF.	POISSON RATIO	WEIGHT DENSITY
1	STK400	STEEL	2.1e+006	8.077e+005	6.667e-006	0.3	0.00785
2	C210	CONC	2.158e+005	9.248e+004	5.556e-006	0.167	0.0024

NO	NAME	TYPE	STRENGTH OF DESIGN MATERIAL STEEL	CONCRETE	MAIN REBAR	SUB REBAR
1	STK400	STEEL	2400	-	-	-
2	C210	CONC	-	210	4200	2800

*** STORY DATA

Ground Level = 0

NAME	LEVEL	HEIGHT	FLOOR DIAPHRAGM
4F	1350.000	0.000	Consider
3F	950.000	400.000	Consider
2F	550.000	400.000	Do not consider

1F	0.000	550.000	Do not consider
FT	-150.000	150.000	Do not consider

*** NODE DATA

NO	X	Y	Z	TEMPERATURE
1	0	0	-150	0
2	210	0	-150	0
3	0	790	-150	0
4	210	790	-150	0
5	0	0	0	0
6	210	0	0	0
7	0	790	0	0
8	210	790	0	0
9	0	0	300	0
10	210	0	300	0
11	0	790	300	0
12	210	790	300	0
13	105	0	425	0
14	105	790	425	0
15	0	0	550	0
16	210	0	550	0
17	0	790	550	0
18	210	790	550	0
19	0	-185	850	0
20	210	-185	850	0
21	0	0	850	0
22	210	0	850	0
23	0	790	850	0
24	210	790	850	0
25	0	945	850	0
26	210	945	850	0
27	0	-185	1250	0
28	210	-185	1250	0
29	0	0	1250	0
30	210	0	1250	0
31	0	790	1250	0
32	210	790	1250	0
33	0	945	1250	0
34	210	945	1250	0

*** SUPPORT / SPECIFIED DISPLACEMENT / POINT SPRING SUPPORT

** SUPPORT / SPECIFIED DISPLACEMENT

NODE	SUPPORT	SPECIFIED DISPLACEMENT					
	DDDRRR	Dx	Dy	Dz	Rx	Ry	Rz
1	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
4	111000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

*** FLOOR DIAPHRAGM / RIGID LINK DATA

MASTER	DDDRRR	NODES OF SAME DISPLACEMENT
4F Floor Diaphragm		
3F Floor Diaphragm		

*** SECTION PROPERTY DATA

NO	NAME	SHAPE	H	B	tw	tf1	r1
101	SC1	P	40.6	0.9	0	0	0
201	SG1	P	21.6	0.6	0	0	0
251	SB1	P	31.9	0.6	0	0	0
252	SCB1	P	31.9	0.6	0	0	0
301	Sb0	P	19.1	0.45	0	0	0
351	BR1	P	21.6	0.6	0	0	0
401	C1	SB	60	60	0	0	0
501	TG1	SB	50	30	0	0	0
551	TB1	SB	50	30	0	0	0

NO	NAME	STIFFNESS SCALE FACTOR							
		A	Asy	Asz	Ix	Iy	Iz	W	Boundary Group
101	SC1								
201	SG1								
251	SB1								
252	SCB1								
301	Sb0								
351	BR1								
401	C1								
501	TG1								
551	TB1								

NO	NAME	AREA	MOMENT OF INERTIA			SHAPE FACTOR	
		[SRC:EQIV.]	Ix	Iy	Iz	k-Y	k-Z
101	SC1	112	4.439e+004	2.22e+004	2.22e+004	0.5016	0.5016
201	SG1	39.6	4386	2190	2190	0.5005	0.5005
251	SB1	58.9	1.439e+004	7190	7190	0.5	0.5
252	SCB1	58.9	1.439e+004	7190	7190	0.5	0.5
301	Sb0	26.3	2283	1140	1140	0.5004	0.5004
351	BR1	39.6	4386	2190	2190	0.5005	0.5005
401	C1	3600	1.823e+006	1.08e+006	1.08e+006	0.8333	0.8333
501	TG1	1500	2.817e+005	3.125e+005	1.125e+005	0.8333	0.8333
551	TB1	1500	2.817e+005	3.125e+005	1.125e+005	0.8333	0.8333

NO	NAME	SECTION MODULUS Sy		SECTION MODULUS Sz	
		I or CONC.	J or STEEL	I or CONC.	J or STEEL
101	SC1	1090	1090	1090	1090
201	SG1	203	203	203	203
251	SB1	452	452	452	452
252	SCB1	452	452	452	452
301	Sb0	120	120	120	120
351	BR1	203	203	203	203

401	C1	3.6e+004	3.6e+004	3.6e+004	3.6e+004
501	TG1	1.25e+004	1.25e+004	7500	7500
551	TB1	1.25e+004	1.25e+004	7500	7500

*** BEAM MEMBER DATA

NO	NODAL CONNECTIVITY I	J	BEAM END RELEASE I	J	MATERIAL	SECTION	LENGTH
1	1	5	-	-	C210	C1	150
2	2	6	-	-	C210	C1	150
3	3	7	-	-	C210	C1	150
4	4	8	-	-	C210	C1	150
5	5	6	-	-	C210	TG1	210
6	7	5	-	-	C210	TB1	790
7	8	6	-	-	C210	TB1	790
8	7	8	-	-	C210	TG1	210
9	5	9	-	-	STK400	SC1	300
10	6	10	-	-	STK400	SC1	300
11	7	11	-	-	STK400	SC1	300
12	8	12	-	-	STK400	SC1	300
13	13	9	-	-	STK400	BR1	163.2
14	13	10	-	-	STK400	BR1	163.2
15	14	11	-	-	STK400	BR1	163.2
16	14	12	-	-	STK400	BR1	163.2
17	9	15	-	-	STK400	SC1	250
18	10	16	-	-	STK400	SC1	250
19	11	17	-	-	STK400	SC1	250
20	12	18	-	-	STK400	SC1	250
21	15	13	-	-	STK400	BR1	163.2
22	16	13	-	-	STK400	BR1	163.2
23	17	14	-	-	STK400	BR1	163.2
24	18	14	-	-	STK400	BR1	163.2
25	15	16	-	-	STK400	SG1	210
26	17	15	-	-	STK400	SB1	790
27	18	16	-	-	STK400	SB1	790
28	17	18	-	-	STK400	SG1	210
29	15	21	-	-	STK400	SC1	300
30	16	22	-	-	STK400	SC1	300
31	17	23	-	-	STK400	SC1	300
32	18	24	-	-	STK400	SC1	300
33	19	20	-	-	STK400	Sb0	210
34	21	19	-	-	STK400	SCB1	185
35	22	20	-	-	STK400	SCB1	185
36	21	22	-	-	STK400	SG1	210
37	23	21	-	-	STK400	SB1	790
38	24	22	-	-	STK400	SB1	790
39	23	24	-	-	STK400	SG1	210
40	23	25	-	-	STK400	SCB1	155
41	26	24	-	-	STK400	SCB1	155
42	25	26	-	-	STK400	Sb0	210
43	21	29	-	-	STK400	SC1	400
44	22	30	-	-	STK400	SC1	400
45	23	31	-	-	STK400	SC1	400
46	24	32	-	-	STK400	SC1	400
47	27	28	-	-	STK400	Sb0	210
48	29	27	-	-	STK400	SCB1	185
49	30	28	-	-	STK400	SCB1	185

50	29	30	-	-	STK400	SG1	210
51	31	29	-	-	STK400	SB1	790
52	32	30	-	-	STK400	SB1	790
53	31	32	-	-	STK400	SG1	210
54	31	33	-	-	STK400	SCB1	155
55	34	32	-	-	STK400	SCB1	155
56	33	34	-	-	STK400	Sb0	210

*** TOTAL WEIGHT / VOLUME / SURFACE AREA SUMMARY

SECTION NO	SECION NAME	SURFACE AREA	VOLUMN	WEIGHT	FRAME NUMBER	TRUSS NUMBER
101	SC1	1.248e+006	5.6e+005	4396	16	0
201	SG1	1.665e+005	4.99e+004	391.7	6	0
251	SB1	9.307e+005	2.792e+005	2192	6	0
252	SCB1	2.67e+005	8.01e+004	628.8	8	0
301	Sb0	9.827e+004	2.209e+004	173.4	4	0
351	BR1	1.726e+005	5.172e+004	406	8	0
401	C1	1.44e+005	2.16e+006	5184	4	0
501	TG1	6.72e+004	6.3e+005	1512	2	0
551	TB1	2.528e+005	2.37e+006	5688	2	0

*** LOAD DATA

; Self Weight, Nodal Load, Specified Displacement, Beam Load, Floor Load, Finishing Material Load,
System Temperature, Nodal Temperature, Element Temperature, Beam Section Temperature,
Wind Load, Static Seismic Load, Time History Analysis Data

** FLOOR LOAD TYPE DATA

NAME	LOADCASE NAME	LOAD	SUB-BEAM WEIGHT
3FL	DL	-0.03	Consider
	LL	-0.04	Do not consider
4FL	DL	-0.03	Consider
	LL	-0.04	Do not consider

** FLOOR LOAD DATA

LOAD TYPE	DISTRIBUTION	DIR.	PROJ	SUB-BEAM NUMBER	SUB-BEAM ANGLE	UNIT-W	NODE LIST
3FL	Two Way	GZ	NO	0	0	0	25 19 20 26
4FL	Two Way	GZ	NO	0	0	0	33 27 28 34

[LOAD CASE : SW]

** SELF WEIGHT DATA

; X=0, Y=0, Z=-1

[LOAD CASE : EX]

** STATIC SEISMIC LOAD DATA : CODE = TAIWAN02

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* MASS GENERATION DATA FOR LATERAL ANALYSIS OF BUILDING [UNIT: kgf, cm]

STORY NAME	TRANSLATIONAL MASS (X-DIR)	TRANSLATIONAL MASS (Y-DIR)	ROTATIONAL MASS	CENTER OF MASS (X-COORD)	CENTER OF MASS (Y-COORD)
4F	0.0	0.0	0.0	0.0	0.0
3F	0.0	0.0	0.0	0.0	0.0
2F	0.0	0.0	0.0	0.0	0.0
1F	0.0	0.0	0.0	0.0	0.0
FT	0.0	0.0	0.0	0.0	0.0
TOTAL :	0.0	0.0			

* ADDITIONAL MASSES FOR THE CALCULATION OF EQUIVALENT SEISMIC FORCE

Note. The following masses are between two adjacent stories or on the nodes released from floor rigid diaphragm by *Diaphragm Disconnect command. The masses are proportionally distributed to upper/lower stories according to their vertical locations. For dynamic analysis, however, floor masses and masses on vertical elements remain at their original locations.

STORY NAME	TRANSLATIONAL MASS (X-DIR)	TRANSLATIONAL MASS (Y-DIR)
4F	11.6842517	11.6842517
3F	12.222208	12.222208
2F	3.26465101	3.26465101
1F	10.5236794	10.5236794
FT	2.64327962	2.64327962
TOTAL :	40.3380697	40.3380697

* EQUIVALENT SEISMIC LOAD IN ACCORDANCE WITH TAIWAN STANDARD (TAIWAN2006) [UNIT: kgf, cm]

Seismic Zone (Z)	: Near Fault Zone
Design Horizontal Acceleration Factor (Ss^D)	: 0.80000
Design Horizontal Acceleration Factor (S1^D)	: 5.11200
Maximum Horizontal Acceleration Factor (Ss^M)	: 1.00000
Maximum Horizontal Acceleration Factor (S1^M)	: 0.55000
Design Near Source Factor (Na)	: 1.23000
Design Near Source Factor (Nv)	: 11.36000
Maximum Near Source Factor (Na)	: 1.25000
Maximum Near Source Factor (Nv)	: 1.50000
Soil Type	: Type 1
Design Site Magnify Factor (Fa)	: 1.00
Design Site Magnify Factor (Fv)	: 1.00
Maximum Site Magnify Factor (Fa)	: 1.00
Maximum Site Magnify Factor (Fv)	: 1.00
Importance Factor (I)	: 1.25

```
  Seismic Magnification Factor (Alpha_y)                : 1.20
  Fundamental Period Associated with X-dir. (Tx)        : 0.4231
  Fundamental Period Associated with Y-dir. (Ty)        : 0.4231
  Response Modification Coefficient for X-dir. (Rx)     : 4.0000
  Response Modification Coefficient for Y-dir. (Ry)     : 4.0000

  Design Horizontal Acceleration Coef. for X-dir. (ZdCx) : 0.2808
  Design Horizontal Acceleration Coef. for Y-dir. (ZdCy) : 0.2808

  Total Effective Weight For X-dir. Seismic Loads (Wx)   : 26643.991176
  Total Effective Weight For Y-dir. Seismic Loads (Wy)   : 26643.991176

  Scale Factor For X-directional Seismic Loads           : 0.00
  Scale Factor For Y-directional Seismic Loads           : 1.00

  Accidental Eccentricity For X-direction (Ex)           : Positive
  Accidental Eccentricity For Y-direction (Ey)           : Positive

  Torsional Amplification for Accidental Eccentricity    : Consider
  Torsional Amplification for Inherent Eccentricity      : Do not Consider

  Total Base Shear Of Model For X-direction              : 0.000000
  Total Base Shear Of Model For Y-direction              : 7481.979776
  Summation Of Wi*Hi Of Model For X-direction            : 0.000000
  Summation Of Wi*Hi Of Model For Y-direction            : 28614295.786545
-------------------------------------------------------------------------------

* ACCELERATION AND REDUNCTION FACTORS
-------------------------------------------------------------------------------

  - X - directional Values ----------------------------------------

  Design Horizontal Acceleration Factor (SaDx_d)         : 0.4148
  Seismic Loads Reduction Factor (Fux_d)                 : 1.0443
  The Modified Ratio of (SaDx_d/Fux_d)m                  : 0.3505
  Design Horizontal Accel. Coef. (ZdCx_d)                : 0.2608

  Design Horizontal Acceleration Factor (SaDx_min)       : 0.4789
  Seismic Loads Reduction Factor (Fux_min)               : 1.0443
  The Modified Ratio of (SaDx_min/Fux_min)m              : 0.3825
  Minimum Horizontal Accel. Coef.(ZdCx_min)              : 0.0991

  Design Horizontal Acceleration Factor (SaDx_max)       : 1.2500
  Seismic Loads Reduction Factor (Fux_max)               : 2.7848
  The Modified Ratio of (SaDx_max/Fux_max)m              : 0.3774
  Maximum Horizontal Accel. Coef.(ZdCx_max)              : 0.2808

  - Y - directional Values ----------------------------------------

  Design Horizontal Acceleration Factor (SaDy_d)         : 0.4148
  Seismic Loads Reduction Factor (Fuy_d)                 : 1.0443
  The Modified Ratio of (SaDy_d/Fuy_d)m                  : 0.3505
  Design Horizontal Accel. Coef. (ZdCy_d)                : 0.2608

  Design Horizontal Acceleration Factor (SaDy_min)       : 0.4789
  Seismic Loads Reduction Factor (Fuy_min)               : 1.0443
  The Modified Ratio of (SaDy_min/Fuy_min)m              : 0.3825
  Minimum Horizontal Accel. Coef.(ZdCy_min)              : 0.0991
```

Design Horizontal Acceleration Factor (SaDy_max) : 1.2500
Seismic Loads Reduction Factor (Fuy_max) : 2.7848
The Modified Ratio of (SaDy_max/Fuy_max)m : 0.3774
Maximum Horizontal Accel. Coef.(ZdCy_max) : 0.2808

==========
ECCENTRICITY RELATED DATA
==========

STORY NAME	X-DIRECTIONAL LOAD ACCIDENTAL ECCENT.	X-DIRECTIONAL LOAD INHERENT ECCENT.	X-DIRECTIONAL LOAD ACCIDENTAL AMP.FACTOR	X-DIRECTIONAL LOAD INHERENT AMP.FACTOR	Y-DIRECTIONAL LOAD ACCIDENTAL ECCENT.	Y-DIRECTIONAL LOAD INHERENT ECCENT.	Y-DIRECTIONAL LOAD ACCIDENTAL AMP.FACTOR	Y-DIRECTIONAL LOAD INHERENT AMP.FACTOR
4F	-56.5	0.0	1.0	0.0	10.5	0.0	1.0	0.0
3F	-56.5	0.0	1.0	0.0	10.5	0.0	1.0	0.0
2F	-39.5	0.0	1.0	0.0	10.5	0.0	1.0	0.0
G.L	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

The accidental amplification factors are automatically set to 1.0 when torsional amplification effect to accidental eccentricity is not considered.
The inherent amplification factors are automatically set to 0 when torsional amplification effect to inherent eccentricity is not considered.
The inherent amplification factors are all set to 'the input value - 1.0'.(This is to exclude the true inherent torsion)

** Story Force = Seismic Force x Scale Factor + Added Force

S E I S M I C L O A D G E N E R A T I O N D A T A X - D I R E C T I O N

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
4F	11457.58	1350.0	4044.455	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3F	11985.1	950.0	2977.136	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2F	3201.317	550.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
G.L.	--	0.0	--	--	--	0.0	0.0	---	---	---

S E I S M I C L O A D G E N E R A T I O N D A T A Y - D I R E C T I O N

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
4F	11457.58	1350.0	4044.455	0.0	4044.455	0.0	0.0	42466.78	0.0	42466.78
3F	11985.1	950.0	2977.136	0.0	2977.136	4044.455	1.6e+006	31259.93	0.0	31259.93
2F	3201.317	550.0	0.0	0.0	0.0	7021.591	4.4e+006	0.0	0.0	0.0
G.L.	--	0.0	--	--	--	7021.591	8.3e+006	---	---	---

```
================================================
 COMMENTS ABOUT TORSION
================================================

-------------------------------------------------------------------------------------------------------
If torsional amplification effects are considered :
-------------------------------------------------------------------------------------------------------
Accidental Torsion = Story Force * Accidental Eccentricity * Amp. Factor for Accidental Eccentricity
Inherent Torsion   = Story Force * Inherent Eccentricity * Amp. Factor for Inherent Eccentricity

-------------------------------------------------------------------------------------------------------
If torsional amplification effects are not considered :
-------------------------------------------------------------------------------------------------------
Accidental Torsion = Story Force * Accidental Eccentricity
Inherent Torsion   = 0

-------------------------------------------------------------------------------------------------------
The inherent torsion above is the additional torsion due to torsional amplification effect.
The true inherent torsion is considered automatically in analysis stage when the seismic force is
applied to the structure.
-------------------------------------------------------------------------------------------------------



 [ LOAD CASE : EY ]


 ** STATIC SEISMIC LOAD DATA : CODE = TAIWAN02

-------------------------------------------------------------------------------------------------------

* MASS GENERATION DATA FOR LATERAL ANALYSIS OF BUILDING         [UNIT: kgf, cm]
-----------------------------------------------------------

    STORY       TRANSLATIONAL MASS      ROTATIONAL   CENTER OF MASS
    NAME        (X-DIR)      (Y-DIR)    MASS         (X-COORD)    (Y-COORD)
 -----------  -----------  -----------  -----------  -----------  -----------
         4F          0.0          0.0          0.0          0.0          0.0
         3F          0.0          0.0          0.0          0.0          0.0
         2F          0.0          0.0          0.0          0.0          0.0
         1F          0.0          0.0          0.0          0.0          0.0
         FT          0.0          0.0          0.0          0.0          0.0
 -----------  -----------  -----------  -----------  -----------  -----------
   TOTAL :           0.0          0.0


* ADDITIONAL MASSES FOR THE CALCULATION OF EQUIVALENT SEISMIC FORCE
-------------------------------------------------------------------------

  Note. The following masses are between two adjacent stories or on the
  nodes released from floor rigid diaphragm by *Diaphragm Disconnect command.
  The masses are proportionally distributed to upper/lower stories according
  to their vertical locations. For dynamic analysis, however, floor masses
  and masses on vertical elements remain at their original locations.

     STORY        TRANSLATIONAL MASS
     NAME         (X-DIR)      (Y-DIR)
  -------------  -----------  -----------
             4F   11.6842517   11.6842517
```

3F	12.222208	12.222208
2F	3.26465101	3.26465101
1F	10.5236794	10.5236794
FT	2.64327962	2.64327962
TOTAL :	40.3380697	40.3380697

* EQUIVALENT SEISMIC LOAD IN ACCORDANCE WITH TAIWAN STANDARD (TAIWAN2006) [UNIT: kgf, cm]

Seismic Zone (Z)	: Near Fault Zone
Design Horizontal Acceleration Factor (Ss^D)	: 0.80000
Design Horizontal Acceleration Factor (S1^D)	: 0.61200
Maximum Horizontal Acceleration Factor (Ss^M)	: 1.00000
Maximum Horizontal Acceleration Factor (S1^M)	: 0.55000
Design Near Source Factor (Na)	: 1.23000
Design Near Source Factor (Nv)	: 1.36000
Maximum Near Source Factor (Na)	: 1.25000
Maximum Near Source Factor (Nv)	: 1.50000
Soil Type	: Type 1
Design Site Magnify Factor (Fa)	: 1.00
Design Site Magnify Factor (Fv)	: 1.00
Maximum Site Magnify Factor (Fa)	: 1.00
Maximum Site Magnify Factor (Fv)	: 1.00
Importance Factor (I)	: 1.25
Seismic Magnification Factor (Alpha_y)	: 1.20
Fundamental Period Associated with X-dir. (Tx)	: 0.4231
Fundamental Period Associated with Y-dir. (Ty)	: 0.4231
Response Modification Coefficient for X-dir. (Rx)	: 4.0000
Response Modification Coefficient for Y-dir. (Ry)	: 4.0000
Design Horizontal Acceleration Coef. for X-dir. (ZdCx)	: 0.2808
Design Horizontal Acceleration Coef. for Y-dir. (ZdCy)	: 0.2808
Total Effective Weight For X-dir. Seismic Loads (Wx)	: 26643.991176
Total Effective Weight For Y-dir. Seismic Loads (Wy)	: 26643.991176
Scale Factor For X-directional Seismic Loads	: 0.00
Scale Factor For Y-directional Seismic Loads	: 1.00
Accidental Eccentricity For X-direction (Ex)	: Positive
Accidental Eccentricity For Y-direction (Ey)	: Positive
Torsional Amplification for Accidental Eccentricity	: Consider
Torsional Amplification for Inherent Eccentricity	: Do not Consider
Total Base Shear Of Model For X-direction	: 0.000000
Total Base Shear Of Model For Y-direction	: 7481.979776
Summation Of Wi*Hi Of Model For X-direction	: 0.000000
Summation Of Wi*Hi Of Model For Y-direction	: 28614295.786545

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* ACCELERATION AND REDUNCTION FACTORS

- X - directional Values ----------

Design Horizontal Acceleration Factor (SaDx_d)	: 0.9840
Seismic Loads Reduction Factor (Fux_d)	: 2.2361

```
  The Modified Ratio of (SaDx_d/Fux_d)m                : 0.3728
  Design Horizontal Accel. Coef. (ZdCx_d)              : 0.2774

  Design Horizontal Acceleration Factor (SaDx_min)     : 0.8000
  Seismic Loads Reduction Factor (Fux_min)             : 2.2361
  The Modified Ratio of (SaDx_min/Fux_min)m            : 0.3300
  Minimum Horizontal Accel. Coef.(ZdCx_min)            : 0.1830

  Design Horizontal Acceleration Factor (SaDx_max)     : 1.2500
  Seismic Loads Reduction Factor (Fux_max)             : 2.7848
  The Modified Ratio of (SaDx_max/Fux_max)m            : 0.3774
  Maximum Horizontal Accel. Coef.(ZdCx_max)            : 0.2808

  - Y - directional Values ----------------------------------------

  Design Horizontal Acceleration Factor (SaDy_d)       : 0.9840
  Seismic Loads Reduction Factor (Fuy_d)               : 2.2361
  The Modified Ratio of (SaDy_d/Fuy_d)m                : 0.3728
  Design Horizontal Accel. Coef. (ZdCy_d)              : 0.2774

  Design Horizontal Acceleration Factor (SaDy_min)     : 0.8000
  Seismic Loads Reduction Factor (Fuy_min)             : 2.2361
  The Modified Ratio of (SaDy_min/Fuy_min)m            : 0.3300
  Minimum Horizontal Accel. Coef.(ZdCy_min)            : 0.1830

  Design Horizontal Acceleration Factor (SaDy_max)     : 1.2500
  Seismic Loads Reduction Factor (Fuy_max)             : 2.7848
  The Modified Ratio of (SaDy_max/Fuy_max)m            : 0.3774
  Maximum Horizontal Accel. Coef.(ZdCy_max)            : 0.2808

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==========================
 ECCENTRICITY RELATED DATA
==========================

              X - D I R E C T I O N A L    L O A D          Y - D I R E C T I O N A L    L O A D

     STORY   ACCIDENTAL INHERENT   ACCIDENTAL INHERENT      ACCIDENTAL INHERENT   ACCIDENTAL INHERENT
     NAME    ECCENT.    ECCENT.    AMP.FACTOR AMP.FACTOR    ECCENT.    ECCENT.    AMP.FACTOR AMP.FACTOR
----------   ---------- ---------- ---------- ----------    ---------- ---------- ---------- ----------
        4F       -56.5        0.0        1.0        0.0          10.5        0.0        1.0        0.0
        3F       -56.5        0.0        1.0        0.0          10.5        0.0        1.0        0.0
        2F       -39.5        0.0        1.0        0.0          10.5        0.0        1.0        0.0
       G.L         0.0        0.0        0.0        0.0           0.0        0.0        0.0        0.0
-------------------------------------------------------------------------------------------------------

-------------------------------------------------------------------------------------------------------
The accidental amplification factors are automatically set to 1.0 when torsional amplification effect
to accidental eccentricity is not considered.
The inherent amplification factors are automatically set to 0 when torsional amplification effect
to inherent eccentricity is not considered.
The inherent amplification factors are all set to 'the input value - 1.0'.(This is to exclude the true
inherent torsion)
-------------------------------------------------------------------------------------------------------

  ** Story Force = Seismic Force x Scale Factor + Added Force
```

S E I S M I C L O A D G E N E R A T I O N D A T A X - D I R E C T I O N

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
4F	11457.58	1350.0	4044.455	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3F	11985.1	950.0	2977.136	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2F	3201.317	550.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
G.L.	--	0.0	--	--	--	0.0	0.0	---	---	---

S E I S M I C L O A D G E N E R A T I O N D A T A Y - D I R E C T I O N

STORY NAME	STORY WEIGHT	STORY LEVEL	SEISMIC FORCE	ADDED FORCE	STORY FORCE	STORY SHEAR	OVERTURN. MOMENT	ACCIDENT. TORSION	INHERENT TORSION	TOTAL TORSION
4F	11457.58	1350.0	4044.455	0.0	4044.455	0.0	0.0	42466.78	0.0	42466.78
3F	11985.1	950.0	2977.136	0.0	2977.136	4044.455	1.6e+006	31259.93	0.0	31259.93
2F	3201.317	550.0	0.0	0.0	0.0	7021.591	4.4e+006	0.0	0.0	0.0
G.L.	--	0.0	--	--	--	7021.591	8.3e+006	---	---	---

==========
COMMENTS ABOUT TORSION
==========

If torsional amplification effects are considered :

Accidental Torsion = Story Force * Accidental Eccentricity * Amp. Factor for Accidental Eccentricity
Inherent Torsion = Story Force * Inherent Eccentricity * Amp. Factor for Inherent Eccentricity

---
If torsional amplification effects are not considered :

Accidental Torsion = Story Force * Accidental Eccentricity
Inherent Torsion = 0

The inherent torsion above is the additional torsion due to torsional amplification effect.
The true inherent torsion is considered automatically in analysis stage when the seismic force is applied to the structure.

[LOAD CASE : EZ]

** SELF WEIGHT DATA

; X=0, Y=0, Z=-0.1

** BEAM LOAD DATA

MEMBER	TYPE	DIR.	PROJ.	D1	P1	D2	P2	D3	P3	D4	P4

33	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
34	Uniform Load	GZ	NO	0	-0	0.31	-0.543	0.69	-0.543	1	-0
35	Uniform Load	GZ	NO	0	-0	0.31	-0.543	0.69	-0.543	1	-0
36	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
36	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
37	Uniform Load	GZ	NO	0	-0	0.15	-0.543	0.85	-0.543	1	-0
38	Uniform Load	GZ	NO	0	-0	0.15	-0.543	0.85	-0.543	1	-0
39	Uniform Load	GZ	NO	0	-0	0.43	-0.465	0.57	-0.465	1	-0
39	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
40	Uniform Load	GZ	NO	0	-0	0.5	-0.465	1	-0	0	0
41	Uniform Load	GZ	NO	0	-0	0.5	-0.465	1	-0	0	0
42	Uniform Load	GZ	NO	0	-0	0.43	-0.465	0.57	-0.465	1	-0
47	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
48	Uniform Load	GZ	NO	0	-0	0.31	-0.543	0.69	-0.543	1	-0
49	Uniform Load	GZ	NO	0	-0	0.31	-0.543	0.69	-0.543	1	-0
50	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
50	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
51	Uniform Load	GZ	NO	0	-0	0.15	-0.543	0.85	-0.543	1	-0
52	Uniform Load	GZ	NO	0	-0	0.15	-0.543	0.85	-0.543	1	-0
53	Uniform Load	GZ	NO	0	-0	0.5	-0.543	1	-0	0	0
53	Uniform Load	GZ	NO	0	-0	0.43	-0.465	0.57	-0.465	1	-0
54	Uniform Load	GZ	NO	0	-0	0.5	-0.465	1	-0	0	0
55	Uniform Load	GZ	NO	0	-0	0.5	-0.465	1	-0	0	0
56	Uniform Load	GZ	NO	0	-0	0.43	-0.465	0.57	-0.465	1	-0

midas Gen - Steel Code Checking [TWN-LSD96] Gen 2013

*.PROJECT :
*.UNIT SYSTEM : kgf, cm

[TWN-LSD96] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB	SECT	Section	Fy	LCB	Len	Ly	Cb	Ky	Bly	B2y	Pu	Muy	Muz	Def
	COM	SHR	Material			Lb	Lz		Kz	Blz	B2z	pPn	pMny	pMnz	Defa
	9	101	SC1, P 406.4x9			550.000	300.000	1.00	1.00	1.00	1.00	-14783	-31272	19456.4	0.02281
OK	0.05	0.00	STK400	2400.00	2	550.000	550.000		1.00	1.00	1.00	212220	3070619	3070619	1.83333
	10	101	SC1, P 406.4x9			550.000	300.000	1.00	1.00	1.00	1.00	-14783	31271.5	19456.4	0.02281
OK	0.05	0.00	STK400	2400.00	2	550.000	550.000		1.00	1.00	1.00	212220	3070619	3070619	1.83333
	11	101	SC1, P 406.4x9			550.000	300.000	1.00	1.00	1.00	1.00	-13860	-29370	-7732.4	0.01729
OK	0.04	0.00	STK400	2400.00	2	550.000	550.000		1.00	1.00	1.00	212220	3070619	3070619	1.83333
	12	101	SC1, P 406.4x9			550.000	300.000	1.00	1.00	1.00	1.00	-13860	29370.2	-7732.4	0.01729
OK	0.04	0.00	STK400	2400.00	2	550.000	550.000		1.00	1.00	1.00	212220	3070619	3070619	1.83333
	13	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-847.77	-13589	833.459	-
OK	0.03	0.01	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	14	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-847.77	-13589	-833.46	-
OK	0.03	0.01	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	15	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-796.31	-12747	-211.79	-
OK	0.03	0.01	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	16	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-796.31	-12747	211.790	-
OK	0.03	0.01	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	21	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-866.17	-3927.4	-750.87	-
OK	0.01	0.00	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	22	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-866.17	-3927.4	750.872	-
OK	0.01	0.00	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	23	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-808.56	-3698.4	125.522	-
OK	0.01	0.00	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	24	351	BR1, P 216.3x6			163.248	163.248	1.00	1.00	1.00	1.00	-808.56	-3698.4	-125.52	-
OK	0.01	0.00	STK400	2400.00	2	163.248	163.248		1.00	1.00	1.00	78918.8	573326	573326	-
	25	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	822.369	1506.76	19.2450	-0.0013
OK	0.01	0.00	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	85536.0	573326	573326	0.58333
	26	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.00	1.00	547.557	-39539	-26.531	-0.0341
OK	0.03	0.01	STK400	2400.00	2	790.000	790.000		1.00	1.00	1.00	127224	1265781	1265781	2.19444

midas Gen - Steel Code Checking [TWN-LSD96] Gen 2013

*.PROJECT :
*.UNIT SYSTEM : kgf, cm

[TWN-LSD96] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz	Def Defa
	27	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.00	1.00	547.557	-39539	26.5308	-0.0341
OK	0.03	0.01	STK400	2400.00	2	790.000	790.000		1.00	1.00	1.00	127224	1265781	1265781	2.19444
	28	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	771.435	1446.08	2.87798	-0.0012
OK	0.01	0.00	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	85536.0	573326	573326	0.58333
	29	101	SC1, P 406.4x9			300.000	300.000	1.00	1.00	1.00	1.00	-13484	23930.7	-117044	0.03091
OK	0.07	0.01	STK400	2400.00	2	300.000	300.000		1.00	1.00	1.00	223516	3070619	3070619	1.00000
	30	101	SC1, P 406.4x9			300.000	300.000	1.00	1.00	1.00	1.00	-13484	-23931	-117044	0.03091
OK	0.07	0.01	STK400	2400.00	2	300.000	300.000		1.00	1.00	1.00	223516	3070619	3070619	1.00000
	31	101	SC1, P 406.4x9			300.000	300.000	1.00	1.00	1.00	1.00	-12605	23192.1	146000	0.02887
OK	0.08	0.01	STK400	2400.00	2	300.000	300.000		1.00	1.00	1.00	223516	3070619	3070619	1.00000
	32	101	SC1, P 406.4x9			300.000	300.000	1.00	1.00	1.00	1.00	-12605	-23192	146000	0.02887
OK	0.08	0.01	STK400	2400.00	2	300.000	300.000		1.00	1.00	1.00	223516	3070619	3070619	1.00000
	33	301	Sb0, P 190.7x4.5			210.000	210.000	1.00	1.00	1.00	1.00	1.00186	21742.7	-21.825	-0.0273
OK	0.06	0.03	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	56808.0	337062	337062	0.58333
	34	252	SCB1, P 318.5x6			185.000	185.000	1.00	1.00	1.00	1.00	0.00000	-193988	163.520	0.01604
OK	0.15	0.04	STK400	2400.00	2	185.000	185.000		1.00	1.00	1.00	127224	1265781	1265781	0.51389
	35	252	SCB1, P 318.5x6			185.000	185.000	1.00	1.00	1.00	1.00	0.00000	-193988	-163.52	0.01604
OK	0.15	0.04	STK400	2400.00	2	185.000	185.000		1.00	1.00	1.00	127224	1265781	1265781	0.51389
	36	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	112.068	-47522	-5.9675	-0.0176
OK	0.08	0.04	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	85536.0	573326	573326	0.58333
	37	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.00	1.00	973.231	-566372	0.00000	-0.6239
OK	0.45	0.10	STK400	2400.00	2	790.000	790.000		1.00	1.00	1.00	127224	1265781	1265781	2.19444
	38	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.00	1.00	973.231	-566372	0.00000	-0.6239
OK	0.45	0.10	STK400	2400.00	2	790.000	790.000		1.00	1.00	1.00	127224	1265781	1265781	2.19444
	39	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	109.971	-46012	11.2044	-0.0170
OK	0.08	0.04	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	85536.0	573326	573326	0.58333
	40	252	SCB1, P 318.5x6			155.000	155.000	1.00	1.00	1.00	1.00	0.00000	-136828	-178.29	0.00816
OK	0.11	0.03	STK400	2400.00	2	155.000	155.000		1.00	1.00	1.00	127224	1265781	1265781	0.43056

midas Gen - Steel Code Checking [TWN-LSD96] Gen 2013

*.PROJECT :
*.UNIT SYSTEM : kgf, cm

[TWN-LSD96] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz	Def Defa
	41	252	SCB1, P 318.5x6			155.000	155.000	1.00	1.00	1.00	1.00	0.00000	-136828	-178.29	0.00816
OK	0.11	0.03	STK400	2400.00	2	155.000	155.000		1.00	1.00	1.00	127224	1265781	1265781	0.43056
	42	301	Sb0, P 190.7x4.5			210.000	210.000	1.00	1.00	1.00	1.00	1.30443	19294.3	23.8999	-0.0243
OK	0.06	0.03	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	56808.0	337062	337062	0.58333
	43	101	SC1, P 406.4x9			400.000	400.000	1.00	1.00	1.00	1.00	-6536.9	59968.7	-358515	0.05530
OK	0.13	0.02	STK400	2400.00	2	400.000	400.000		1.00	1.00	1.00	219731	3070619	3070619	1.33333
	44	101	SC1, P 406.4x9			400.000	400.000	1.00	1.00	1.00	1.00	-6536.9	-59969	-358515	0.05530
OK	0.13	0.02	STK400	2400.00	2	400.000	400.000		1.00	1.00	1.00	219731	3070619	3070619	1.33333
	45	101	SC1, P 406.4x9			400.000	400.000	1.00	1.00	1.00	1.00	-6085.4	58262.4	368332	0.06733
OK	0.14	0.02	STK400	2400.00	2	400.000	400.000		1.00	1.00	1.00	219731	3070619	3070619	1.33333
	46	101	SC1, P 406.4x9			400.000	400.000	1.00	1.00	1.00	1.00	-6085.4	-58262	368332	0.06733
OK	0.14	0.02	STK400	2400.00	2	400.000	400.000		1.00	1.00	1.00	219731	3070619	3070619	1.33333
	47	301	Sb0, P 190.7x4.5			210.000	210.000	1.00	1.00	1.00	1.00	-1.1856	23104.0	42.0259	-0.0297
OK	0.07	0.03	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	51072.7	337062	337062	0.58333
	48	252	SCB1, P 318.5x6			185.000	185.000	1.00	1.00	1.00	1.00	0.00000	-193988	-177.31	0.01604
OK	0.15	0.04	STK400	2400.00	2	185.000	185.000		1.00	1.00	1.00	127224	1265781	1265781	0.51389
	49	252	SCB1, P 318.5x6			185.000	185.000	1.00	1.00	1.00	1.00	0.00000	-193988	177.306	0.01604
OK	0.15	0.04	STK400	2400.00	2	185.000	185.000		1.00	1.00	1.00	127224	1265781	1265781	0.51389
	50	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	-251.26	-43969	44.4451	-0.0209
OK	0.08	0.04	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	77720.9	573326	573326	0.58333
	51	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.01	1.00	-1534.6	-556044	0.00000	-0.6964
OK	0.45	0.10	STK400	2400.00	2	790.000	790.000		1.00	1.01	1.00	93975.0	1265781	1265781	2.19444
	52	251	SB1, P 318.5x6			790.000	790.000	1.00	1.00	1.01	1.00	-1534.6	-556044	0.00000	-0.6964
OK	0.45	0.10	STK400	2400.00	2	790.000	790.000		1.00	1.01	1.00	93975.0	1265781	1265781	2.19444
	53	201	SG1, P 216.3x6			210.000	210.000	1.00	1.00	1.00	1.00	-243.51	-42566	-51.711	-0.0202
OK	0.08	0.04	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	77720.9	573326	573326	0.58333
	54	252	SCB1, P 318.5x6			155.000	155.000	1.00	1.00	1.00	1.00	0.00000	-136828	230.352	0.00816
OK	0.11	0.03	STK400	2400.00	2	155.000	155.000		1.00	1.00	1.00	127224	1265781	1265781	0.43056

midas Gen - Steel Code Checking [TWN-LSD96] Gen 2013

*.PROJECT :
*.UNIT SYSTEM : kgf, cm

[TWN-LSD96] CODE CHECKING SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

CHK	MEMB COM	SECT SHR	Section Material	Fy	LCB	Len Lb	Ly Lz	Cb	Ky Kz	B1y B1z	B2y B2z	Pu pPn	Muy pMny	Muz pMnz	Def Defa
	55	252	SCB1, P 318.5x6			155.000	155.000	1.00	1.00	1.00	1.00	0.00000	-136828	230.352	0.00816
OK	0.11	0.03	STK400	2400.00	2	155.000	155.000		1.00	1.00	1.00	127224	1265781	1265781	0.43056
	56	301	Sb0, P 190.7x4.5			210.000	210.000	1.00	1.00	1.00	1.00	-1.7973	20673.8	-48.225	-0.0267
OK	0.06	0.03	STK400	2400.00	2	210.000	210.000		1.00	1.00	1.00	51072.7	337062	337062	0.58333

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 midas Gen - RC-Beam Design     [ TWN-USD92 ]                                    Gen 2013
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 *.PROJECT   :
 *.UNIT SYSTEM : kgf, cm

   [TWN-USD92] RC-BEAM DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

 *.MEMB =   5, SECT =   501 (TG1, RECT), Span = 210.000
 *.Bc   = 30.000, Hc   = 50.000
 *.fc   = 210.000, fy   = 4200.00, fys = 2800.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	21098.1(1)	0.1707	2-D19	1553.59(33)	0.0126	2-D19	529.200(1)	0.0000	2-D13 @210
M	OK	288.536(25)	0.0023	2-D19	6728.17(33)	0.0544	2-D19	264.600(1)	0.0000	2-D13 @210
J	OK	21098.1(1)	0.1707	2-D19	1553.59(33)	0.0126	2-D19	529.200(1)	0.0000	2-D13 @210

 *.MEMB =   6, SECT =   551 (TB1, RECT), Span = 790.000
 *.Bc   = 30.000, Hc   = 50.000
 *.fc   = 210.000, fy   = 4200.00, fys = 2800.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	243980(1)	1.9985	2-D19	47670.5(1)	0.3862	2-D19	1974.41(1)	0.0000	2-D13 @210
M	OK	0.00000(41)	0.0000	2-D19	142730(1)	1.1625	2-D19	1011.79(1)	0.0000	2-D13 @210
J	OK	256926(1)	2.1061	2-D19	41197.8(1)	0.3337	2-D19	2007.19(1)	0.0000	2-D13 @210

 *.MEMB =   7, SECT =   551 (TB1, RECT), Span = 790.000
 *.Bc   = 30.000, Hc   = 50.000
 *.fc   = 210.000, fy   = 4200.00, fys = 2800.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	243980(1)	1.9985	2-D19	47670.5(1)	0.3862	2-D19	1974.41(1)	0.0000	2-D13 @210
M	OK	0.00000(41)	0.0000	2-D19	142730(1)	1.1625	2-D19	1011.79(1)	0.0000	2-D13 @210
J	OK	256926(1)	2.1061	2-D19	41197.8(1)	0.3337	2-D19	2007.19(1)	0.0000	2-D13 @210

 *.MEMB =   8, SECT =   501 (TG1, RECT), Span = 210.000
 *.Bc   = 30.000, Hc   = 50.000
 *.fc   = 210.000, fy   = 4200.00, fys = 2800.00

POS	CHK	N-Mu(LCB)	AsTop	Rebar	P-Mu(LCB)	AsBot	Rebar	Vu(LCB)	AsV	Stirrups
I	OK	20790.6(1)	0.1682	2-D19	1676.58(33)	0.0135	2-D19	529.200(1)	0.0000	2-D13 @210
M	OK	97.6846(25)	0.0008	2-D19	6992.39(1)	0.0565	2-D19	264.600(1)	0.0000	2-D13 @210
J	OK	20790.6(1)	0.1682	2-D19	1676.58(33)	0.0135	2-D19	529.200(1)	0.0000	2-D13 @210

midas Gen - RC-Column Design [TWN-USD92] Gen 2013

*.PROJECT :
*.UNIT SYSTEM : kgf, cm

[TWN-USD92] RC-COLUMN DESIGN SUMMARY SHEET --- SELECTED MEMBERS IN ANALYSIS MODEL.

MEMB SECT	Section Name Bc	Hc	fc Height	fy fys	LCB	Pu Rat-P	Mc Rat-M	Ast V-Rebar	Vu Rat-V	As-H H-Rebar
1	C1, RT		210.000	4200.00	1	18962.8	234384	40.180	1559.89	0.0000
401	60.000	60.000	150.000	2800.00		0.054	0.055	14- 5-D19	0.036	4-D13 @300
2	C1, RT		210.000	4200.00	1	18962.8	234384	40.180	1559.89	0.0000
401	60.000	60.000	150.000	2800.00		0.054	0.055	14- 5-D19	0.036	4-D13 @300
3	C1, RT		210.000	4200.00	1	17911.4	234288	40.180	1559.89	0.0000
401	60.000	60.000	150.000	2800.00		0.053	0.053	14- 5-D19	0.036	4-D13 @300
4	C1, RT		210.000	4200.00	1	17911.4	234288	40.180	1559.89	0.0000
401	60.000	60.000	150.000	2800.00		0.053	0.053	14- 5-D19	0.036	4-D13 @300

開挖擋土柱支撐分析計算書

目　　錄

開挖擋土柱支撐分析計算書

壹、土壤基本性質

（一）資料來源：固基鑽探有限公司鑽探及試驗分析報告書（4-4 節）
（二）地盤種類：第一類地盤（堅實地盤）
（三）工程性質參數建議值：

表 4-4 簡化設計用土壤參數表

層次	分類	深度 (m)	N值	單位重 (t/m^{33})	Kh (t/m^3)	Kv (t/m^3)	C (t/m^2)	φ
1	SF	0～1.8	—	1.8	—	—	0.0	26
2	GP/GM	1.8～30	50	2.2	5000	8000	1.0	40

（四）地下水位：設計高水位 GL-10m，常水位 GL-15m（P4-5）

貳、材料規格說明

一、鋼材料強度：A36
1. 降伏應力強度：fy=2500 kg/cm^2
2. 容許彎曲應力：Fb=0.6fy=1500 kg/cm^2
3. 容許剪斷應力：Fv=1000 kg/cm^2
4. 材料彈性模數：E=2.04 $\times 10^5$ kg/cm^2

二、材料應符合我國國家標準（CNS）或美國材料試驗學會規範（ASTM）或日本工業規格（JIS）等之規定。

三、材料斷面性質
1. 型鋼 H-300×300×10×15

斷面慣性矩　$Ix = 20400cm^4$
斷面模數　$Sx = 1360cm^3$
斷面積　$A = 119.8cm^2$
二次半徑　$\gamma x = 13.1cm$　$\gamma y = 7.51cm$

2. 鋼軌樁 50Kg（JIS-E1101）

斷面慣性矩　$Ix = 1740cm^4$
斷面模數　$Sx = 225cm^3$

斷面積　　$A = 64.3cm^2$

二次半徑　　$\gamma = 5.20cm$

參、開挖擋土設施分析

一、設計土壤參數及土壓應力

1. 第一層土壤（GL+0.0~GL-2.0m）

靜止土壓係數 $K_0 = 1 - \sin\phi = 1 - \sin 26^\circ = 0.562$

主動土壓係數 $K_a = \frac{1-\sin\phi}{1+\sin\phi} = \frac{1-\sin 26^\circ}{1+\sin 26^\circ} = 0.391$

被動土壓係數 $K_a = \frac{1+\sin\phi}{1-\sin\phi} = \frac{1+\sin 26^\circ}{1-\sin 26^\circ} = 2.56$

2. 第二層土壤（GL-2.0 以下）

靜止土壓係數 $K_0 = 1 - \sin\phi = 1 - \sin 40^\circ = 0.357$

主動土壓係數 $K_a = \frac{1-\sin\phi}{1+\sin\phi} = \frac{1-\sin 40^\circ}{1+\sin 40^\circ} = 0.217$

被動土壓係數 $K_a = \frac{1+\sin\phi}{1-\sin\phi} = \frac{1+\sin 40^\circ}{1-\sin 40^\circ} = 4.60$

3. 設計地下水位 GL-10.0m

4. 設計主動土壓應力

$\sigma_1 = \Sigma\gamma HKa = 1.8 \times 2.0 \times 0.391 = 1.41t/m^2 (GL - 2.0)$

$\sigma_2 = \Sigma\gamma HKa = 1.41 + 2.2 \times 1.15 \times 0.217 = 1.96t/m^2 (GL - 3.15)$

$\sigma_3 = \Sigma\gamma HKa = 1.96 + 2.2 \times 2.0 \times 0.217 = 2.91t/m^2 (GL - 5.15)$

$\sigma_4 = \Sigma\gamma HKa = 2.91 + 2.2 \times 3.85 \times 0.217 = 4.75t/m^2 (GL - 9.0)$

5. 設計地表加載土壓應力（$q = 2t/m^2$）

$qKa = 2.0 \times 0.391 = 0.782t/m^2 (GL + 0.0 \sim GL - 2.0)$

$qKa = 2.0 \times 0.217 = 0.434t/m^2 (GL - 2.0 \sim GL - 9.0)$

6. 設計被動土壓應力

$\sigma_p = \gamma DKp = 2.2 \times 3.85 \times 4.60 = 38.96t/m^2 (GL - 9.0)$

二、貫入深度分析

以斜撐頂部處為支點 0，計算主、被動土壓產生力矩

1.主動土壓力計算

$P_{a1} = 1.96 \times 2.0 = 3.92t/m$

$$P_{a2}=\frac{1}{2}(2.91-1.96)\times 2.0=0.95t/m$$
$$P_{a3}=2.91\times 3.85=11.2t/m$$
$$P_{a4}=\frac{1}{2}(4.75-2.91)\times 3.85=3.54t/m$$
$$P_{a5}=qHK_a=0.434\times 5.85=2.54t/m$$
$$P_a=P_{a1}+P_{a2}+P_{a3}+P_{a4}+P_{a5}$$
$$=3.92+0.95+11.2+3.54+2.54=22.2t/m$$

2.被動土壓力

$$P_P=0.5\gamma' D'^2 K_P=0.5\times 2.2\times 3.85^2\times 4.6=75.0t/m$$

$$M_o=3.92\times\frac{1.0}{2}+0.95\times\frac{2}{3}\times 2.0+11.2\times\left(\frac{3.85}{2}+2.0\right)+3.54\left(\frac{2}{3}\times 3.85+2.0\right)$$
$$+2.54\times\frac{5.85}{2}=70.8t-m$$
$$M_p=75.0\times\left(\frac{2}{3}\times 3.85+2.0\right)=342.5t-m$$
$$F_s=\frac{M_p}{M_a}=\frac{342.5}{70.8}=4.84\geq 1.5(ok)$$

三、支撐各部之設計

1. 設計土壓力, 採用 Peck 之視土壓力

$$P_a=\Sigma 0.65rHK_a=0.65\times(1.8\times 2.0\times 0.391+2.2\times 3.15\times 0.217)=1.89t/m^2$$

2. 擋土柱設計

$$Mu=1.6\times\frac{1.89\times 2\times 3.15^2}{2}=30.0t-m$$
$$m=\frac{fy}{0.85fc'}=\frac{4200}{0.85\times 210}=23.5$$
$$Rn=\frac{Mu}{\phi bd^2}=\frac{30.0\times 10^5}{0.9\times 100\times(50-5)^2}=16.5$$
$$\rho=\frac{1}{m}\left(1-\sqrt{1-\frac{2mRn}{fy}}\right)=\frac{1}{23.5}\left(1-\sqrt{1-\frac{2\times 23.5\times 16.5}{4200}}\right)=4.13\times 10^{-3}$$
$$As=\rho bd=4.13\times 10^{-3}\times 100\times 45=18.6cm^2$$

主筋 Use 5-#7（基地外側）

3. 水平圍苓設計

常用尺寸 $H-300\times300\times10\times15 \quad A=119.8cm^2 \quad Sx=1360cm^3$

$$M=\frac{1.89\times5.15\times1.0^2}{8}=1.21t-m/m$$

$$S_{req}=\frac{M}{Fb}=\frac{1.21\times10^5}{0.6\times2500}=80.7cm^3<1360cm^3(ok)$$

4. 斜撐強度檢核（50Kg 鋼軌樁）

$$\frac{K\ell}{r}=\frac{200\sqrt{2}}{5.20}=54.4<C_c=\sqrt{\frac{2\pi^2E}{Fy}}=126.9$$

$$Fs=\frac{5}{3}+\frac{3}{8}\left(\frac{\frac{k\ell}{r}}{C_c}\right)-\frac{1}{8}\left(\frac{\frac{k\ell}{r}}{C_c}\right)^3=\frac{5}{3}+\frac{3}{8}\left(\frac{54.4}{126.9}\right)-\frac{1}{8}\left(\frac{54.4}{126.9}\right)^3=1.82$$

$$Fa=\frac{1}{FS}\times\left[1-\frac{1}{2}\left(\frac{\frac{K\ell}{r}}{C_C}\right)^2\right]fy=\left[1-\frac{1}{2}\left(\frac{54.4}{126.9}\right)^2\right]\times2500/1.82=1079.2kg/cm^2$$

$$P=\frac{1}{\sqrt{2}}\times1.89\times1.0\times5.15=6.9t$$

$$f_a=\frac{P}{A}=\frac{6.9\times10^3}{64.3}=107.3kg/cm^2<1079.2kg/cm^2(OK)$$